## **OPERATION & MAINTENANCE PLAN**

# CORNELL-DUBILIER ELECTRONICS SUPERFUND SITE OPERABLE UNIT 2 333 HAMILTON BOULEVARD

SOUTH PLAINFIELD, NJ 07080-3310

FEBRUARY 2014 VERSION

# OPERATION AND MAINTENANCE PLAN CORNELL-DUBILIER ELECTRONICS SUPERFUND SITE

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This Operation and Maintenance (O&M) Plan is follow-on work to the Remedial Actions (RA) completed at the Cornell-Dubilier Electronics Superfund Site, Operable Unit 2 (Site). The RA was performed pursuant to the Comprehensive Environmental Response, Compensation, and Liability Act, as amended, ("CERCLA"), and in accordance with the Record of Decision (ROD) for Operable Unit 2 (OU2). The ROD was signed by the United States Environmental Protection Agency (EPA) in September 2004. EPA Region II is the lead agency performing the work at the Site, and the Remedial Project Manager is the main Point of Contact (POC) for O&M at the Site.

This O&M Plan includes the operation and maintenance of the following primary site systems and components installed as part of RA restoration at the Site:

- Stormwater Retention Basin
  - The stormwater retention basin includes all conveyance piping, collection system structures, sand filter systems, and features collectively to manage stormwater runoff at the site in accordance with NJDEP regulatory requirements.
- Pavement Cap
  - o The Pavement Cap includes approximately 23 acre bituminous pavement serving as an engineered cap, in accordance with the OU2 ROD.
- Miscellaneous Site Maintenance
  - O Site access control features including perimeter fencing and gates and warning signage. Also includes sidewalk snow removal, vegetation and weed control, trash and debris removal, and general housekeeping.

This O&M Plan documents the strategy to address the O&M regulatory requirements for the stormwater management measures as required by NJDEP, and includes O&M for the other site features summarized above. This plan is required by N.J.A.C. 7:8 Stormwater Management Rules and contains the required elements described in the NJDEP Stormwater BMP Manual, Chapter 8 – Maintenance and Retrofit of Stormwater Management Measures and Chapter 9.9 Standard for Sand Filters. This plan also addresses the inspection routine and long term maintenance required for the Pavement and site maintenance. This O&M Plan contains specific preventative and corrective maintenance tasks, schedules, and the name, address, and telephone number of the EPA Point of Contact responsible for the site maintenance.

#### I.I STORMWATER MANAGEMENT MEASURES

According to the NJDEP Stormwater Management Rules, all maintenance plans for stormwater management measures must include the following items as addressed in this maintenance plan:

- The contact information of the person(s) responsible for the preventative and corrective maintenance of the stormwater management measure.
  - Refer to Appendix E POC information.
- Specific preventative and corrective maintenance tasks such as:
  - Removal of sediment, trash, and debris;
  - Mowing, pruning, and restoration of vegetation;
  - Inspecting for erosion and restoration of eroded areas;
  - Elimination of mosquito breeding habitats;
  - Control of aquatic vegetation;
  - Repair or replacement of damaged or deteriorated components.

#### Refer to Section 2

- A schedule of regular inspections and tasks, including detailed inspection tasks and schedules for the structural stormwater management measures.
  - Refer to Table 1 Scheduled Maintenance Schedule
- Detailed logs of all preventative and corrective maintenance performed at the stormwater management measure, including all maintenance-related work performed.
  - Refer to Appendix D Example Field Report forms.

In addition, as suggested by the NJDEP Stormwater Management Facility Maintenance Manual, the following items are also included in the maintenance plan:

- Recommended corrective responses to various emergency conditions that may be encountered at the stormwater management measure.
  - Refer to Section 2
- Site Specific Health and Safety Plan (SSHASP). Procedures and equipment required to protect the safety of inspection and maintenance personnel. TO BE PROVIDED BY O&M CONTRACTOR.

- Approved disposal or recycling sites and procedures for sediment, trash, debris, and other material removed from the measure during maintenance operations. TO BE DETERMINED BY O&M CONTRACTOR.
- As-built construction plans of the stormwater management measure and copies of pertinent construction documents such as laboratory test results, permits, and completion certificates.

Refer to Appendix C- Basin Record Drawings.

As required by the NJDEP Stormwater Management Rules, this Plan addresses the site-specific maintenance plans for stormwater management measures, as listed in Table 1 and 2, and includes the following items:

- Scheduled Maintenance preventative and corrective maintenance tasks, including:
  - Removal of sediment, trash, and debris;
  - Mowing, pruning, and removal of vegetation;
  - Inspecting for erosion;
  - Elimination and/or treatment of mosquito breeding habitats;
  - Control of aquatic vegetation;
  - Pavement inspections, crack sealing, and seal coating
- Unscheduled Maintenance corrective maintenance tasks, including:
  - Removal and replacement of clogged filter material
  - Restoration of eroded areas
  - Repair or replacement of damaged or deteriorated components

This section also addresses the following topics:

Recommended corrective responses to various emergency conditions that may be encountered at the stormwater management measure.

#### 2.1 BASIN MAINTENANCE

This O&M Plan documents primarily the maintenance regulatory requirements for the stormwater management measures for the Site as required by NJDEP. The creation of this plan is required by N.J.A.C. 7:8 Stormwater Management Rules and this O&M Plan contains all of the required elements described in the NJDEP Stormwater BMP Manual, Chapter 8 – Maintenance and Retrofit of Stormwater Management Measures and Chapter 9.9 Standard for Sand Filters.

As implied by their name, stormwater management measures are expected to become the repositories for sediment, nutrients, trash, debris, and other pollutants targeted by the NJDEP Stormwater Management Rules. For this reason, stormwater management measures share maintenance requirements related to the other remedial controls of the Site including the Pavement Cap and drainage structures, all of which require regular inspection and cleaning, sediment and debris removal, and periodic replacement and/or repairs.

Research and experience have demonstrated that regular and thorough maintenance is necessary for stormwater management measures to perform effectively and reliably. They have also

demonstrated that failure to perform such maintenance can lead to diminished performance, deterioration, and failure, in addition to a range of health and safety problems including mosquito breeding, vermin, and the potential for drowning. The potential for such problems to develop is accentuated by many of the very features and characteristics that allow stormwater management measures to do their job, including standing or slowing moving water, dense vegetation, forebays, trash racks, dams, and the need to continually function in all types of weather.

In recognition of these needs and potential problems, the NJDEP Stormwater Management Rules require that an O&M Plan be developed for all stormwater management measures incorporated into the design of a major development. Preparation of a an O&M Plan is an administrative requirement, and under CERCLA Section 121(e)(1), a Superfund response action need only comply with substantive, not administrative, requirements. However, EPA may elect to participate in a "permit equivalency" process, as a means of determining compliance with the substantive requirements of a permitting program. This O&M Plan has been prepared in accordance with the permit equivalency process.

The stormwater management measures defined as the Basin system for the Site consists of the following components. These components are illustrated on the Basin Record Drawings located in <u>Appendix C.</u>

- Stormwater collection system
  - 23 Catch Basins
  - 5 Inlet Structures
- Stormwater Detention Basin
  - Eastern Forebay
  - Western Forebay
  - Surface Sand Filter Center
  - Orifice Outlet Structure
  - Secondary Outlet Structure
  - Emergency Spillway

As described in Chapter 8 of the NJDEP Stormwater BMP Manual, sand filters are normally used to remove relatively large amounts of sediments, metals, hydrocarbons, and floatables from stormwater runoff. Sand filters use solids settling, filtering, and adsorption processes to reduce pollutant concentrations in stormwater. The NJDEP adopted total suspended solids ("TSS")

removal rate for sand filters is 80 percent. At this Site, runoff entering the sand filter is conveyed first through the eastern and western forebays, which removes trash, debris, and coarse sediment, and is then conveyed through the forebay / sand filter transition structure and into the surface sand filter bed located in the center. The treated stormwater then exits through the orifice outlet structure and the secondary outlet structure.

Sand filters are normally used in highly impervious areas with relatively high TSS, heavy metal, and hydrocarbon loadings such as roads, driveways, drive-up lanes, parking lots, and urban areas.

As required by NJDEP, a sand filter must have a maintenance plan and, should be protected by easement, deed restriction, ordinance, or other legal measures that prevent its neglect, adverse alteration, and removal. Because this work is being performed pursuant CERCLA, EPA will implement institutional controls to protect and maintain the stormwater management measures for the Site.

An emergency action plan for the spillway is not required as it is not a Class I or II Dam as defined in the NJDEP Dam Safety Standards at N.J.A.C.

#### 2.1.1 Drain Time Inspections

Sufficient total temporary storage volume is provided within the forebay and sand bed zones (including the sand bed itself) to contain the design runoff volume and direct it through the sand bed without overflow. The design thickness of the sand bed provides adequate pollutant removal, while the bed's permeability or infiltration rate must drain the stored runoff to the top of the sand bed within 72 hours, as required by NJDEP Stormwater BMP Manual, Table 9.9-1. In addition, the capacity of the sand bed underdrain geotextile must allow the sand bed to drain freely, while the overflow must safely convey the runoff from storms greater than the design storm. The calculated normal drain or drawdown time for the Site's sand filter is 21 hours. Backup calculations are included in Appendix A.

The sand filter's normal drain (or drawdown) time is used to evaluate the filter's actual performance. If significant increases or decreases in the normal drain time are observed, the filter's sand bed, underdrain system, and tailwater levels must be evaluated and appropriate measures taken in order to comply with the maximum drain time requirements set by NJDEP and to maintain the proper functioning of the filter.

As required by NJDEP, twice a year the basin's drain time shall be inspected and monitored after a storm event which meets the design criteria of 1.25 inches in 2 hours, as measured at the nearest rain gauge, currently the Middlesex USGS unheated rain gage 403506074302801. Beginning 16 hours after such a storm event, the water level shall be observed every 2 hours until it drops below the top of the sand layer. The approximate time that occurs shall be noted and compared to the normal drain time of 21 hours. Any variance of 20% or greater is cause for the filter's sand bed, underdrain system, and/or tailwater levels to be evaluated. The infiltration rate of the sand bed material may also be retested. If the water fails to infiltrate 72 hours after the end of the stormwater quality design storm, corrective measures shall be taken by performing unscheduled maintenance to further investigate and mitigate any localized flow reductions.

Refer to Table 1 – Scheduled Maintenance Schedule.

# 2.1.2 Debris, Trash, and Sediment Removal and Inspections (Scheduled and Unscheduled)

As required by NJDEP, all sand filter components expected to receive and/or trap debris and sediment shall be inspected for clogging and excessive debris and sediment accumulation at least four times annually as well as after every storm exceeding 1 inch of rainfall. These inspection and removal activities are considered scheduled and unscheduled events on the operation and maintenance schedule (refer to Tables 1 and 2).

The inspection components include inlets, diversion structures, forebays, sand beds, and overflows. Sediment removal shall take place when all runoff has drained from the sand bed and the sand is reasonably dry. Disposal of debris, trash, sediment, and other waste material shall be at a EPA-approved landfill which is in compliance with all applicable local, state, and federal waste regulations. Disposal documentation shall be included with the field report from each event.

Inspection and elimination or treatment of mosquito breeding habitat shall be performed during the quarterly events if necessary, except during winter. The mosquito habitat treatment or elimination protocol shall be in accordance and coordinated with the Middlesex County Health Department.

Refer to Table 1 - Scheduled Maintenance Schedule.

#### 2.1.3 Annual Basin Maintenance and Inspections

As required by NJDEP, all structural components as described in this Section shall be inspected for cracking, subsidence, spalling, erosion, and deterioration at least annually. In addition to structural inspections, each structure shall be inspected for the accumulation of debris, trash, and sediment. Each catch basin, inlet, outlet, and overflow structure in the collection system shall be cleaned annually by a sewer vacuum truck. The degree of accumulations for each structure shall be documented on the field report, and structures exhibiting faster rates of accumulations shall be scheduled for more frequent cleanings.

Refer to Table 1 - Scheduled Maintenance Schedule.

#### 2.1.4 Basin Unscheduled Maintenance

Additional maintenance measures may be identified and required following scheduled inspection.

These unscheduled emergency conditions or as-needed repairs are included as a separate unscheduled maintenance event for the Basin to cover items such as:

- Erosion repairs
- Liner /Filter penetrations
- Check Valve replacements
- Filter material Replacement

#### 2.1.5 Filter Material Replacement Events

The filter drain time evaluation as described in Section 2.1.1 will be used to determine when to schedule the replacement of filter materials within the forebays and surface sand filter. Filter Material replacement is not expected to occur frequently and shall be performed as a last resort after an engineering evaluation is performed by USACE / EPA.

The filter material replacement would consist of mobilizing rubber tired earthwork equipment to the bottom of the Basin to perform the following steps:

- Remove and segregate the 4-inch layer of aggregate for disposal or screening and reuse
- Remove and dispose of the geotextile filter fabric
- Remove and dispose of approximately the top 6-inches of the 24-inch sand layer
- Replace the top of the sand layer with new filter sand, restoring to design elevations
- Replace the geotextile filter fabric
- Replace the 4-inch layer of aggregate

The sideslopes of the basin would require use of specialized equipment such as conveyors, chutes, and/or roll-off boxes to lift and lower materials to and from the bottom of the basin to the surface for subsequent consolidation, staging and disposal. A filter material replacement event report shall be prepared to document the volumes and costs of the materials disposed and replaced.

Refer to Table 2 – Unscheduled Maintenance.

#### 2.2 PAVEMENT MAINTENANCE

The general objectives of the pavement inspections and maintenance for this Site are to maintain the infiltration barrier/protective cap, preserve the capital investments, and provide a safe traveling surface. Maintenance of pavement includes the preservation, restoration, and repair of both surface and underlying layers.

An effective pavement maintenance program specifies the procedures to be followed to assure that proper preventative and remedial pavement maintenance is performed. This pavement maintenance plan is based on guidance documents from the US Department of Transportation and the National Cooperative Highway Research Program (NCHRP). The pavement maintenance program for this Site consists of the following activities as outlined in each of the following subsections:

- Annual inspections
- Pavement Condition Index (PCI) Surveys
- Capital Preventative Maintenance (CPM)

#### 2.2.1 Annual Inspections

A drive-by inspection shall occur annually to document changes in the pavement condition. A field report shall be prepared to document the types of distress, their locations, and remedial action proposed or performed (refer to Appendix D for an example pavement drive-by inspection and the blank report template). Various external signs or indicators make the deterioration of a pavement apparent, and often reveal the probable causes of the failure. While different pavement distresses possess their own particular characteristics, the various types of flexible pavement distress generally fall into one of the following broad categories as outlined below for purposes of the drive-by inspections.

- Cracking: Cracks in flexible pavements are caused by deflection of the surface over an unstable foundation, shrinkage of the surface, thermal expansion and contraction of the surface, poorly constructed joints, or reflection cracking. Since cracks allow surface water to enter the subgrade and further destroy the stability of the subgrade, sealing should be accomplished as soon as practical. Efforts should be made to avoid a buildup of crack sealing material.
- <u>Disintegration</u>: Disintegration in a flexible pavement is caused by insufficient compaction of the surface, insufficient asphalt binder in the mix, loss of adhesion between the asphalt coating and aggregate particles, or severe overheating of the mix.
- <u>Distortion</u>: Distortion in HMA pavements is caused by foundation settlement, insufficient compaction of the pavement courses, lack of stability in the bituminous mix, poor bond between the surface and the underlying layer of the pavement structure, and swelling soils or frost action in the subgrade.

#### 2.2.2 Pavement Condition Index Surveys

It is currently envisioned that the asphalt capping material will be subject to very limited vehicular traffic while under EPA control and therefore a Pavement Condition Index (PCI) survey is not expected to be required. However, should the Site usage be increased and found to be subject to more vehicular traffic than expected, a PCI survey shall be required every 3 years, to measure and record the pavement deterioration. The PCI survey shall be in accordance with the applicable provisions of ASTM D 5340 and submitted by a licensed civil engineer experienced in the inspection and maintenance of pavements. The PCI, developed by the United States Army Corps of Engineers, is widely used in transportation engineering and is based on a visual survey of the pavement and a numerical value between 0 and 100 defines the condition with 100 representing an excellent pavement. The process involves the following steps:

- Divide the total pavement section into sample units (each approximately 5,000 square feet).
- Based on the number of sample units in the total section, a certain number of these units are selected to be tested. For example if there are 40 or more sample units, 10% are tested.
- The type, extent and severity of pavement distress in each section are recorded using the ASTM Standard D 5340 method.
- The PCI of each tested sample unit is calculated using the method defined in the standard. In summary this involves calculating the distress quantities and the distress densities for each

tested unit. These values are used to determine a deduct value and this deduct value is subtracted from 100 to give the PCI value.

The PCI of the total section is then determined based on the sample values.

The PCI provide a good indication of the pavement condition of a network. The licensed civil engineer shall submit with the PCI survey a recommendation and plan for pavement capital preventative maintenance (CPM) treatments and corresponding cost estimates. The typical CPM treatments are as described in the following section.

#### 2.2.3 Critical Preventative Maintenance Pavement Treatments

Techniques used to determine the appropriate time to apply capital preventive maintenance include the following:

- A pavement management system
  - A predetermined treatment schedule or time since a previous maintenance or rehabilitation event
- Maintenance surveys
  - Determining existing distresses to be treated, or anticipated distresses to be prevented or slowed

This maintenance plan combines both approaches by using maintenance (PCI) surveys and scheduled CPM treatments to preserve the life of the pavement cap based on recommendations from the National Cooperative Highway Research Program (NCHRP). Typical bituminous-surfaced pavement capital preventative maintenance (CPM) treatments consist of the following:

- Crack Filling/Crack Sealing
- Fog Seals
- Slurry Seals
- Scrub Seals
- Microsurfacing
- Chip Seals
- Thin Overlay
- Ultrathin Friction Courses

The typical expected life and costs of each CPM treatment are as follows. There is a broad range for some of the treatment lives because the expected life estimates are based on the use of treatment in both a preventative and reactive manner. The costs also cover a wide range of applications and

regional variations. Since the treatments also depend on the distresses, the schedule and budget in this O&M Plan assume an average of 3 years between CPM treatments.

- Crack Filling/Crack Sealing
  - Expected Life: 2 to 6 years
  - Typical Costs: \$0.30 to \$1.50/linear foot
- Fog Seals
  - Expected Life: 1 to 2 years
  - Typical Costs: \$0.30 to \$0.45/square yard
- Slurry Seals
  - Expected Life: 3 to 5 years
  - Typical Costs: \$0.30 to \$0.45/square yard
- Scrub Seals
  - Expected Life: 1 to 3 years
  - Typical Costs: \$0.75 to \$1.25/square yard
- Microsurfacing
  - Expected Life: 4 to 7 years
  - Typical Costs: \$0.90 to \$1.70/square yard
- Chip Seals
  - Expected Life: 4 to 7 years
  - Typical Costs: \$0.75 to \$0.90/square yard for single application
  - Typical Costs: \$1.10 to \$1.25/square yard for double application
- Thin Overlay
  - Expected Life: 7 to 10 years
  - Typical Costs: \$1.75 to \$2.00/square yard for dense graded mixes
- Ultrathin Friction Courses
  - Expected Life: 7 to 10 years
  - Typical Costs: \$2.50 to \$3.00/square yard

Emergency repairs may be necessary because of the time of the year and season (i.e., winter) may delay implementation of the CPM treatments. Emergency repairs may include the following and shall be included in the annual unscheduled maintenance budget:

Cold patching of pot holes and/or large cracks

Hot patching of pot holes and/or large cracks

#### 2.3 MISCELLANEOUS SITE MAINTENANCE

The general objectives of maintenance for the miscellaneous site features are to perform seasonal cleanup, maintenance, and repairs to preserve the capital investments and to provide emergency / unscheduled maintenance if needed.

The maintenance program, at a minimum shall consist of the following activities as outlined in each of the following sub-sections:

- Quarterly inspections.
- Seasonal maintenance.
- Unscheduled maintenance / repairs.

#### 2.3.1 Quarterly Inspections

An inspection shall occur quarterly and a field report shall be prepared and submitted to identify and document any of the following common problems:

- Infrastructure Damage.
- Accumulation of trash and debris.
- Vegetation and weed growth.

#### 2.3.2 Quarterly Maintenance

Maintenance shall be performed quarterly at a minimum. All disposal activities shall be in accordance with the approved waste disposal policies. Trees, shrubs, and underbrush shall be pruned, trimmed, or removed and weeds shall be controlled as necessary during the growing season. The quarterly seasonal maintenance events shall include the following activities:

- Site Maintenance
  - Removal of trash and debris and dead and downed vegetative debris
  - Perimeter fencing repairs, gate locks, monitoring well locks, signage.
  - Vegetation and weed control, mowing, trimming, pruning or removal.
- Winter Maintenance
  - Removal of snow from Hamilton Blvd pedestrian sidewalk and entrance gate areas.

#### 2.3.3 Unscheduled Site Maintenance

Unscheduled site maintenance includes provisions for emergencies such as storm or vandalism damage and debris removal, perimeter fence repairs for site security.

#### 2.4 SCHEDULED MAINTENANCE

As described in this O&M Plan, there are multiple "scheduled" maintenance events on quarterly and yearly cycles for each major component of the Site. Refer to Table 1 for the scheduled maintenance events and their corresponding time cycles. These scheduled maintenance events shall occur based on the scope of work as defined in this O&M Plan a field report shall be prepared and submitted to document the completion of each scheduled event. Completed field reports from scheduled maintenance events shall be retained in accordance with Section 3.0 of this O&M Plan.

#### 2.5 Unscheduled Maintenance

As described in this O&M Plan, there are multiple "unscheduled" maintenance activities that are expected to occur on an as-needed or emergency basis. Potential unscheduled maintenance activities are outlined on Table 2. Upon identification of the unscheduled maintenance, a Field Change Request shall be submitted to the contracting officer. Following completion of the unscheduled maintenance a field report shall be prepared to document the corrective actions implemented and shall be retained in accordance with Section 3.0 of this O&M Plan.

The NJDEP Stormwater Management Rules require that the following procedures be followed:

- This O&M plan shall be maintained by EPA has owner / operator and Point-of-Contact of the stormwater management measure.
  - Copies shall also be submitted to the NJDEP upon request.
  - Copies shall be provided to the local mosquito control (Middlesex County Health Department) upon request.
- The title and date of the maintenance plan and the name, address, and telephone number of the person with stormwater management measure maintenance responsibility as specified in the plan shall be recorded on the deed of the property on which the measure is located. Any change in this information due, for example to a change in property ownership, shall also be recorded on the deed.

(Note: EPA is currently the POC for the Site, as the lead agency conducting the remedial activities. EPA does not and will not hold title to the property. EPA will advise NJDEP when and if another party assumes responsibility for O&M at the Site. Further, EPA anticipates that upon completion of Superfund remedial activities, EPA and NJDEP will work cooperatively, as lead agency and support agency, to ensure that appropriate information is recorded on the deed.)

- The person with maintenance responsibility as designated in this plan (EPA) shall evaluate the maintenance plan for effectiveness and at least annually and revise as necessary.
- Field Report Forms (Appendix D) and any additional information documenting preventative and corrective maintenance performed at the Site shall be kept as part of the Maintenance History (Appendix G). The person with maintenance responsibility as designated in this plans shall retain and, upon request, make available the maintenance plan and associated logs and other records for review by a public entity with administrative, health, environmental, or safety authority over the site.

#### 3.1.1 Site Access

All stormwater management measures' components, pavement, and site features, shall be readily accessible for inspection and maintenance. Therefore, trees, shrubs, and underbrush shall be pruned, trimmed, or removed as necessary to maintain access to the stormwater management measure.

#### 3.1.2 Training of Maintenance Personnel

Maintenance personnel shall be trained as appropriate in specialized inspection and maintenance tasks and/or the operation and care of specialized maintenance equipment. Refresher training of maintenance personnel shall occur as needed, preferably just prior to annual inspection milestones, to cover lessons learned during the previous preventive and corrective actions, and to review any new maintenance provisions and/or requirements.

- New Jersey Stormwater BMP Manual, New Jersey Department of Environmental Protection, February 2004.
- Guidelines and Procedures for Maintenance of Airport Pavements, Advisory Circular No. 150/5380-6B, US Department of Transportation, Federal Aviation Administration, 2007.
- Optimal Timing of Pavement Preventative Maintenance Treatment Applications, NCHRP Report 523, National Cooperative Highway Research Program, Transportation Research Board, 2004.
- ASTM D5340-10: Standard Test Method for Airport Pavement Condition Index Surveys.
- United States Environmental Protection Agency, Region II, September 2004. "Record of Decision for Operable Unit 2" Cornell-Dubilier Electronics Superfund Site, South Plainfield, New Jersey.

### Cornell-Dubilier Electronics Superfund Site Operable Unit 2 February 2014

**Table 1 - Routine Scheduled Maintenance** 

Maintenance Category	Event	Frequency
Stormwater Basin and Collection System	Drain Time Inspections	Twice a Year
	Debris, Trash, Vegetation and Sediment Removal and Inspections	Quarterly
	Annual System Structural Maintenance and Inspections	Annually
	Unscheduled Inspections After 1-inch Storm Events	As needed
Pavement	Annual Inspections	Twice a Year
	Pavement Condition Index Surveys	2-Year Cycle
	Pavement Maintenance and Repairs	As needed
General Housekeeping	Monthly Inspections	Monthly
	Debris, Trash, and Vegetation Cleanup	As Needed
	Snow Removal	Per Event > 2"

**Table 2 - Unscheduled Maintenance** 

Maintenance Category	Event	Frequency
Stormwater Basin and	Unscheduled Filter Material Replacement Events (Assumes	As needed -
Collection System	24,000 sq feet )	Dependent on Drain
		Time Inspections
Pavement	Critical Preventative Maintenance Treatments (Assumes 106,000 sq yds)	3-Year Cycle
General House Keeping	NA	

## **APPENDIX A**

# APPENDIX A Normal Drain (Drawdown) Time Calculation Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

From the New Jersey Stormwater Best Management Practices Manual, Equation 9.9-1:

$$A_S = \frac{(V_{QS})(TH_S)}{[(k)(\frac{D_{ST}}{2} + TH_S)(T_D)]}$$

Equation 9.9-1

#### Where:

 $A_S$  = Minimum Sand Bed Surface Area (in square feet)

 $V_{QS}$  = Runoff Volume from the Stormwater Quality Design Storm (in cubic feet)

TH<sub>S</sub> = Thickness of Sand in Sand Bed (in feet)

k = Sand Bed Design Permeability (in feet per day)

 $D_{ST}$  = Maximum Temporary Sand Bed Depth (in feet)

 $T_D$  = Sand Bed Drain Time (in days)

Solving for the Sand Bed Drain Time (T<sub>D</sub>) and using the values and/or actual measurements below,

 $A_S$  = Minimum Sand Bed Surface Area = 9,660 ft<sup>2</sup>

 $V_{QS}$  = Runoff Volume from the Stormwater Quality Design Storm = 82,621 ft<sup>3</sup>

 $TH_S$  = Thickness of Sand in Sand Bed = 2 ft

k = Sand Bed Design Permeability = 4 ft/day

 $D_{ST}$  = Maximum Temporary Sand Bed Depth = 3.5 ft

 $T_D = .88$  days or 21.1 hours

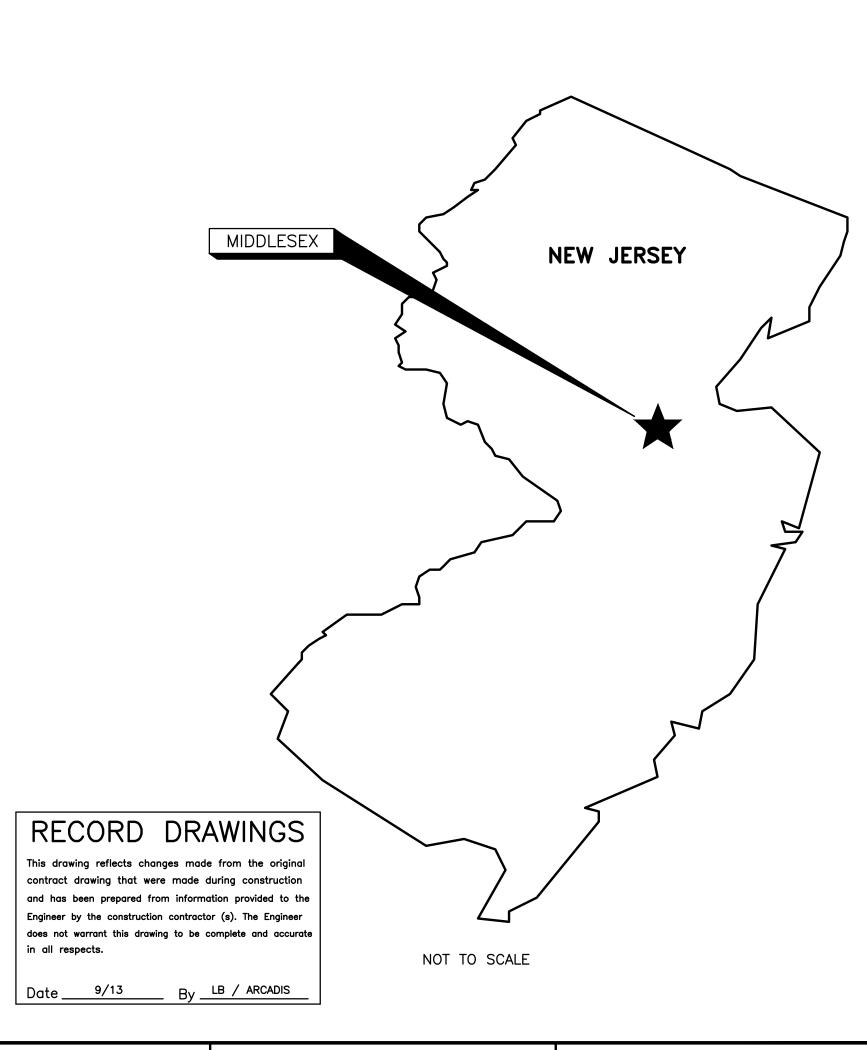
# **APPENDIX B**

# CORNELL-DUBILIER ELECTRONICS SUPERFUND SITE OU2 SOILS REMEDIATION SOUTH PLAINFIELD, NEW JERSEY

BOROUGH OF SOUTH PLAINFIELD MIDDLESEX COUNTY, NEW JERSEY

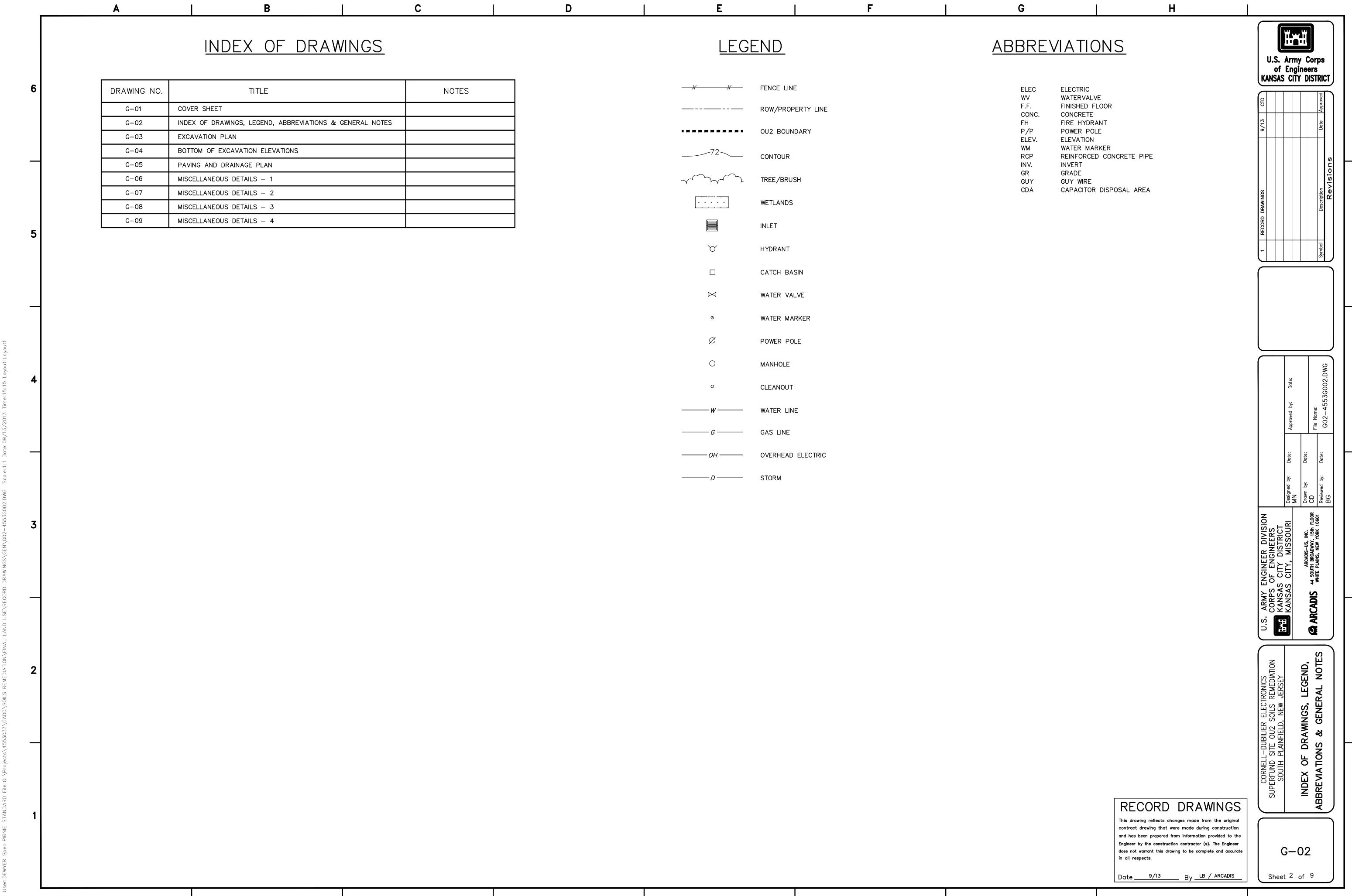


SCALE: 1" = 24,000'

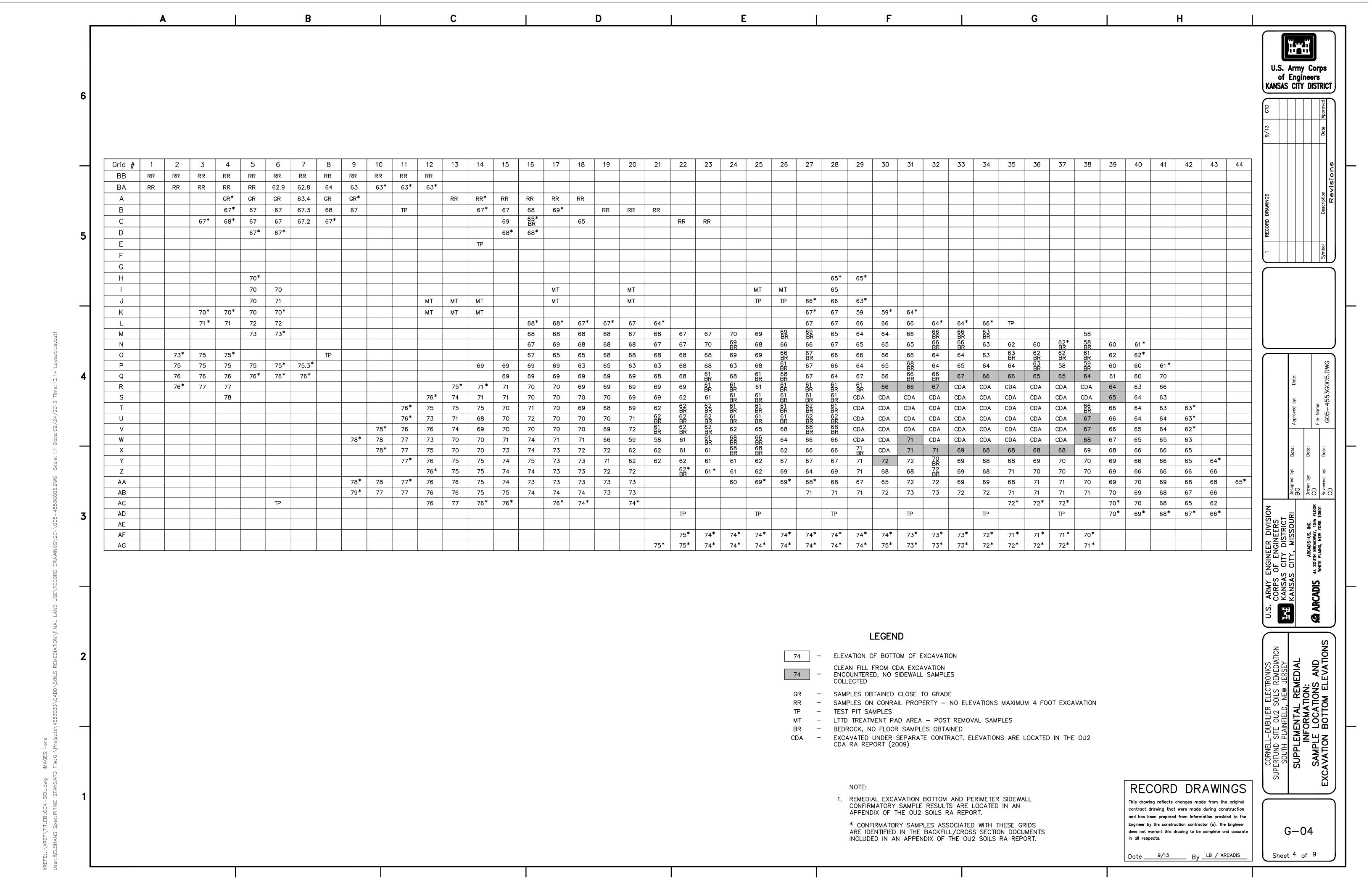


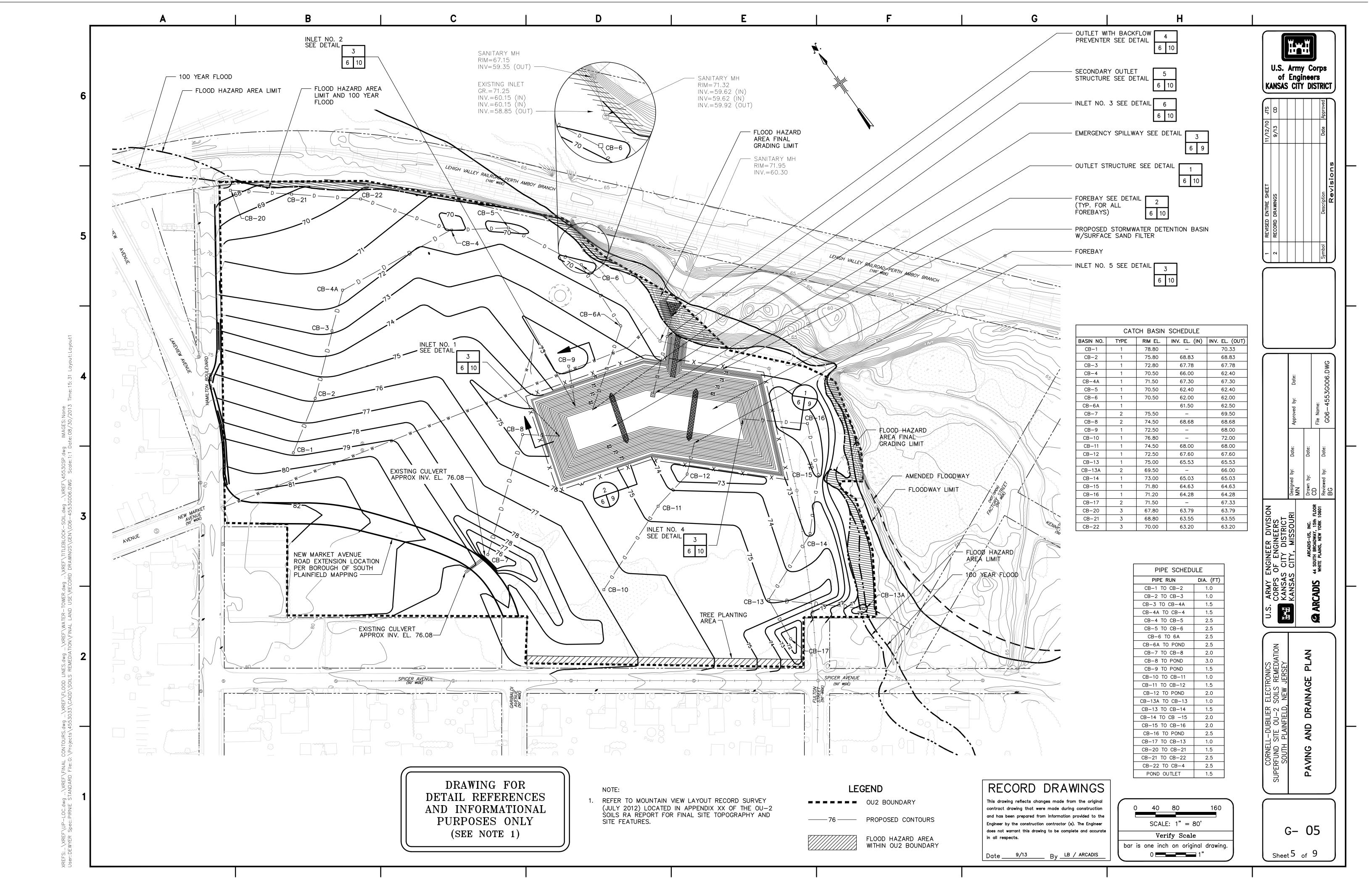
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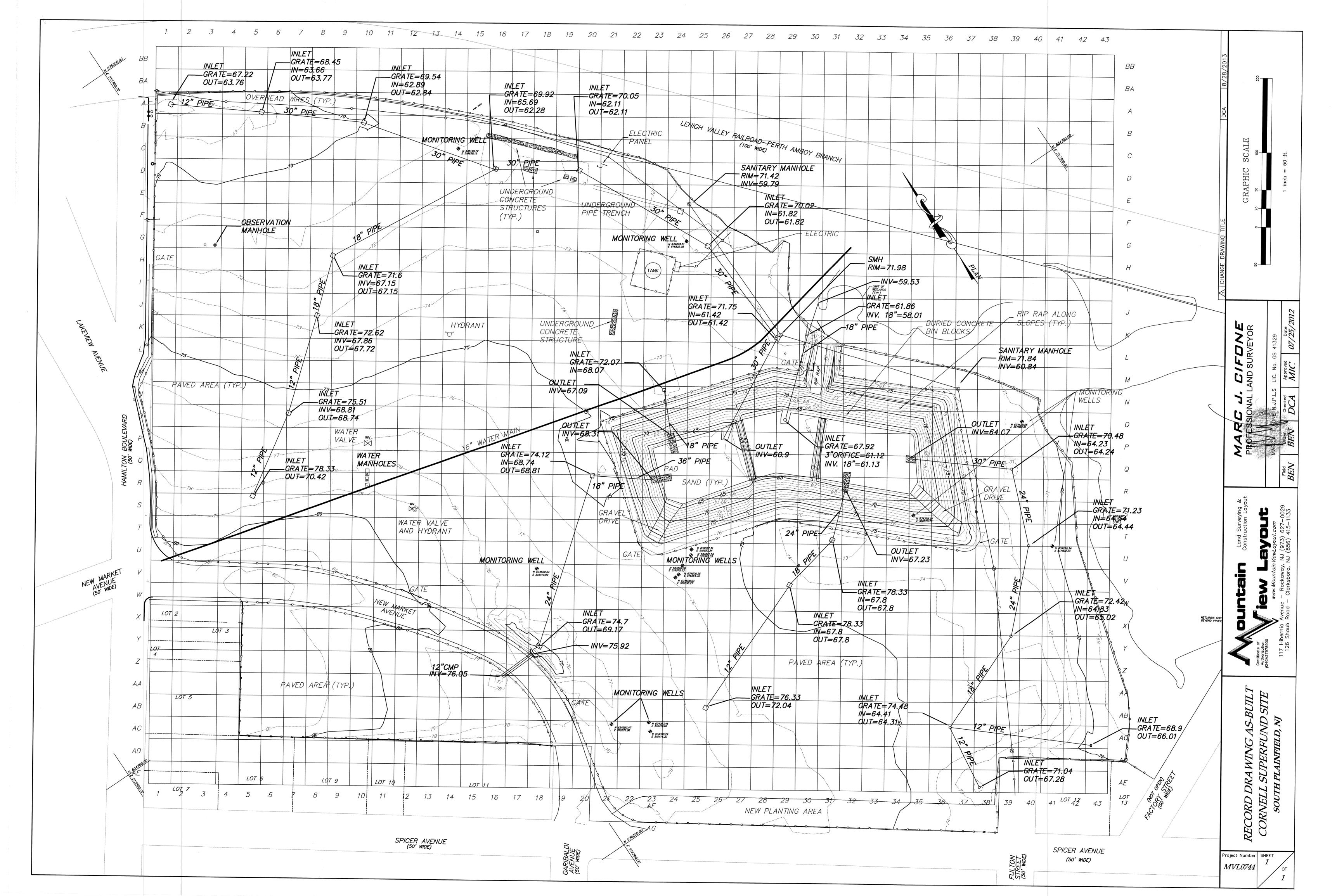
Sheet <sup>1</sup> of

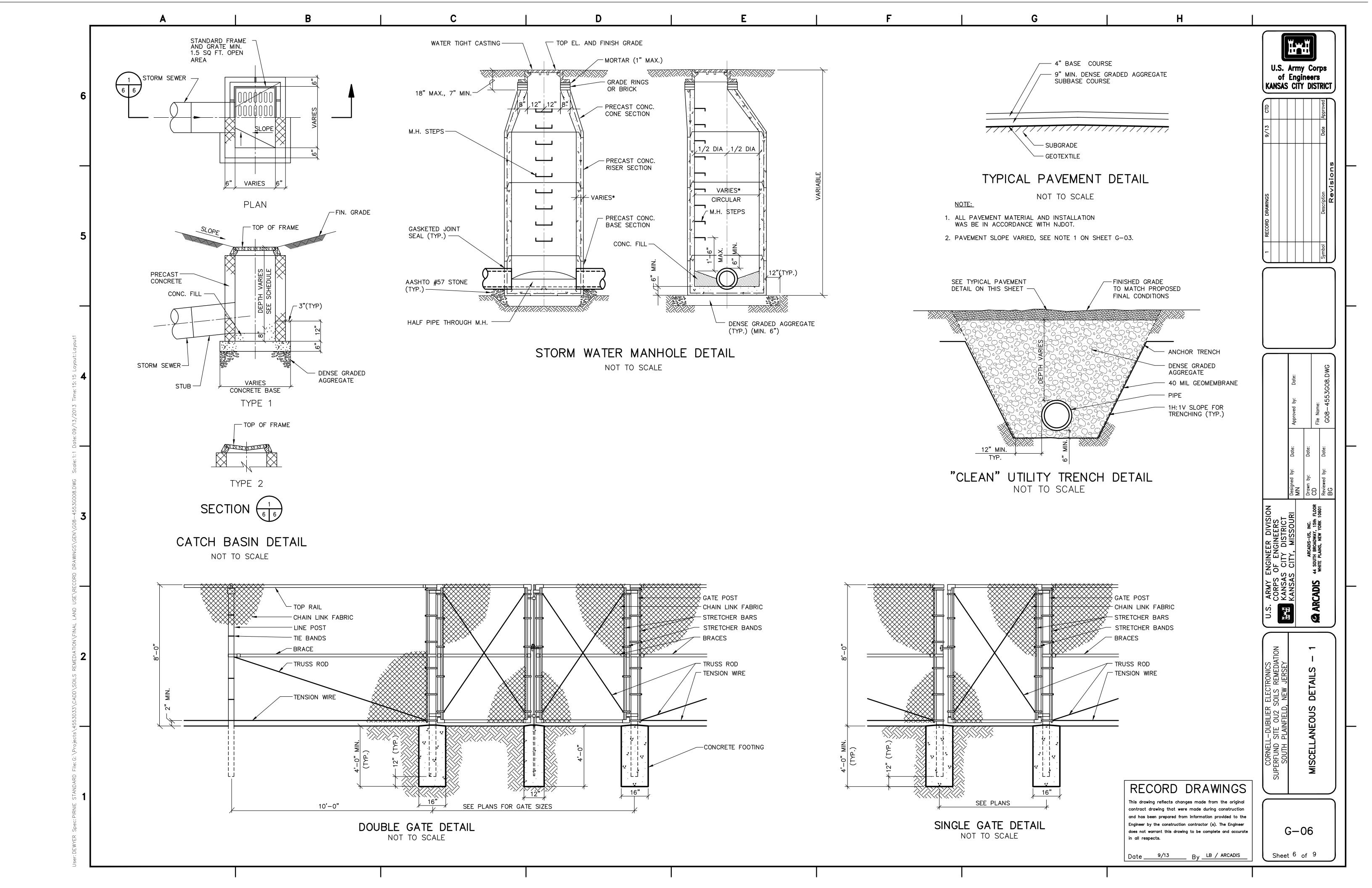


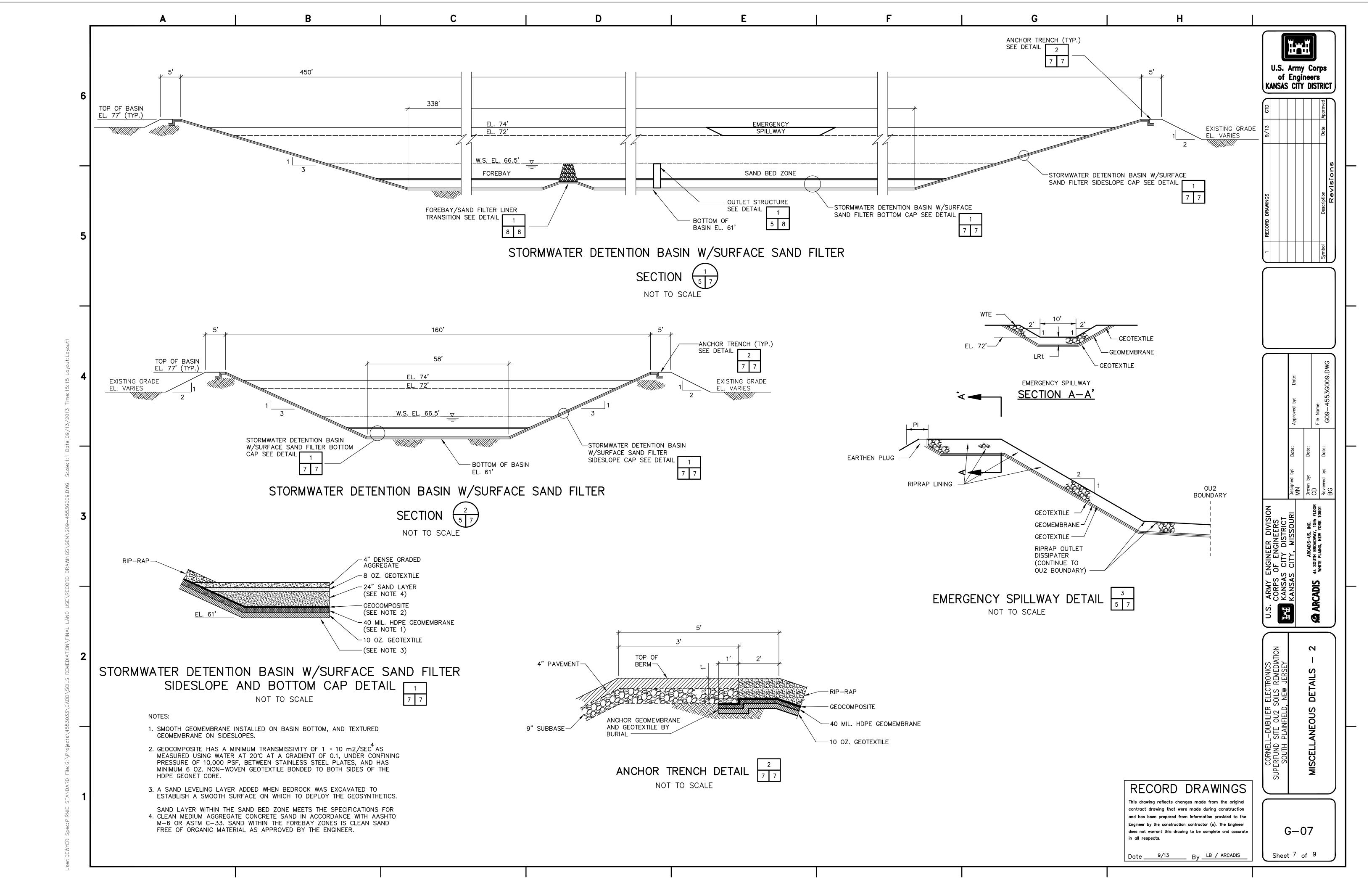


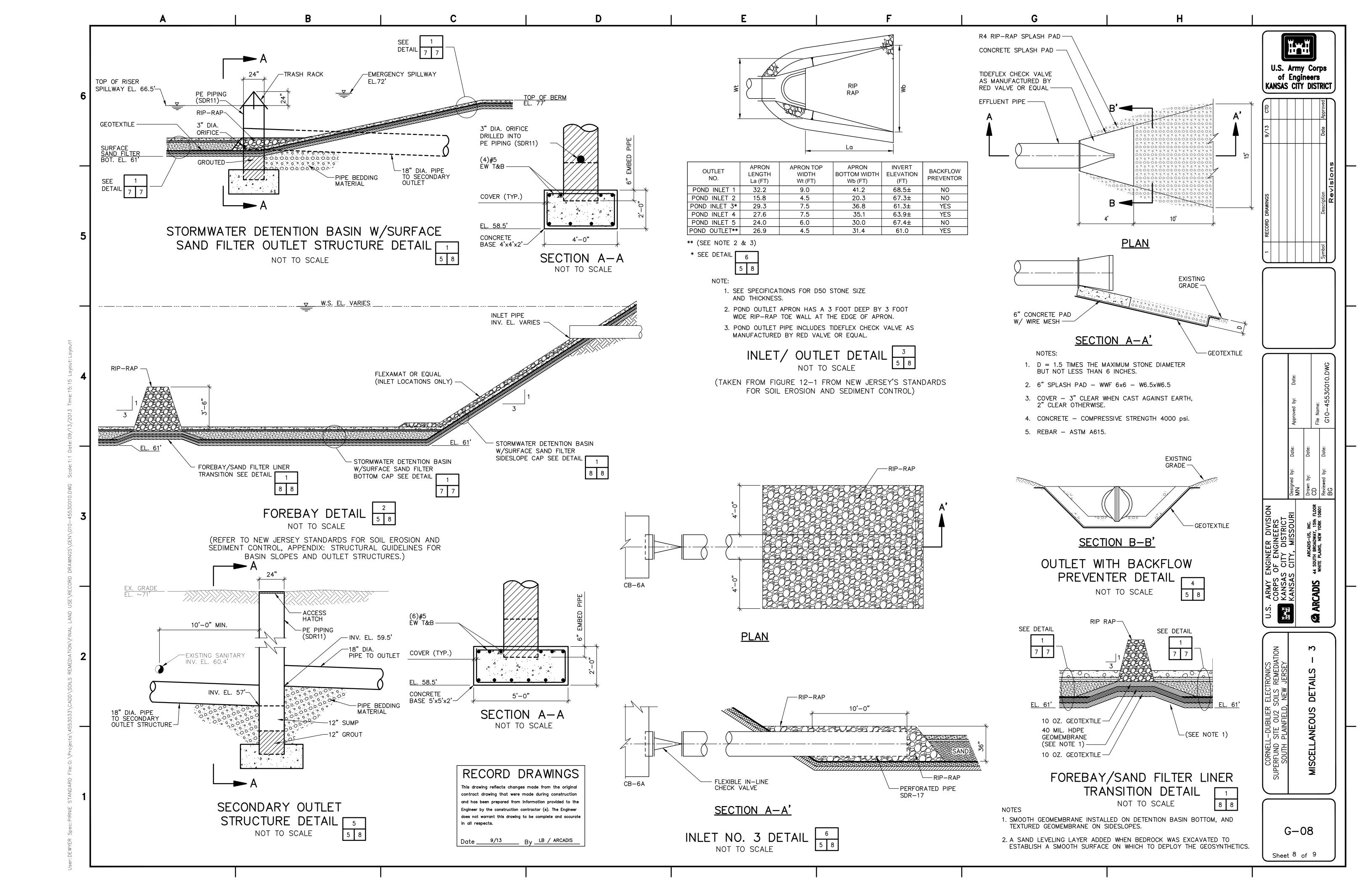


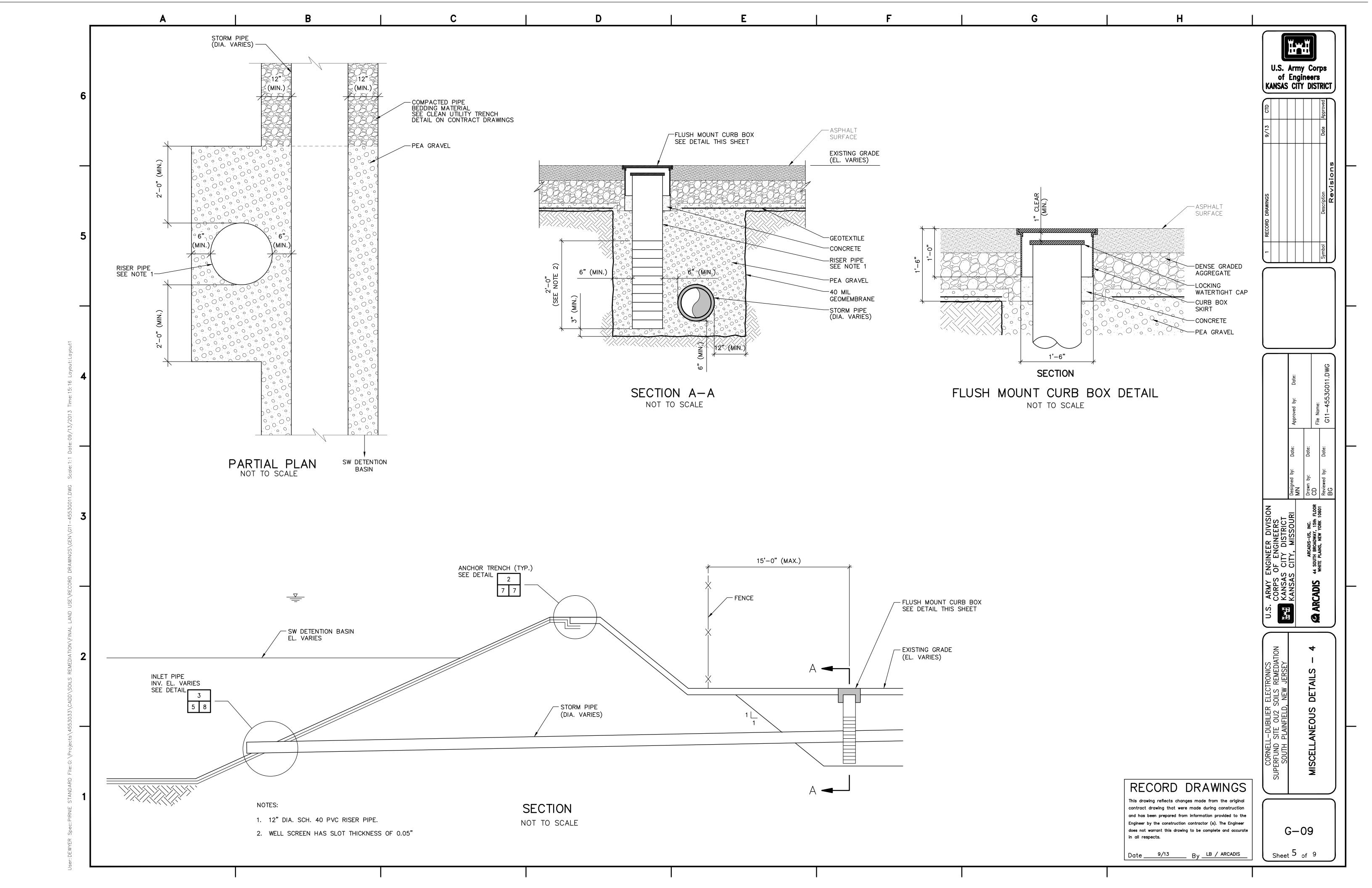


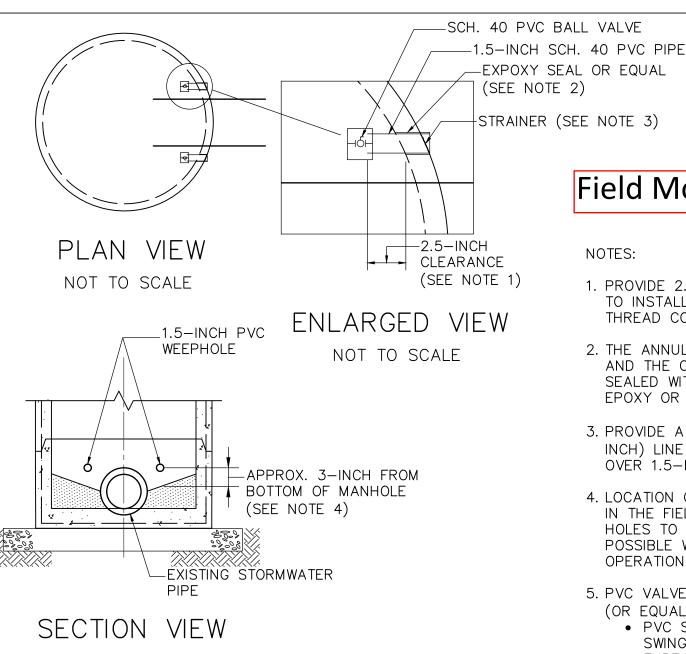












NOT TO SCALE

#### U.S. ARMY ENGINEER DIVISION CORPS OF ENGINEERS KANSAS CITY DISTRICT KANSAS CITY, MISSOURI Designed by JTŠ Drawn by: KBI MALCOLM PIRNIE, INC. 44 SOUTH BROADWAY WHITE PLAINS, NY 10601 Reviewed by: Revisions

06/26/13

06/26/13

06/26/13 File Name:

Field Modification CB-12

#### NOTES:

- 1. PROVIDE 2.5-INCH CLEARANCE OF PVC PIPE TO INSTALL 1.5-INCH PVC SLIP BALL VALVE. THREAD CONNECTION PREFERRED.
- 2. THE ANNULAR SPACE BETWEEN THE PVC PIPE AND THE CONCRETE MANHOLE SHALL BE SEALED WITH POLYURETHANE INJECTION GROUT. EPOXY OR EQUAL TO PROVIDE A TIGHT SEAL.
- 3. PROVIDE A STAINLESS STEEL 30 MESH (1/64th INCH) LINE STRAINER OR EQUAL THAT FITS OVER 1.5-INCH NPT.
- 4. LOCATION OF WEEP HOLES TO BE COORDINATED IN THE FIELD WITH THE ENGINEER. WEEP HOLES TO BE SITUATED AT LOWEST ELEVATION POSSIBLE WHILE ALLOWING SUFFICIENT OPERATION OF THE BALL VALVE.
- 5. PVC VALVE TO CONSIST OF THE FOLLOWING (OR EQUAL):
  - PVC SCHEDULE 40 BALL VALVE WITH SWING HANDLE
  - THREADED CONNECTION

CORNELL-DUBILIER ELECTRONICS SUPERFUND SITE OU-2 SOILS REMEDIATION SOUTH PLAINFIELD, NEW JERSEY

BALL VALVE INSTALLATION PLAN

FIGURE 1

# **APPENDIX C**

## U.S. Army Corps of Engineers

**Kansas City District** 



## Design Analysis Report Operable Unit 2 – Storm Water Management

Cornell-Dubilier Electronics Superfund Site South Plainfield, NJ

USACE Contract No. W912DQ-06-D-0006 Task Order No. 0001

June 2008

Prepared by: Malcolm Pirnie, Inc. 1900 Polaris Parkway, Suite 200 Columbus, OH 43240



### Contents

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#### **Attachments**

- A. PondPack Model Output
- B. Pre-Development Calculations
- C. Catch Basin Calculations
- D. Piping Calculations
- E. Conduit Outlet Protection Calculations

### 1. Introduction

The former Cornell – Dublier Electronics property is currently a superfund site in South Plainfield, NJ. The site has been contaminated due to the improper disposal of manufactured components on site. The contaminated soil will be decontaminated and once the remediation is completed the entire site will be paved. Malcolm Pirnie has designed a storm water system to convey water from the site without impacting the surrounding area. The design of the system follows the State of New Jersey's Standards for Soil Erosion and Sediment Control as well as the New Jersey Storm Water Best Management Practices Manual.

#### 1.1. Extended Detention Basin Location

One of the main issues to be resolved with this project was the location of an extended detention basin (basin). There were many factors for determining the location of the basin. Originally it was determined that the basin would not be placed over areas of contaminated soil. The natural grading of the site slopes to the north and to the east toward the Bound Brook so the optimal place to locate the basin is along the northern boundary of the site. Several potential basin locations were evaluated south of the rail system and to the west of the major contaminated areas. Areas in the northwest corner of the site contained uncontaminated areas but the bedrock was very shallow. Because of the shallowness of the bedrock in these locations, it was apparent that avoiding excavation in contaminated areas would necessitate significant rock removal. It was determined that the basin would be placed in the northeast corner of the site, where bedrock was deepest. The location is in a contaminated area but it was deemed the best choice in terms of constructability. The location of the basin can be viewed on sheet G-

2. Storm Water System Design

The "New Jersey Storm Water Best Management Practices Manual (NJSBMPM), was

used for the development of the storm water system. The storm water system consists of

catch basins, piping, an extended detention basin, and a basin outlet structure.

All the various components of the developed storm water system were placed into a

computer model called PondPack. Sub-areas of storm water collection to each catch

basin were input into the model as well as sheet flow and channel flow lengths. Piping

reach distances and estimated pipe sizes as well as the size and volume of the basin were

also input into the model. The initial basin volume was estimated using an input

hydrograph generated for the preliminarily storm water network following the TR-55

tabular hydrograph method. Note catch basins CB-23 and CB-24 are not included in the

model because they are solely designed to assist in cleaning the system and have no

storm water responsibility.

The model was conducted using the 2, 5, 10, 25, 50, 100 year – 24 hr type III storm

events. From the NJSBMPM, the outlet structure for the basin was designed so post-

development peak runoff rates for the 2, 10 and 100 year storm events were 50, 75, and

80 percent, respectively, of the pre-development peak runoff rates. Output from the

PondPack model is attached in attachment A.

Pre-development runoff rates were calculated using the Rational Method and assuming

conservatively that the pre-developed area was entirely grass. Pre-development

calculations are attached in attachment B. Equations for the pre-development

calculations were taken from the PondPack user's guide. Table 2.1 shows the flows that

will have to be matched for the post-construction outlet structure.

**Table 2.1** 

Storm Frequency	Pre-Development Peak Runoff (Q <sub>i</sub> )	Post-Development Peak Design Flow (Q <sub>i</sub> )		
2–yr	20.2 cfs	10.1 cfs		
10–yr	27.4 cfs	20.5 cfs		
100–yr	37.8 cfs	30.2 cfs		

#### 2.1. Storm Water System Design Components

#### 2.1.1. Catch Basins

A total of 22 catch basins were used for the storm water configuration at the site. A catch basin detail is presented on sheet G-14. There are three types of catch basins which will be constructed. The first type is a general catch basin which will be constructed level in low areas of the site. The second type of catch basin contains angled grates. The catch basin grate is angled to ensure that flow along the low area troughs in the pavement will enter the catch basin. If the catch basin were constructed flat the flow may concentrate along the sides of the drain and flow past the catch basin. The third type of catch basin is a general curb inlet which will be installed in some perimeter areas of the site. A catch basin network schedule is located on sheet G-12. Catch basin network calculations are attached in Attachment C

#### **2.1.2.** Piping

The piping for the storm water collection system is HDPE pipe. Because it is fusion welded, HDPE pipe is virtually leak proof, therefore mitigating the potential for infiltration of surface water into contaminated soils. Invert elevations for piping entering and leaving catch basins can be found on the catch basin schedule. Catch basin network piping invert elevations were determined from the assumption that at least 1 foot of pipe

cover material and 1.25 feet of pavement material would need to be installed over the pipe.

The sizing of the storm water pipe was accomplished by using the 25-year design flow (cfs) from the PondPack output along with user-defined pipe geometries. A manning's (n) roughness number of 0.009 was used for solid HDPE pipe. Calculated flow relationships were determined assuming a ¾ full pipe. From the calculation a final design diameter for the piping was obtained. A spreadsheet summarizing the piping calculations is attached in Attachment D. A pipe size schedule is located on sheet G-12.

#### 2.1.3. Extended Detention Basin

The basin has 3:1 inner side slopes, a 5-foot bench on top of the berm and 2:1 side slopes to grade on the perimeter. The basin will be lined with 40-mil LLDPE flexible membrane liner to mitigate potential infiltration of surface water into contaminated soils. The basin will also have a 12-inch sand layer and two sets of geotextiles (one as a cushion layer to protect the geomembrane, the other as an indicator layer to prevent accidental damage during maintenance activities. A detail of the cross-section of the basin with layer details is on sheet G-16.

#### 2.1.3.1. Basin Outlet Structure

The basin outlet structure consists of a multi-stage structure that includes a low-flow orifice and a standpipe overflow or "primary discharge." The low-flow orifice is a 6-inch diameter opening at the base of the standpipe. The standpipe itself will be a 5-foot high (elevation 65.0), 18-inch diameter HDPE riser equipped with a trash rack to prevent clogging. The low-flow orifice controls the 2-yr storm event and the primary spillway controls the 10-yr / 24-hr storm event. An emergency spillway is located 8 feet above the bottom of the basin (elevation 68.0). The emergency spillway elevation is set above the 100-yr / 24-hr storm event elevation. The outlet structure meets the "Standards for Soil Erosion and Sediment Control in New Jersey, Appendix A10 - Modified Structural Guidelines for Storm Water Management Basins". A basin outlet detail can be found on

Sheet 17. Output from the PondPack model based on the basin outlet structures is summarized in Table 2-2 below.

**Table 2.2** 

Storm Frequency	Post-Development Peak Required Flow (Q <sub>i</sub> )	Model Output Flow
2–yr	10.1 cfs	3.5 cfs
10–yr	20.5 cfs	13.1 cfs
100–yr	30.2 cfs	27.6 cfs

#### 2.1.3.2. Basin Forebays

In the NJSBMPM it states that a 24-hour detention time is required in an extended detention basin in order to achieve a 60 percent TSS removal rate. In order to achieve this percentage, a minimum of 10 percent of the storm water quality design storm runoff volume must be retained for the 24-hour time period. In order to retain the water quality volume from a storm event forebays will be constructed at the conduit inlets of the basin for sediment removal. The definition of a storm water quality design storm is a 2-hr storm with a rain total of 1.25 inches. The volume of the quality design storm was determined from the PondPack data and used in the design of the forebays. The forebays will be constructed with a retained volume greater than 10 percent of the quality design storm volume.

#### 2.1.3.3. Conduit Outlet Protection

Pipe inlets to the basin are required to have conduit outlet protection. From the Standards for Soil Erosion and Sediment Control in New Jersey Manual, the need for conduit protection is determined by comparing the allowable velocity for the soil onto which the conduit is discharging to the velocity in the conduit. Because the velocities from the inlet



pipes to the basin are greater than the velocity allowable for sand, outlet protection is needed. Calculations for the required size of the outlet protection apron and riprap size requirements as well as equations used from section 12-1 of the Sediment Control Manual are attached as Attachment E.

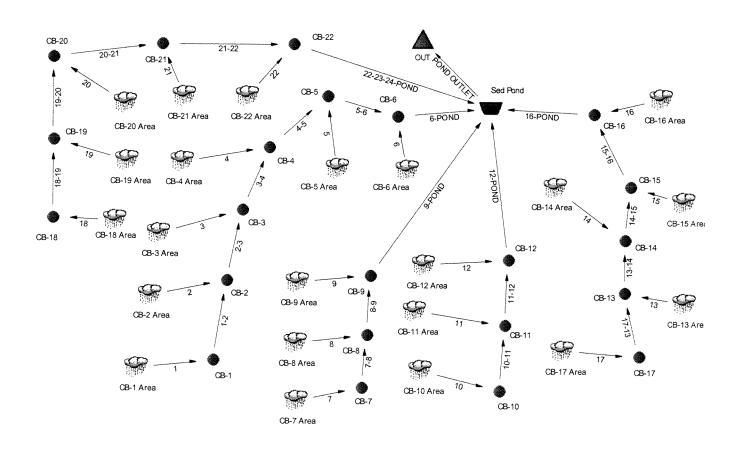
3. Summary

Malcolm Pirnie Inc designed a storm water system for the former Cornell-Dublier electronics manufacturing site that meets the requirements for the State of New Jersey's Standards for Soil Erosion and Sediment Control as well as the New Jersey Storm water Best Management Practices Manual.

Pre-development flows from the site were calculated and used to determine the required flows for post-development conditions. Storm water components were developed and input into a storm water modeling system and an outlet structure was designed to meet the required post-development flows. Storm water modeling was performed using PondPack and the modeling output meets the requirements for post-development.

Storm water piping was sized based on calculated flows from the modeling system. Velocities in the pipes are sufficient to move any grit or debris caught in the system. Conduit outlet protection calculations were performed to determine the required dimensions of riprap aprons to be used at pipe outlets. It has been recommended that HDPE Pipe and LLDPE flexible membrane liner be used in the storm water system to mitigate the potential for infiltration of surface water into contaminated soils.

Attachment A
PondPack Model Output



Job File: F:\Gearing\Superfund Site OU-2\PondPack\CORNELL-DUBILIER 95%.PPW

Rain Dir: F:\Gearing\Superfund Site OU-2\PondPack\

### JOB TITLE

Project Date: 2/25/2008 Project Engineer: Gearing Project Title: Cornell-Dubilier

Project Comments:

Superfund Site OU-2 Soils Remediation

Page 2.01

Name.... Watershed

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#### MASTER DESIGN STORM SUMMARY

Network Storm Collection: South Plainfield

Return Event	Total Depth in	Rainfall Type	RNF ID
Dev 2	3.3000	Synthetic Curve	TypeIII 24hr
Dev 5	4.3000	Synthetic Curve	TypeIII 24hr
Dev 10	5.2000	Synthetic Curve	TypeIII 24hr
Dev 25	5.9000	Synthetic Curve	TypeIII 24hr
Dev 50	6.4000	Synthetic Curve	TypeIII 24hr
Dev100	7.5000	Synthetic Curve	TypeIII 24hr
Dev 1	1.2500	Time-Depth Curve	Gauged Rain

### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Туре	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
CB-1 CB-1 CB-1 CB-1 CB-1	JCT JCT JCT JCT JCT JCT	2 5 10 25 50 100	.235 .312 .380 .434 .472		12.0960 12.0900 12.0930 12.0960 12.0960 12.0960	2.53 3.31 4.01 4.56 4.95 5.80		
CB-1  CB-1 AREA  CB-1 AREA  CB-1 AREA  CB-1 AREA  CB-1 AREA  CB-1 AREA  CB-1 AREA	JCT AREA AREA AREA AREA AREA AREA AREA	1 2 5 10 25 50 100	.079 .235 .312 .380 .434 .472 .557		1.0860 12.0960 12.0900 12.0930 12.0960 12.0960 12.0960 1.0860	2.75 2.53 3.31 4.01 4.56 4.95 5.80 2.75		

Name.... Watershed

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Page 2.02

### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Туре	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
CB-10	JCT	2	.110		12.0840	1.18		
CB-10		5	.146		12.0930	1.55		
CB-10	002	10	.178		12.0930	1.88		
CB-10	JCT	25	.203		12.0900	2.13		
CB-10	JCT	50 100	.221		12.0930	2.31		
CB-10		100	.260		12.0960	2.71		
CB-10	JCT	1	.037		1.0830	1.28		
CB-10 AREA	AREA	2	.110		12.0840	1.18		
CB-10 AREA		5	.146		12.0930	1.55		
CB-10 AREA	AREA		.178		12.0930	1.88		
CB-10 AREA	AREA		.203		12.0900			
CB-10 AREA	AREA		.221		12.0930			
CB-10 AREA		100	.260		12.0960			
CB-10 AREA	AREA	1	.037		1.0830	1.28		
CB-11		2						
CB-11		5	.440		12.0930	4.68		
CB-11	JCT	10	.538		12.0930	5.67		
CB-11	JCT	25	.613		12.0930	6.44		
CB-11	JCT	50	.667		12.0930	6.99		
CB-11		100	.786		12.0930			
CB-11	JCT	1	.112		1.0890	3.88		
CB-11 AREA	AREA	2	.222		12.0960	2.39		
CB-11 AREA	AREA	5	.295		12.0900	3.13		
CB-11 AREA	AREA	10	.360		12.0930	3.80		
CB-11 AREA	AREA		.410		12.0900	4.31		
CB-11 AREA	AREA		.447		12.0960	4.68		
CB-11 AREA		100	.526		12.0960	5.49		
CB-11 AREA	AREA	1	.075		1.0860	2.60		

Type.... Master Network Summary Page 2.03

Name.... Watershed

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Туре	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max Pond Storage ac-ft
CB-12	JCT	2	.948		12.0960	10.20	 THE THE SAME AND SAME AND SAME WITH THE THE PART WHEN THE THE
CB-12		5	1.256		12.0960	13.35	
CB-12	JCT	10	1.534		12.0960	16.18	
CB-12	JCT	25	1.750		12.0960	18.38	
CB-12			1.905		12.0960	19.95	
CB-12	JCT	50 100	2.244		12.0960	23.40	
CB-12		1	.320		1.0890	11.06	
Cor Andre Sala Sala	001	_	.520		1.0090	11.00	
CB-12 AREA	AREA	2	.616		12,0930	6.63	
CB-12 AREA	AREA		.816		12.0930	8.68	
CB-12 AREA	AREA		.997		12.0930	10.51	
CB-12 AREA	AREA	25	1.137		12.0930	11.94	
CB-12 AREA	AREA	50	1.237		12.0930	12.96	
CB-12 AREA	AREA	100	1.458		12.0930		
CB-12 AREA	AREA	1	.208		1.0890	7.20	
CB-13		2			12.0930	1.21	
CB-13	JCT	5	.149		12.0960	1.58	
CB-13	JCT	10	.182		12.0930	1.92	
CB-13	JCT	25	.208		12.0930	2.18	
CB-13	JCT	50	.226		12.0960	2.37	
CB-13		100	.266		12.0900		
CB-13	JCT	1	.038		1.0860	1.31	
CD 12 NDES	אי ודו כע מד	0	07.4		10 0040		
CB-13 AREA	AREA	2	.074		12.0840	.80	
CB-13 AREA	AREA	5	.098		12.0840	1.04	
CB-13 AREA CB-13 AREA	AREA	10	.120		12.0840	1.26	
	AREA	25	.137		12.0930	1.44	
CB-13 AREA	AREA		.149		12.0930	1.56	
CB-13 AREA	AREA	100	.175		12.0960	1.83	
CB-13 AREA	AREA	1	.025		1.0890	.87	

Page 2.04

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Туре	Event	HYG Vol ac-ft	Trun		Qpeak cfs	Max Pond Storage ac-ft
CB-14	JCT	2	.355		12.0960		
CB-14	JCT	5	.471		12.0930		
CB-14	JCT	10	.575		12.0930		
CB-14	JCT	25	.656			6.88	
CB-14	JCT	50	.714			7.47	
CB-14		100			12.0930	8.76	
CB-14	JCT	1	.120		1.0890	4.14	
CB-14 AREA		2	.243		12.0930	2.61	
	AREA		.322		12.0900	3.42	
CB-14 AREA	AREA		.393		12.0930	4.14	
CB-14 AREA		25	.448		12.0960	4.71	
CB-14 AREA		50	.488		12.0960	5.11	
CB-14 AREA		100			12.0930	5.99	
CB-14 AREA	AREA	1	.082		1.0860	2.84	
CB-15					12.0960		
CB-15		5			12.0960	8.53	
CB-15	JCT	10	.980		12.0960	10.33	
CB-15	JCT	25	1.118		12.0960	11.74	
CB-15	JCT	50	1.217		12.0960	12.74	
CB-15		100	1.434		12.0960	14.94	
CB-15	JCT	1	.204		1.0890	7.06	
CB-15 AREA	AREA	2	.250		12.0930	2.70	
CB-15 AREA	AREA		.332		12.0900	3.53	
CB-15 AREA	AREA	10	.405		12.0900	4.27	
CB-15 AREA	AREA	25	.462		12.0960	4.86	
CB-15 AREA	AREA	50	.503		12.0960	5.27	
CB-15 AREA	AREA	100	.593		12.0930	6.18	
CB-15 AREA	AREA	1	.084			2.93	

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID		Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
CB-16	JCT	2	.882		12.0960	9.47		
CB-16	JCT	5	1.168		12.0960	12.40		
CB-16	JCT	10	1.427		12.0960	15.04		
CB-16	JCT	25	1.628		12.0960	17.08		
CB-16	JCT	50	1.771		12.0960			
CB-16		100	2.087		12.0960	21.74		
CB-16	JCT	1	.297		1.0890	10.25		
CB-16 AREA		2	.276		12.0900	2.97		
CB-16 AREA	AREA	5	.366		12.0930	3.89		
CB-16 AREA	AREA		.447		12.0960	4.71		
CB-16 AREA	AREA	25	.510		12.0960	5.35		
CB-16 AREA	AREA	50	.554		12.0930	5.81		
CB-16 AREA	AREA	100	.653		12.0930	6.81		
CB-16 AREA	AREA	1	.093		1.0860	3.22		
CB-17	JCT	2	.038		12.0690	.41		
CB-17	JCT	5	.051		12.0780	.54		
CB-17		10	.062		12.0750	.65		
CB-17	JCT	25	.071		12.0780	.74		
CB-17	JCT	50	.077		12.0900	.81		
CB-17	JCT	100	.091		12.0870	.95		
CB-17	JCT	1	.013		1.0800	. 45		
CB-17 AREA	AREA	2	.038		12.0690	.41		
CB-17 AREA	AREA	5	.051		12.0780	.54		
CB-17 AREA	AREA	10	.062		12.0750	.65		
CB-17 AREA		25	.071		12.0780	.74		
CB-17 AREA	AREA	50	.077		12.0900	.81		
CB-17 AREA	AREA	100	.091		12.0870	.95		
CB-17 AREA	AREA	1	.013		1.0800	.45		

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

							Max Pond Storage
Node ID	Туре		ac-ft	hrs	cfs	ft	ac-ft
CB-18	JCT	2	.043	12.0900	.47		
CB-18		5	.058	12.0750	.61		
CB-18	JCT	10	.070	12.0930	.74		
CB-18	JCT	25	.080	12.0840	.84		
CB-18	JCT	50 100	.087	12.0870	.91		
CB-18	JCT	100	.103	12.0840	1.07		
CB-18	JCT	1	.015	1.0830	.51		
CB-18 AREA	AREA		.043	12.0900	.47		
CB-18 AREA	AREA		.058	12.0750	.61		
CB-18 AREA	AREA		.070	12.0930	.74		
CB-18 AREA	AREA		.080	12.0840	.84		
CB-18 AREA	AREA	50	.087	12.0870	.91		
CB-18 AREA		100	.103	12.0840	1.07		
CB-18 AREA	AREA	1	.015	1.0830	.51		
CB-19		2	.112	12.0930	1.21		
CB-19	JCT	5	.149	12.0930	1.58		
CB-19		10	.182	12.0930	1.92		
CB-19	JCT	25	.208	12.0960	2.18		
CB-19	JCT	50	.226	12.0960	2.37		
CB-19		100	.266	12.0930			
CB-19	JCT	1	.038	1.0860	1.31		
CB-19 AREA	AREA		.069	12.0900	.74		
CB-19 AREA	AREA	5	.091	12.0810	.97		
CB-19 AREA	AREA	10	.112	12.0840	1.18		
CB-19 AREA	AREA		.127	12.0840	1.34		
CB-19 AREA	AREA		.139	12.0930	1.45		
CB-19 AREA	AREA		.163	12.0870	1.70		
CB-19 AREA	AREA	1	.023	1.0860	.81		

Type.... Master Network Summary Page 2.07

Name.... Watershed

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Туре 	Event	HYG Vol ac-ft	Trun	Qpeak hrs		Max Pond Storage ac-ft
CB-2	JCT	2	.483		12.0930	5.19	
CB-2	JCT	5	.640		12.0930	6.80	
CB-2		10	.782		12.0930	8.24	
CB-2	JCT	25	.892		12.0930	9.36	
CB-2	JCT	50	.970		12.0960		
CB-2	JCT	100	1.143		12.0960	11.92	
CB-2	JCT	1	.163		1.0890	5.64	
CB-2 AREA		2	.248		12.0930	2.67	
CB-2 AREA	AREA	5	.329		12.0900	3.49	
CB-2 AREA	AREA	10	.401		12.0900	4.23	
CB-2 AREA	AREA	25	.458		12.0960	4.81	
CB-2 AREA	AREA	50	.498		12.0960	5.22	
CB-2 AREA	AREA	100	.587		12.0930	6.12	
CB-2 AREA	AREA	1	.084		1.0860	2.90	
CB-20		2	.442		12.0930	4.75	
CB-20	JCT	5	.586		12.0930	6.22	
CB-20		10	.715		12.0930	7.54	
CB-20	JCT	25	.816		12.0930	8.57	
CB-20	JCT	50	.888		12.0930	9.30	
CB-20	JCT	100	1.047		12.0930	10.91	
CB-20	JCT	1	.149		1.0890	5.15	
CB-20 AREA	AREA	2	.330		12.0930	3.55	
CB-20 AREA	AREA	5	.437		12.0960	4.64	
CB-20 AREA	AREA	10	.533		12.0930	5.63	
CB-20 AREA	AREA	25	.609		12.0930	6.39	
CB-20 AREA	AREA	50	.662		12.0930	6.94	
CB-20 AREA	AREA	100	.780		12.0930	8.14	
CB-20 AREA	AREA	1	.111		1.0860	3.85	

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Name.... Watershed

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Type		HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max Pond Storage ac-ft
CB-21	JCT	2	.524		12.0960	5.63	
CB-21	JCT	5	.694		12.0960	7.37	
CB-21		10	.848		12.0960	8.94	
CB-21	JCT		.967		12.0960		
CB-21	JCT	50	1.052		12.0960	11.02	
CB-21		100	1.240		12.0960		
CB-21	JCT	1	.177		1.0890	6.10	
CB-21 AREA		2	.082		12.0870	.88	
	AREA		.108		12.0810	1.15	
CB-21 AREA	AREA	10	.132		12.0930	1.40	
CB-21 AREA	AREA		.151		12.0930	1.59	
CB-21 AREA	AREA	50	.164		12.0840	1.72	
CB-21 AREA		100	.194			2.02	
CB-21 AREA	AREA	1	.028		1.0830	.95	
CB-22		2	.583			6.26	
CB-22		5	.772		12.1020		
CB-22		10	.943	R	12.0990	9.94	
CB-22	JCT	25	1.076	R	12.0990	11.29	
CB-22	JCT	25 50 100	1.171	R	12.0990	12.25	
CB-22	001	200		R	12.0990	14.37	
CB-22	JCT	1	.197		1.0920	6.77	
CB-22 AREA	AREA	2	.059		12.0750	.63	
CB-22 AREA	AREA	5	.078		12.0840	.83	
CB-22 AREA		10	.095		12.0900	1.00	
CB-22 AREA	AREA	25	.109		12.0810	1.14	
CB-22 AREA	AREA	50	.118		12.0840	1.24	
		100	.139		12.0930	1.45	
CB-22 AREA	AREA	1	.020		1.0800	.68	

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Name.... Watershed

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID		Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
CB-3	JCT	2	.733	R	12.0960	7.88		
CB-3		5	.972	R	12.0960	10.32		
CB-3	JCT	10	1.187	R	12.0960	12.51		
CB-3	JCT	25	1.354	R	12.0960	14.21		
CB-3	JCT	50	1.473	R	12.0960	15.43		
CB-3	JCT		1.736	R	12.0960	18.09		
CB-3	JCT	1	.247		1.0890	8.54		
CB-3 AREA	AREA	2	.250		12.0930	2.70		
CB-3 AREA	AREA	5	.332		12.0900	3.53		
CB-3 AREA	AREA	10	.405		12.0900	4.27		
CB-3 AREA	AREA	25	.462		12.0960	4.86		
CB-3 AREA	AREA	50	.503		12.0960	5.27		
CB-3 AREA	AREA	100	.593		12.0930	6.18		
CB-3 AREA	AREA	1	.084		1.0860	2.93		
CB-4	JCT	2	1.237		12.0960	13.29		
CB-4	JCT	5	1.639		12.0960	17.40		
CB-4	JCT	10	2.001		12.0960	21.09		
CB-4	JCT	25	2.283		12.0960	23.96		
CB-4	JCT	50	2.485		12.0960	26.01		
CB-4	JCT	100	2.928		12.0960	30.51		
CB-4	JCT	1	.417		1.0890	14.39		
CB-4 AREA	AREA	2	.503		12.0930	5.42		
CB-4 AREA	AREA	5	.667		12.0930	7.09		
CB-4 AREA	AREA	10	.815		12.0930	8.59		
CB-4 AREA	AREA	25	.929		12.0930	9.76		
CB-4 AREA	AREA	50	1.011		12.0930	10.60		
CB-4 AREA	AREA	100	1.192		12.0930	12.43		
CB-4 AREA	AREA	1	.170		1.0890	5.89		

Name.... Watershed

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Type	Return Event	HYG Vol ac-ft		Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
CB-5	JCT	2	1.390	R	12.0990	14.94		
CB-5	JCT	5	1.842	R	12.0990	19.56		
CB-5	JCT	10	2.249	R	12.0990	23.71		
CB-5	JCT	25	2.566	R	12.0990	26.93		
CB-5	JCT	50	2.793	R	12.0990	29.23		
CB-5		100	3.291	R	12.0990	34.29		
CB-5	JCT	1	.469		1.0920	16.16		
CB-5 AREA	AREA		.153		12.0960	1.65		
CB-5 AREA	AREA		.203		12.0900	2.16		
CB-5 AREA	AREA		.248		12.0960	2.62		
CB-5 AREA	AREA		.283		12.0930	2.97		
CB-5 AREA	AREA		.308		12.0900	3.23		
CB-5 AREA	AREA		.363		12.0900	3.78		
CB-5 AREA	AREA	1	.052		1.0860	1.79		
CB-6	JCT	2	1.559	R	12.1020	16.74		
CB-6	JCT	5	2.066	R	12.1020	21.92		
CB-6	JCT	10	2.522	R	12.1020	26.57		
CB-6	JCT	25	2.878	R	12.1020	30.19		
CB-6	JCT	50	3.131	R	12.1020	32.77		
CB-6	JCT	100	3.690	R	12.1020	38.43		
CB-6	JCT	1	.526		1.0950	18.09		
CB-6 AREA	AREA	2	.169		12.0930	1.82		
CB-6 AREA	AREA	5	.224		12.0930	2.38		
CB-6 AREA	AREA	10	.273		12.0930	2.88		
CB-6 AREA	AREA	25	.311		12.0900	3.27		
CB-6 AREA	AREA	50	.339		12.0900	3.55		
CB-6 AREA	AREA		.399		12.0930	4.16		
CB-6 AREA	AREA	1	.057		1.0860	1.97		

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Name.... Watershed

File....F:\Gearing\Superfund Site OU-2\PondPack\Cornell-Dubilier\_95%.ppw

### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID	Туре	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max Pond Storage ac-ft
CB-7	JCT	2	.279		12.0900	3.00	
CB-7	JCT	5	.369		12.0930	3.92	
CB-7	JCT	10	.451		12.0960	4.76	
CB-7	JCT	25	.514		12.0960	5.40	
CB-7		50	.560		12.0930	5.86	
CB-7		100	.659		12.0930	6.88	
CB-7	JCT	1	.094		1.0860	3.25	
CB-7 AREA	AREA		.279		12.0900	3.00	
CB-7 AREA	AREA	5	.369		12.0930	3.92	
CB-7 AREA	AREA	10	.451		12.0960	4.76	
CB-7 AREA	AREA	25	.514		12.0960	5.40	
CB-7 AREA	AREA	50	.560		12.0930	5.86	
CB-7 AREA	AREA	100	.659		12.0930	6.88	
CB-7 AREA	AREA	1	.094		1.0860	3.25	
CB-8	JCT	2	.580		12.0930	6.24	
CB-8	JCT	5	.769		12.0930	8.17	
CB-8	JCT	10	.939		12.0960	9.90	
CB-8	JCT	25	1.071		12.0960	11.25	
CB-8	JCT	50	1.165		12.0960	12.21	
CB-8	JCT	100	1.373		12.0960	14.32	
CB-8	JCT	1	.196		1.0890	6.77	
CB-8 AREA	AREA	2	.302		12.0900	3.24	
CB-8 AREA	AREA	5	.400		12.0960	4.25	
CB-8 AREA	AREA	10	.488		12.0960	5.15	
CB-8 AREA	AREA	25	.557		12.0930	5.85	
CB-8 AREA	AREA	50	.606		12.0930	6.35	
CB-8 AREA	AREA	100	.714		12.0930	7.44	
CB-8 AREA	AREA	1	.102		1.0860	3.52	

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Name.... Watershed

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

Node ID		Туре	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
CB-9		JCT	2	1.150		12.0960	12.37		
CB-9		JCT	5	1.524		12.0960	16.19		
CB-9		JCT	10	1.861		12.0960	19.62		
CB-9		JCT	25	2.123		12.0960	22.29		
CB-9		JCT	50	2.310		12.0960	24.19		
CB-9		JCT	100	2.722		12.0960	28.38		
CB-9		JCT	1	.388		1.0890	13.40		
CB-9 AREA		AREA	2	.570		12.0930	6.13		
CB-9 AREA		AREA	5	.755		12.0930	8.03		
CB-9 AREA		AREA	10	.922		12.0930	9.73		
CB-9 AREA		AREA	25	1.052		12.0930	11.05		
CB-9 AREA		AREA	50	1.145		12.0930	11.99		
CB-9 AREA		AREA	100	1.349		12.0930	14.07		
CB-9 AREA		AREA	1	.192		1.0890	6.66		
*OUT		JCT	2	3.338	R	13.8960	3.47		
*OUT		JCT	5	4.825	R	12.6630	9.58		
*OUT		JCT	10	6.194	R	12.5970	13.07		
*OUT		JCT	25	7.275	R	12.5910	15.10		
*OUT		JCT	50	8.055	R	12.5970	16.36		
*OUT		JCT	100	9.799	R	12.5040	27.59		
*OUT		JCT	1	.825	R	1.8570	1.80		
SED POND	IN	POND	2	5.120	R	12.1050	54.96		
SED POND	IN	POND	5	6.786		12.1020	71.97		
SED POND	IN	POND	10	8.286		12.1020	87.25		
SED POND	IN	POND	25	9.453	R	12.1020	99.12		
SED POND	IN	POND	50	10.287		12.1020	107.59		
SED POND	IN	POND	100	12.123		12.1020	126.21		
SED POND	IN	POND	1	1.727		1.0980	59.28		

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Name.... Watershed

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### MASTER NETWORK SUMMARY SCS Unit Hydrograph Method

(\*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node	e ID		Туре	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
SED	POND	OUT	POND	2	3.338	LR	13.8960	3.47	65.13	3.899
SED	POND	OUT	POND	5	4.825	LR	12.6630	9.58	65.81	4.545
SED	POND	OUT	POND	10	6.194	LR	12.5970	13.07	66.59	5.328
SED	POND	OUT	POND	25	7.274	LR	12.5910	15.10	67.21	5.971
SED	POND	OUT	POND	50	8.055	LR	12.5970	16.36	67.64	6.437
SED	POND	OUT	POND	100	9.799	LR	12.5040	27.59	68.39	7.285
SED	POND	OUT	POND	1	.825	LR	1.8570	1.80	63.90	2.816

4/4/2008

Name.... POND OUTLET

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#### \*\*\*\* COMPOSITE OUTFLOW SUMMARY \*\*\*\*

WS Elev, Total Q	Notes
Elev. Q	TW Elev Error ft +/-ft Contributing Structures
60.50 .47	Free Outfall None contributing Free Outfall 00
61.50 1.06	Free Outfall 00 Free Outfall 00
62.50 1.42	Free Outfall 00 Free Outfall 00 Free Cutfall 00
63.50 1.70	Free Outfall 00 Free Outfall 00
65.00 2.06	Free Outfall 00 Free Outfall R0 +00
66.00 10.77	Free Outfall R0 +00 Free Outfall R0 +00 Free Outfall R0 +00
	Free Outfall RO +OO
68.50 30.40	Free Outfall R0 +O0 +W0 Free Outfall R0 +O0 +W0 Free Outfall R0 +O0 +W0

Attachment B
Pre-development Calculations

# Cornell-Dublier Pre-Development-Rational Method

Subject: Use Rational Method to Estimate Flow of Surface Water for Pre-Development Conditions

- Assuming total acres that contribute to pre-development run-off = 18 acres
- Assuming all acreage is grass

#### TR-55 Sheet Flow

$$T = \frac{0.007{(nL)}^{0.8}}{{(P_2)}^{0.5} S_f^{0.4}}$$

- n	0.0 /===	1 04000	l of aroos)
n =	0.2 (good	a stanc	d of grass)
L =	160 ft		
$P_2 =$	3.3 in		
- 2	0.0 111		
$S_f =$	0.02		
Т –	<b>0.29</b> hr	or	<b>17.7</b> min
	0.23 111	Oi	17.7

n = Manning's roughness coefficient from TR-55 table

L = Flow length (ft)

P<sub>2</sub> = Two-year, 24-hour rainfall (in)

 $S_f$  = Slope (ft/ft)

#### TR-55 Shallow Concentrated Flow

Unpaved Surfaces

$$V = 16.1345 S_f^{0.5}$$

$$S_f = 0.013$$
  
V = **1.83962** fps

V = Average velocity (ft/sec.)

 $S_f$  = Slope of hydraulic grade line (ft/ft)

$$T_c = \left(\frac{L_f}{V}\right) \left(\frac{1 \,\text{hr.}}{3600 \,\text{sec.}}\right)$$

 $L_f = 720 \text{ ft}$   $T_c =$  **0.11** hr or **6.5** min

 $T_c$  = Time of concentration (hr.)

 $L_f$  = Flow length (ft)

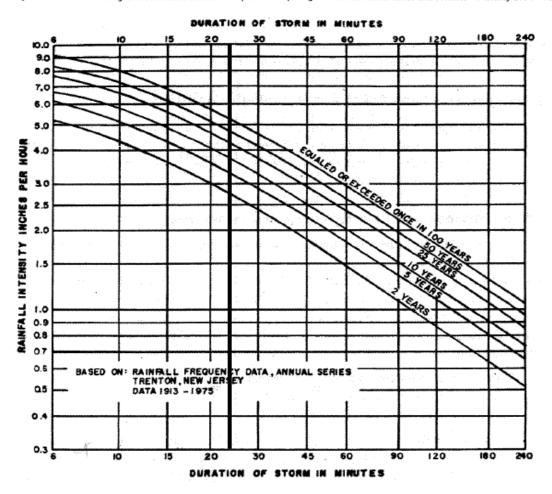
V = Average velocity (ft/sec.)

Total Tc = 24.2 min

<sup>\*</sup>Equations taken from PondPack User's Guide

# Cornell-Dublier Pre-Development-Rational Method

New Jersey Stormwater Best Management Practices Manual • Chapter 5: Computing Stormwater Runoff Rates and Volumes • February 2004 • Page 5-11



		Q 01/1
Q	=	Discharge in cubic feet per second
С	=	Coefficient of runoff

Q = CiA

I = Average rainfall intensity in inches per hour, for a given storm frequency and for a duration equal to the time of concentration.

A = Drainage area in acres

$I_{(2)} =$	2.8	in/hr
$I_{(5)} =$	3.3	in/hr
$I_{(10)} =$	3.8	in/hr
$I_{(25)} =$	4.25	in/hr
$I_{(50)} =$	4.7	in/hr
$I_{(100)} =$	5.25	in/hr

Types of Surface	Coefficient Runoff "C"	of
Pavement & paved shoulders	0.9	
Berms and slopes 4:1 or flatter	0.5	
Berms and slopes steeper than 4:1	1 0.7	
Contributing areas		
Residential (single family)	0.3-0.5	
Residential (multi-family)	0.4-0.7	
Woods	0.3	
Cultivated	0.3-0.6	

_				
C = 0.4	$Q_{(2)} =$	20.2 cfs	50%	10.1
A= 18	$Q_{(5)} =$	23.8 cfs		
	$Q_{(10)} =$	27.4 cfs	75%	20.5
	$Q_{(25)} =$	30.6 cfs		
	$Q_{(50)} =$	33.8 cfs		
	$Q_{(100)} =$	37.8 cfs	80%	30.2

Attachment C Catch Basin Calculations

## Cornell Dublier Catch Basin Network

Catch Basin Number	Rim (Elev.)	Pipe Invert In (Elev.)	Pipe Invert Out (Elev.)	Pipe Diameter D (ft)	Rim to Top of Pipe In (Depth.)	Rim to Top of Pipe Out (Depth.)	Pipe Run	Approx. Length (ft)	Slope (ft/ft)	Upslope Invert (Elev.)	Downslope Invert (Elev.)
1	76.00	70.50	70.50	1.50		4.00	1-2	160.00	0.0031	70.5000	70.0000
2	74.00	70.00	68.00	2.00	2.50	4.00	2-3	160.00	0.0031	68.0000	67.5000
3	72.00	67.50	65.80	2.00	2.50	4.20	3-4	160.00	0.0031	65.8000	65.3000
4	69.80	65.30	64.80	2.50	2.50	2.50	4-5	100.00	0.0040	64.8000	64.4000
5	69.80	64.40	64.40	2.50	2.90	2.90	5-6	180.00	0.0033	64.4000	63.8000
6	69.80	63.80	63.80	2.50	3.50	3.50	6-POND	250.00	0.0032	63.8000	63.0000
7	76.00	70.00	70.00	1.50		4.50	7-8	200.00	0.0035	70.0000	69.3000
8	73.60	69.30	69.30	2.00	2.80	2.30	8-POND	90.00	0.0367	69.3000	66.0000
9	71.50	66.00	66.50	2.00	3.50	3.00	9-POND	50.00	0.0040	66.5000	66.3000
10	76.00	70.50	70.50	1.00		4.50	10-11	200.00	0.0030	70.5000	69.9000
11	73.50	69.90	68.00	1.50	2.60	4.00	11-12	160.00	0.0044	68.0000	67.3000
12	71.50	67.30	66.60	2.50	2.70	2.40	12-POND	100.00	0.0040	66.6000	66.2000
13	74.00	68.90	68.90	1.00	2.60	4.10	13-14	140.00	0.0036	68.9000	68.4000
14	72.00	68.40	67.20	1.50	2.60	3.30	14-15	140.00	0.0036	67.2000	66.7000
15	71.00	66.70	65.50	2.00	2.80	3.50	15-16	160.00	0.0031	65.5000	65.0000
16	70.00	65.00	65.00	2.00	3.00	3.00	16-POND	110.00	0.0036	65.0000	64.6000
17	73.00	69.50	69.50	1.00		2.50	17-13	140.00	0.0043	69.5000	68.9000
18	74.50	68.90	67.50	1.00	4.60	6.00	18-19	160.00	0.0031	67.5000	67.0000
19	70.50	67.00	63.90	1.00	2.50	5.60	19-20	140.00	0.0036	63.9000	63.4000
20	67.50	63.40	63.40	1.50	3.10	2.60	20-21	120.00	0.0033	63.4000	63.0000
21	68.90	63.00	63.00	2.00	4.40	3.90	21-22	100.00	0.0030	63.0000	62.7000
22	69.80	62.70	62.70	2.00	5.10	5.10	22-23	200.00	0.0030	62.7000	62.1000
23	71.00	62.10	62.10	2.00	6.90	6.90	23-24	200.00	0.0030	62.1000	61.5000
24	71.00	61.50	61.50	2.00	7.50	7.50	24-25	140.00	0.0036	61.5000	61.0000
25	70.00	61.00	61.00	2.00	7.00	7.00	25-POND	260.00	0.0038	61.0000	60.0000

<sup>\*</sup> Assuming 1 foot of pipe cover and 1.25 feet of pavement/sub-base, need at least 2.25 ft depth from rim to top of pipe.

Attachment D
Piping Calculations

### **Pipe Calculations**

### Cornell-Dublier South Plainfield, NJ

	User-Defined Pipe Geometry					Calculated Flow Relationships										
Pipe Run	Length (ft)	Slope (ft/ft)	Upslope Invert (Elev.)	Downslope Invert (Elev.)	25 YEAR Observed/ Design Flow (cfs)	Diameter D (ft)	Flow Depth d (ft)	Depth-to- Pipe- Diameter Ratio d/D	Depth to Pipe Diameter Ratio d/D	Flow Area A (ft <sup>2</sup> )	Wetted Perimeter P (ft)	Hydraulic Radius R <sub>h</sub> (ft)	Flow Velocity v (ft/s)	Travel Time T <sub>t</sub> (min)	Calculated Flow Rate Q <sub>calc</sub> (cfs)	Observed/ Design Flow Rate Q <sub>des</sub> (cfs)
1-2	160	0.0031	70.5	70.0	4.6	1.50	1.13	0.750	0.75	1.422	3.142	0.453	5.45	0.49	7.753	4.56
2-3	160	0.0031	68.0	67.5	9.4	2.00	1.50	0.750	0.75	2.527	4.189	0.603	6.61	0.40	16.698	9.36
3-4	160	0.0031	65.8	65.3	14.2	2.00	1.50	0.750	0.75	2.527	4.189	0.603	6.61	0.40	16.698	14.21
4-5	100	0.0040	64.8	64.4	24.0	2.50	1.88	0.750	0.75	3.949	5.236	0.754	8.68	0.19	34.256	23.97
5-6	180	0.0033	64.4	63.8	26.9	2.50	1.88	0.750	0.75	3.949	5.236	0.754	7.92	0.38	31.271	26.94
6-Pond	240	0.0033	63.8	63.0	30.2	2.50	1.88	0.750	0.75	3.949	5.236	0.754	7.92	0.51	31.271	30.19
7-8	200	0.0035	70.0	69.3	5.4	1.50	1.13	0.750	0.75	1.422	3.142	0.453	5.77	0.58	8.205	5.40
8-Pond	100	0.0330	69.3	66.0	11.3	2.00	1.50	0.750	0.75	2.527	4.189	0.603	21.47	0.08	54.263	11.25
9-Pond	60	0.0033	66.5	66.3	11.1	2.00	1.50	0.750	0.75	2.527	4.189	0.603	6.82	0.15	17.246	11.10
10-11	200	0.0030	70.5	69.9	2.1	1.00	0.75	0.750	0.75	0.632	2.094	0.302	4.08	0.82	2.576	2.13
11-12	160	0.0044	68.0	67.3	6.4	1.50	1.13	0.750	0.75	1.422	3.142	0.453	6.45	0.41	9.173	6.44
12-Pond	100	0.0040	66.6	66.2	18.4	2.50	1.88	0.750	0.75	3.949	5.236	0.754	8.68	0.19	34.256	18.38
13-14	140	0.0036	68.9	68.4	2.2	1.00	0.75	0.750	0.75	0.632	2.094	0.302	4.45	0.52	2.811	2.18
14-15	140	0.0036	67.2	66.7	6.9	1.50	1.13	0.750	0.75	1.422	3.142	0.453	5.83	0.40	8.288	6.88
15-16	160	0.0031	65.5	65.0	11.7	2.00	1.50	0.750	0.75	2.527	4.189	0.603	6.61	0.40	16.698	11.74
16-Pond	100	0.0040	65.0	64.6	17.1	2.00	1.50	0.750	0.75	2.527	4.189	0.603	7.48	0.22	18.892	17.08
17-13	140	0.0043	69.5	68.9	0.7	1.00	0.75	0.750	0.75	0.632	2.094	0.302	4.87	0.48	3.079	0.74
18-19	160	0.0031	67.5	67.0	0.8	1.00	0.75	0.750	0.75	0.632	2.094	0.302	4.16	0.64	2.629	0.84
19-20	140	0.0036	63.9	63.4	2.2	1.00	0.75	0.750	0.75	0.632	2.094	0.302	4.45	0.52	2.811	2.18
20-21	120	0.0033	63.4	63.0	8.6	1.50	1.13	0.750	0.75	1.422	3.142	0.453	5.63	0.36	8.007	8.57
21-22	100	0.0030	63.0	62.7	10.2	2.00	1.50	0.750	0.75	2.527	4.189	0.603	6.47	0.26	16.361	10.15
22-23-24-25-Pond	820	0.0033	62.7	60.0	11.3	2.00	1.50	0.750	0.75	2.527	4.189	0.603	6.78	2.02	17.140	11.29

User-Defined Input Data:		
Pipe Material:	10	HDPE,
Manning's Roughness Coeff	icient, n:	0.009
Observed/Design Flow, gpm	n:	-
gal	llons/day:	-
gallo	ons/month:	-

### **Gravity Flow in Circular Pipes**

The Sheet *Flow Calcs* is a spreadsheet tabulation designed to allow the user to calculate either the flow rate through an existing circular pipe, the flow depth through a circular pipe, or the required minimum diameter of a proposed pipe.

#### **Basic User-Defined Input:**

The user must define the Manning's Coefficient and the Observed or Design Flow Rate in Cells D24 and D25 (beneath the table).

Manning's Coefficient, n: This may be directly input by the user (Cell D24) or the user may enter the number of a pipe/surface material from the chart on Sheet "Manning n" in (Cell C23) and Excel will look up the value to enter in (Cell D24).

Flow Rate: The user may enter the flow rate, in gallons/minute, in (Cell D25). Excel will calculate the flow in gallons/day and gallons/month in the cells

The user may also enter the pipe segment, length, and slope in Columns A through E of the table.

#### If the Spreadsheet is used to calculate Q or flow depth, d, of an existing pipe:

Enter the *pipe diameter*, **D**, in Column F.

The *Flow Depth*, *d*, (Column G) may be determined in one of two ways:

- 1. The user may enter this in an iterative fashion until the calculated Flow Rate,  $Q_{calc}$ , approximately equals the design Flow Rate,  $Q_{des}$ .
- 2. The user may use the "Goal Seek..." feature under Excels "Tools" pull-down menu as follows:

"Set Cell" - select the cell under  $Q_{\text{calc}}$  for which the optimazation is to be conducted

"To Value" - insert the numerical value of  $Q_{\text{calc}}$  (equal to  $Q_{\text{des}})$ 

"By Changing Cell" - select the cell under Flow Depth which will be changed to yield the desired Q<sub>calc</sub>

<u>Note</u>: the Goal Seek tool may not converge to an answer.

Excel will calculate the *Flow-Depth-to-Pipe-Diameter Ratio*, *d/D*, in Column H.

Using d/D, Excel will look up the geometric relationships on Sheet Circular and calculate the the Flow Area, A, the Wetted Perimeter,  $P_w$ , and the Hydraulic Radius,  $R_h$ , in Columns I - K.

Excel calculates the *Flow Velocity*, v, in Column L using Manning's Equation:

Excel calculates the *Travel Time*,  $T_t$ , in Column M using the equation:

$$T_t = L/v$$

Excel calculates the *Flow Rate*,  $Q_{calc}$ , as follows:

$$Q_{calc} = v*A$$
  $v = Flow Velocity$ , as calculated by Manning's Equation  $A = Flow Area$ , as calculated in Column I

Excel calculates the *design Flow Rate*,  $Q_{des}$ , in cubic feet per second (cfs) based on a unit conversion of the user-defined flow rate entered in Cell D25.

#### If the Spreadsheet is used to determine the design diameter of a pipe based on a design flow rate:

In the final two columns, Excel calculates the minimum design diameter for full-flow conditions based on rearrangement of the Chezy-Manning Equation, as recommended by the American Pipe Manual (American Cast Iron Pipe Company, 1986):

$$Q = (1.49/n) A R^{2/3} S^{1/2}$$

$$A R^{2/3} = (Qn/S^{1/2})(1/1.49)$$

$$(c_1D^2)(c_2D)^{2/3} = (Qn/S^{1/2})(0.671)$$

Where  $c_1,\,c_2$  are the values of  $A/D^2$  and  $R_h/D$  for a given d/D (see Sheet Circular)

$$D = [(Qn/S^{1/2})\{0.671/(c_1)(c_2)^{2/3}\}]^{3/8}$$

Note: for gravity flow, the "full-flow" capacity is generally assumed to correspond to a d/D ratio of approximately 0.67 and the equation reduces to:

$$D = [(2.76On)/S^{1/2}]^{3/8}$$

### Flow Through Circular Pipes Geometric Relationships

(Area, Wetted Perimeter, and Hydraulic Radius)

d/D	$A/D^2$	$P_{\rm w}/D$	R <sub>h</sub> /D
0.01	0.0013	0.2003	0.0066
0.01	0.0013	0.2838	0.0030
0.02	0.0057	0.2838	0.0132
0.04	0.0105	0.4027	0.0262
0.05	0.0147	0.4510	0.0326
0.06	0.0192	0.4949	0.0389
0.07	0.0242	0.5355	0.0451
0.08	0.0294	0.5735	0.0513
0.09	0.0350	0.6094	0.0574
0.10	0.0409	0.6435	0.0635
0.11	0.0470	0.6761	0.0695
0.12	0.0534	0.7075	0.0754
0.13	0.0600	0.7377	0.0813
0.14	0.0688	0.7670	0.0871
0.15	0.0739	0.7954	0.0929
0.16	0.0811	0.8230	0.0986
0.17	0.0885	0.8500	0.1042
0.18	0.0961	0.8763	0.1097
0.19	0.1039	0.9020	0.1152
0.20	0.1118	0.9273	0.1206
0.21	0.1199	0.9521	0.1259
0.22	0.1281	0.9764	0.1312
0.23	0.1365	1.0003	0.1364
0.24	0.1449	1.0239	0.1416
0.25	0.1535	1.0472	0.1466

## Flow Through Circular Pipes Geometric Relationships

(Area, Wetted Perimeter, and Hydraulic Radius)

d/D	$A/D^2$	$P_{\rm w}/D$	R <sub>h</sub> /D
0.26	0.1623	1.0701	0.1516
0.27	0.1711	1.0928	0.1566
0.28	0.1800	1.1152	0.1614
0.29	0.1890	1.1373	0.1662
0.30	0.1982	1.1593	0.1709
0.31	0.2074	1.1810	0.1755
0.32	0.2167	1.2025	0.1801
0.33	0.2260	1.2239	0.1848
0.34	0.2355	1.2451	0.1891
0.35	0.2450	1.2661	0.1935
0.36	0.2546	1.2870	0.1978
0.37	0.2642	1.3078	0.2020
0.38	0.2739	1.3284	0.2061
0.39	0.2836	1.3490	0.2102
0.40	0.2934	1.3694	0.2142
0.41	0.3032	1.3898	0.2181
0.42	0.3130	1.4101	0.2220
0.43	0.3229	1.4303	0.2257
0.44	0.3328	1.4505	0.2294
0.45	0.3428	1.4706	0.2331
0.46	0.3527	1.4907	0.2366
0.47	0.3627	1.5108	0.2400
0.48	0.3727	1.5308	0.2434
0.49	0.3827	1.5508	0.2467
0.50	0.3927	1.5708	0.2500

## Flow Through Circular Pipes Geometric Relationships

(Area, Wetted Perimeter, and Hydraulic Radius)

d/D	$A/D^2$	$P_{\rm w}/D$	R <sub>h</sub> /D
0.51	0.4027	1.5908	0.2531
0.52	0.4127	1.6108	0.2561
0.53	0.4227	1.6308	0.2591
0.54	0.4327	1.6509	0.2620
0.55	0.4426	1.6710	0.2649
0.56	0.4526	1.6911	0.2676
0.57	0.4625	1.7113	0.2703
0.58	0.4723	1.7315	0.2728
0.59	0.4822	1.7518	0.2753
0.60	0.4920	1.7722	0.2776
0.61	0.5018	1.7926	0.2797
0.62	0.5115	1.8132	0.2818
0.63	0.5212	1.8338	0.2839
0.64	0.5308	1.8546	0.2860
0.65	0.5404	1.8755	0.2881
0.66	0.5499	1.8965	0.2899
0.67	0.5594	1.9177	0.2917
0.68	0.5687	1.9391	0.2935
0.69	0.5780	1.9606	0.2950
0.70	0.5872	1.9823	0.2962
0.71	0.5964	2.0042	0.2973
0.72	0.6054	2.0264	0.2984
0.73	0.6143	2.0488	0.2995
0.74	0.6231	2.0714	0.3006
0.75	0.6318	2.0944	0.3017

## Flow Through Circular Pipes Geometric Relationships

(Area, Wetted Perimeter, and Hydraulic Radius)

d/D	$A/D^2$	$P_{\rm w}/D$	R <sub>h</sub> /D
0.76	0.6404	2.1176	0.3025
0.77	0.6489	2.1412	0.3032
0.78	0.6573	2.1652	0.3037
0.79	0.6655	2.1895	0.3040
0.80	0.6736	2.2143	0.3042
0.81	0.6815	2.2395	0.3044
0.82	0.6893	2.2653	0.3043
0.83	0.6969	2.2916	0.3041
0.84	0.7043	2.3186	0.3038
0.85	0.7115	2.3462	0.3033
0.86	0.7186	2.3746	0.3026
0.87	0.7254	2.4038	0.3017
0.88	0.7320	2.4341	0.3008
0.89	0.7384	2.4655	0.2995
0.90	0.7445	2.4981	0.2980
0.91	0.7504	2.5322	0.2963
0.92	0.7560	2.5681	0.2944
0.93	0.7612	2.6061	0.2922
0.94	0.7662	2.6467	0.2896
0.95	0.7707	2.6906	0.2864
0.96	0.7749	2.7389	0.2830
0.97	0.7785	2.7934	0.2787
0.98	0.7816	2.8578	0.2735
0.99	0.7841	2.9412	0.2665
1.00	0.7854	3.1416	0.2500

#### Notes:

from the Engineer-in-Training Reference Manual, Professional Publications Inc., Belmont, CA

## **Manning's Coefficients**

Pipe/Channel Material	Manning n
1 Planed Wood	0.012
2 Unplaned Wood	0.013
3 Finished Concrete	0.012
4 Unfinished Concrete	0.014
5 Concrete Pipe	0.015
6 Sewer Pipe	0.013
7 Vitrified Clay	0.013
8 Brick	0.016
9 Plastic Pipe (SDR, S&D, etc.)	0.011
# HDPE, smooth	0.009
# HDPE, corrugated (3-6" d)	0.015
# HDPE, corrugated (18-24" d)	0.020
# Ductile Iron w/ poly liner	0.010
# Cast/Wrought Iron	0.015
# Riveted Steel	0.017
# Corrugated Metal Flumes	0.025
# Earth, straight channel	0.022
# Rubble	0.030
# Earth, w/ stones, weeds	0.035
# Mountain Streams	0.050

#### Notes:

Manning's Coefficients are dependent on flow velocity, and cited values are typically based on velocities ranging from approximately 2.0 ft/sec to 5.0 ft/sec. For lower flow velocities, the appropriate Manning's Coefficient may be greater than values listed in this table.

#### Attachment E Conduit Outlet Protection Calculations

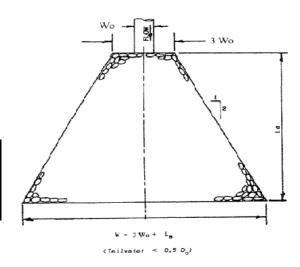
## Rip Rap Apron Conduit Outlet Protection Cornell-Dublier

Pipe Inlet	Q (cfs)	q (unit discharge)	D <sub>o</sub> (ft)	W <sub>o</sub> (ft)	L <sub>a</sub>	Top Width (ft)	Bottom Width (ft)
1 (CB-8)	11.3	4.52	2.5	2.5	22.65	7.5	30.15
2 (CB-9)	11.1	4.44	2.5	2.5	22.55	7.5	30.05
3 (CB-6)	30.2	15.10	2	2	33.22	6	39.22
4 (CB-25)	11.3	4.52	2.5	2.5	22.65	7.5	30.15
5 (CB-16)	17.1	8.55	2	2	24.88	6	30.88
6 (CB-12)	11.3	5.65	2	2	21.19	6	27.19
Pond Outlet	15.1	10.07	1.5	1.5	25.29	4.5	29.79

$$L_a = (1.8 \frac{q}{D_o^{1/3}}) + 7Do$$
 TW < 1/2 D<sub>o</sub>

$$W = 3$$
  $W_o + L_a$ 

Equations and Diagram Taken from: Standards for Soil Erosion and Sediment Control in New Jersey July 1999 12-5 Conduit Outlet Protection



### "RipRap Size"

The median stone diameter, D<sub>50</sub>, in feet, shall be determined from the formula:

$$D_{50} = \frac{0.016}{T_{w}} q^{1.33}$$
 TW = 0.5  
D<sub>50</sub> = 0.24 feet  
2.86 inches

\*Assming Tailwater is .2 Do and using largest Diameter Pipe

## **APPENDIX D**

## Appendix D

## Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

#### INSPECTION BEING CONDUCTED:

QU	ARTERLY ANNUALLY AFTER 1" or GREATER RAIN	FALL		
AF'	TER 1.25" IN 2 HOUR STORM EVENT UNDER SPECIAL CIRC	CUMSTAN	NCES	_
Ins	pection Date: Weather:			
Ins	pector's Name:			
	SASIN" Inspection:			
		Yes	No	N/A
	Catch Basins (23 Structures):			
1.	Are catch basins properly draining?			
2.	Are the catch basins clear of trash, sediment, and debris?			
3.	Has vegetation been removed from all catch basin areas?			
4.	Are there any signs of damage or deterioration of catch basins?  If yes, which catch basin(s)?			
	(Refer to Record Drawings for catch basin numbers)			
	Stormwater Detention Basin and Surface Sand Filter:			
5.	Does the basin have pooled or standing water?			
	If yes, describe where:			
6.	What is the water height?			
	Approximately how many hours ago was the last rainfall?			
	How many inches of rain?			
7.	Does the bottom appear relatively flat? No sand has washed away?			
8.	Are concentrated flows of runoff being unexpectedly directed into the basin?			
	If yes, describe where:			
9.	Is there any damage to the sand bed or berms?			
10.	Has vegetation been removed from the basin areas?			

## Appendix D

## Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

	Yes	No	N/A
Inlet and Outlet Structures:			
11. Are the five inlet and four outlet structures draining properly?			
12. Is there any standing water?  If yes, describe where:			
13. Are the inlets clear of trash, sediment, and debris?			
14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?			
15. Are there any signs of damage or deterioration of inlet/outlet structures? If yes, describe where:			
16. Has vegetation been removed from the inlets and outlets?			
Additional descriptions of where repairs or maintenance is needed:			
Inspector's Signature			

#### **APPENDIX D**

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

#### "Pavement" Inpsection (Part of Annual Inspection):

	Yes	No	N/A
1. Is there any standing water?			
If yes, describe where:			
3. Are there any signs of disintegration?  If so, note location and maintenance effort below.			
4. Are there any signs of distortion?  If so, note location and maintenance effort below.			
5. Has all vegetation been removed?  If applicable, note location of vegetation below.			
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?			
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatments been applied?  When was the date of the last CPM treatment?			
Additional descriptions of where repairs or maintenance is needed:			
Inspector's Signature			

#### **APPENDIX D**

#### **Operation & Maintenance Inspection Form** Cornell-Dubilier Electronics Superfund Site **Operable Unit (OU-2)**

## Basin Drainage Rate Inspection: (completed twice a year after design rainfall event)

esign Rainfall Ev	vent Information		
	" of rain in 2 hours		
art:			
op:			
ches of Rainfall:			
	5 hours after design rain event inspections every 2 hours ddle basin.		r drops below the top o
Inspection	Target Time From	Actual Time	Water height (ft)
Run #	Event (hrs) 16		
2	18		
	20		
3	20		
3 4	22		
4	22		
5	22 24		
4 5 6	22 24 26		
4 5 6 7	22 24 26 28		

Inspector's Signature

## **APPENDIX E**

# APPENDIX E Point of Contact Information Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

Mr. Diego Garcia USEPA Region 2 290 Broadway New York City, NY 10007-1866

garcia.diego@epamail.epa.gov

Office Phone: 212-637-4947

## **APPENDIX F**

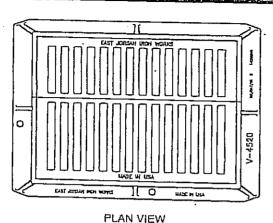
Below are Malcolm Pirnie's comments on the attached Drainage Structure Cut Sheets, which was submitted via email on April 13, 2011:

- 1. Please provide a proposed material list which corresponds to the Catch Basin Schedule shown on Drawing G-12. It is not clear what type or size structure is proposed for a specific location on-site. Also indicate the number and size of any riser sections that will be required at each location.
- 2. Currently a mortared joint is shown for the 48" diameter stormwater manhole and a grouted joint is shown for the 60" diameter stormwater manhole. Please confirm that the Press Seal gaskets will be utilized for manhole-to-pipe seals.
  - 3. It appears only the "type 2" style grate was included in the submittal. Please provide information for "Type 1" and "Type 3" styles as shown on Drawings G-12 and G-14.

Please let me know if you have any questions

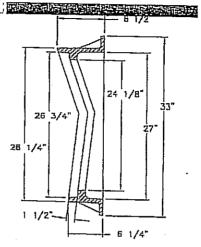
Thanks Ben

## V4520 V4520-1 ASSEMBLY



1 15/16\* 26 1/2 15 7/16" 1 11/16

GRATE SECTION



FRAME SECTION

#### PRODUCT NUMBER 44520001

#### **DESIGN FEATURES**

MATERIALS **GRATE-GRAY IRON** ASTM A48 CL35B FRAME-GRAY IRON

ASTM A48 CL358

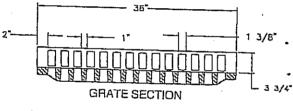
**DESIGN LOAD** HEAVY DUTY

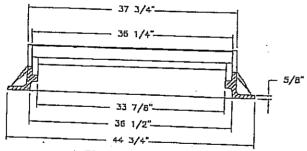
COATING

UNDIPPED **OPEN AREA** 

458 SQ. INCHES

√ DESIGNATES MACHINE SURFACE





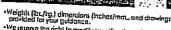
CB-1,2,3,4A,7,8,10,11,13,13A,14,15,17,4,19,20,21,22 (TYPE 2)

FRAME SECTION

Corporate Headquarters 301 Spring Street PO Box 439 East Jordan, MI 49727-0439 800.874.4100







We receive the right to modify specifications without pile notice. ·Uncontrated distribution.



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#### REFERENCE INFORMATION

44520130 44520010

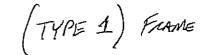
#### DRAWING DETAILS

ORIGINAL DRAWING: DEW 11/11/03

REVISED BY: 10/29/10 DEW

#### V-5660 FRAME

CB-4,5,6,9,12,16



PRODUCT NUMBER

#### 45660010

#### DESIGN FEATURES

MATERIALS

FRAME-GRAY IRON ASTM A48 CL358

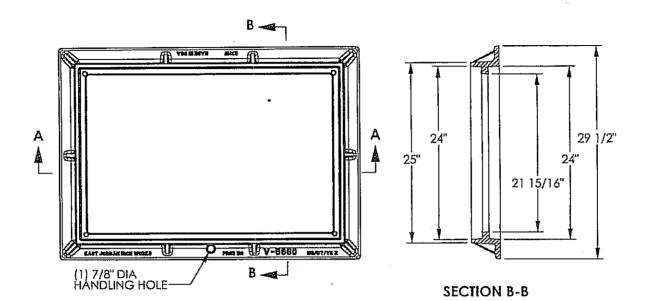
DESIGN LOAD HEAVY DUTY

> COATING UNDIPPED

OPEN AREA

N/A

√ DESIGNATES MACHINE SURFACE



36"

1 1/2"

4"

34"

41 1/2"

**SECTION A-A** 

NOTE: FRAME IS REVERSIBLE AND CAN BE INSTALLED AS A TOP FLANGE UNIT.

Corporate Headquarters 301 Spring Street PO Box 439 East Jordan, MI 49727-0439 800.874.4100

**☑** GROUP

Coll loday for More Information

. Uncontrolled distribution.

 Weights (lbs/kg.) dimensions (inches/mm., and drawings provided for your guidence.

. We reserve the right to modify specifications without prior notice.

800.626.4653

EJIM EASTJORDAN
TRON WORKS EST BES
WE COVER YOUR INFRASTRUCTURE

www.ejiw.com MADE IN THE USA

CONFIDENTIAL: This drawing is the property of East Jordan from works, Inc., and embodies confidential information, registered marks, trade secret Information, and/or know how that is the property of East Jordan from Works, Inc. OCopyright 2009 East Tordan from Works, Inc.

\_ DRAWING DETAILS

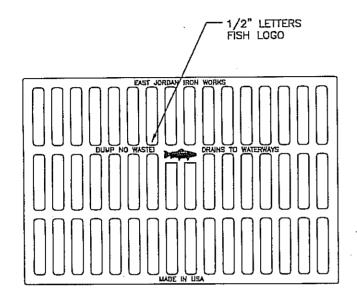
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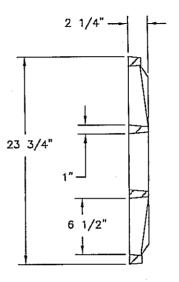
REVISED BY:

DEW 2/8/2011

CB-4,5,6,9,12,16

(TYPE 1) GRATE





**GRATE SECTION** 

#### EJIW EAST JORDAN

800-626-4653 www.ejiw.com MADE IN USA

PRODUCT NUMBER

45660033

CATALOG NUMBER

V-5660

#### CATCH BASIN GRATE

LOAD RATING
HEAVY DUTY

COATING

UNDIPPED

ESTIMATED WEIGHT GRATE: 190 LBS

MATERIAL SPECIFICATION

GRATE - GRAY IRON ASTM A48 CL35B

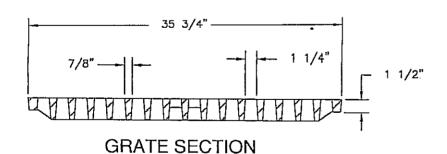
> OPEN AREA 390 SQ IN

√ DESIGNATES MACHINED SURFACE

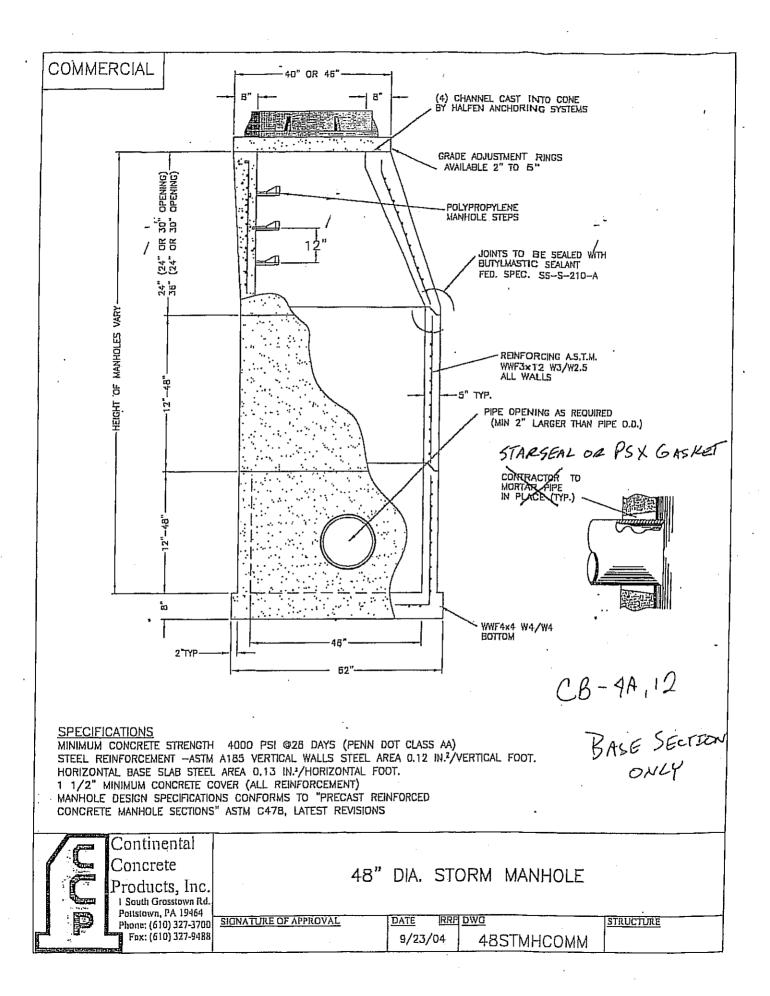
DRAWN	DATE
DEW	07/29/02
LAST REVISED	
SBB	08/02/06

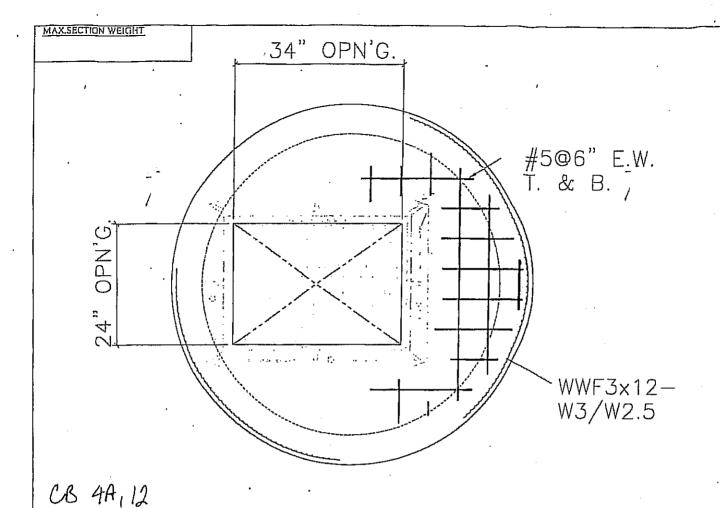
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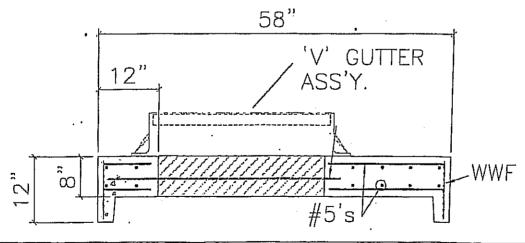
4566033.2C 4566033.1D



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SPECIFICATIONS

\*MATERIAL AND CONSTRUCTION
SHALL COMPLY WITH NJ D.O.T.

\*NJ DOT CLASS B CONCRETE
4500 P.S.I. @ 28 DAYS

\*REINFORCEMENT~ A.S.T.M. A615

GRADE 60, MIN. 1 1/2" COVER U.N.O.



Continental Concrete

Products, Inc. 1 South Grosstown Rd. Potistown, PA 19464

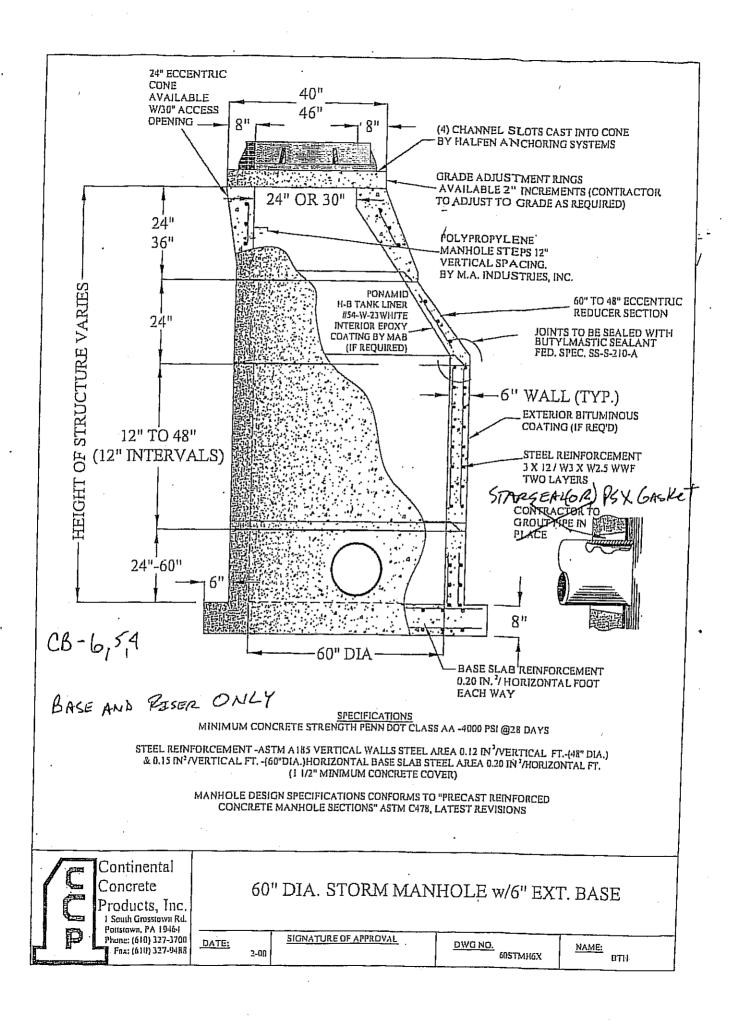
Plione: (610) 327-3700 DATE Fax: (610) 327-9488

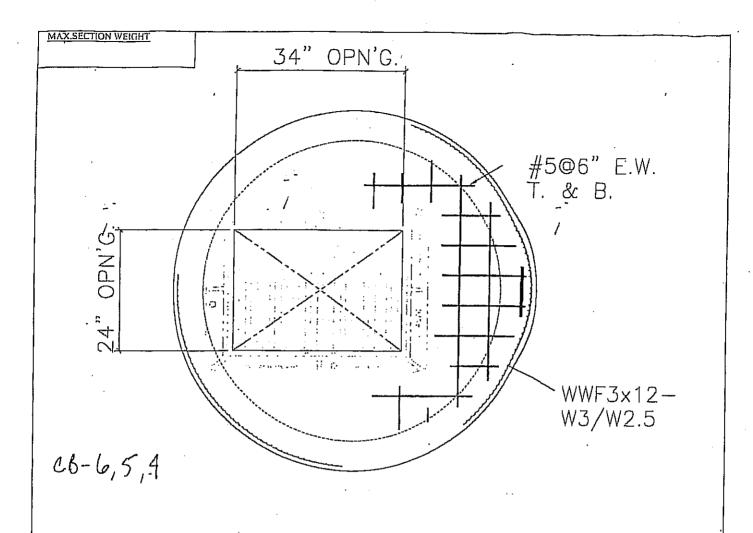
CORNELL DUBILIER SUPERFUND SITE SOUTH PLAINFIELD, N.J. SEVENSON ENVIREMENTAL SERVICES Ø48" INLET LID

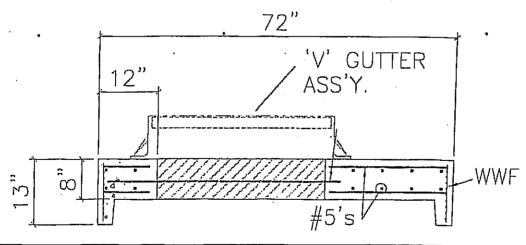
RRP DWG

4/11/11 1187-4A STRUCTURE

4A







**SPECIFICATIONS** \*MATERIAL AND CONSTRUCTION SHALL COMPLY WITH NJ D.O.T.

\*NJ DOT CLASS B CONCRETE

4500 P.S.I. © 28 DAYS

\*REINFORCEMENT~ A.S.T.M. A615

GRADE 60, MIN. 1 1/2" COVER

U.N.O.



Continental Concrete

Products, Inc. I South Grosstown Rd.

Polisiown, PA 19464 Phone: (610) 327-3700 DATE Fax: (610) 327-9488

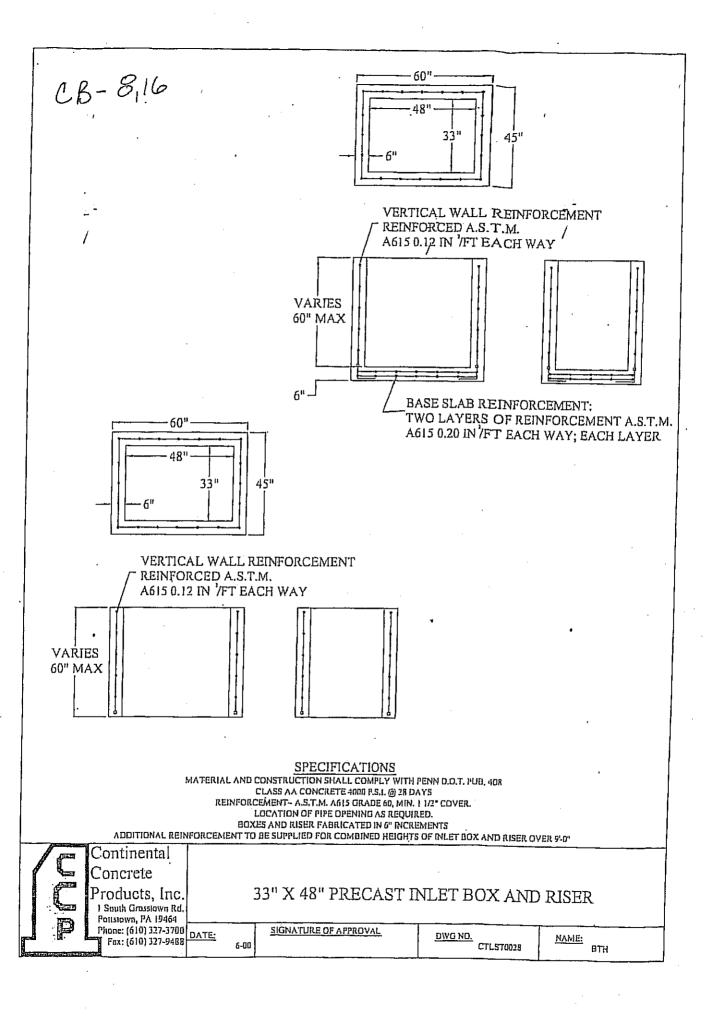
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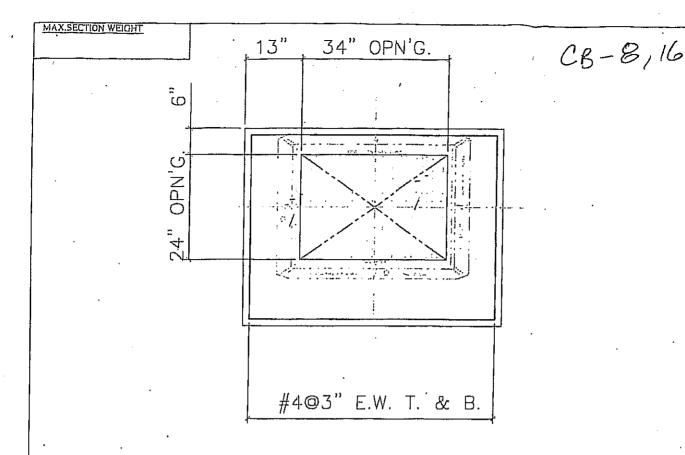
Ø60" INLET LID

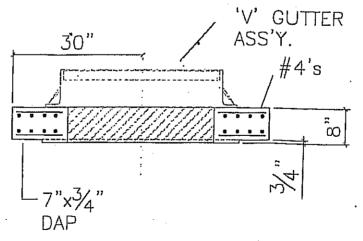
RRP DWG 4/11/11

1187-4

STRUCTURE 4







SPECIFICATIONS

\*MATERIAL AND CONSTRUCTION
SHALL COMPLY WITH NJ D.O.T.

\*NJ DOT CLASS B CONCRETE
4500 P.S.I. © 28 DAYS

\*REINFORCEMENT~ A.S.T.M. A615'
GRADE 60, MIN. 1 1/2" COVER

U.N.O.



Continental Concrete

Products, Inc.
1 South Grosstown Rd.
Polisiown, FA 19464

Pottstown, PA 19464
Phone: (610) 327-3700

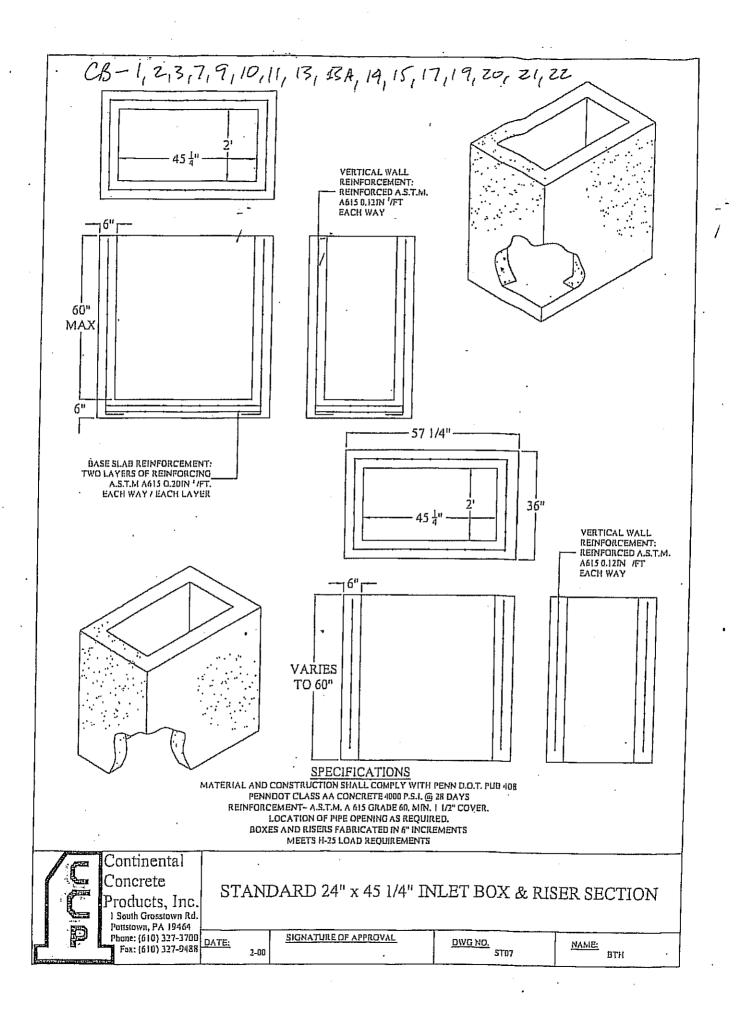
Fax: (610) 327-9488

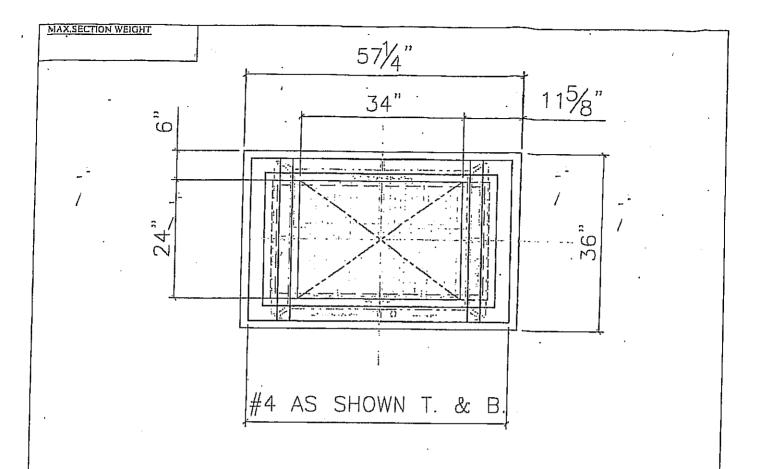
4/1

CORNELL DUBILIER SUPERFUND SITE SOUTH PLAINFIELD, N.J. SEVENSON ENVIREMENTAL SERVICES

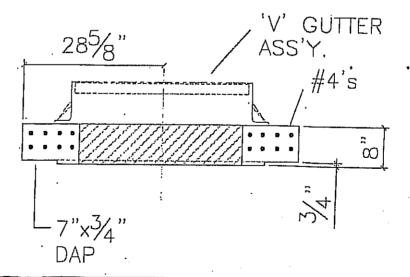
33"'x48" INLET LID

DATE | RRP DWG | STRUCTURE | 4/11/11 | 1187-816 | 8,16





CB-1,213,7,9,10,11,13,13A,14,15,17,19,20,21,22



#### **SPECIFICATIONS**

\*MATERIAL AND CONSTRUCTION SHALL COMPLY WITH NJ D.O.T.

\*NJ DOT CLASS B CONCRETE 4500 P.S.I. @ 28 DAYS

\*REINFORCEMENT~ A.S.T.M. A615 GRADE 60, MIN. 1 1/2" COYER



#### Continental Concrete

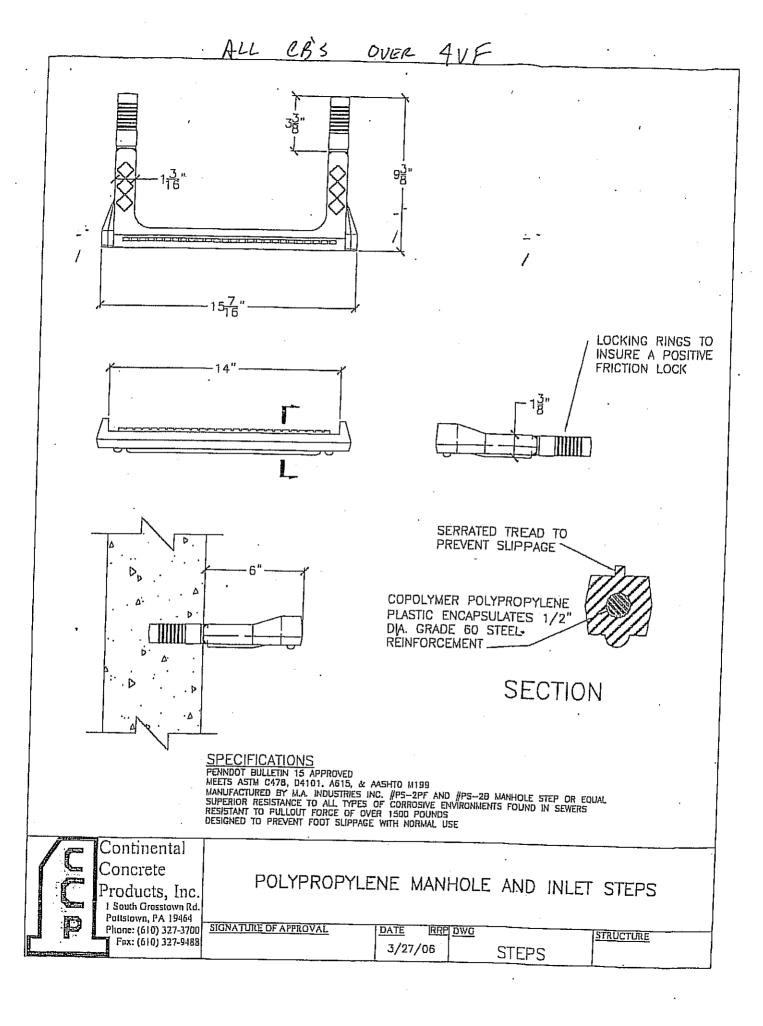
Products, Inc. l South Grosstown Rd.

Pottstown, PA 19464 Phone: (610) 327-3700 DATE Fax: (610) 327-9488

## CORNELL DUBILIER SUPERFUND SITE

SOUTH PLAINFIELD, N.J. SEVENSON ENVIREMENTAL SERVICES 2'x4' INLET LID

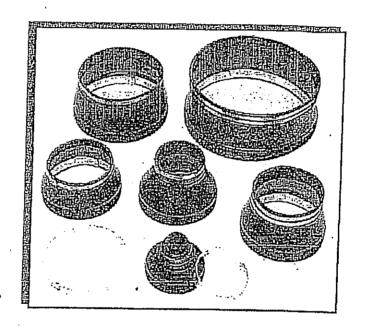
4/11/11 1187-24 STRUCTURE MULTI







## POSITIVE SEAL GASKET SYSTEM WITH POWER SLEEVE EXPANSION





PRESS-SEAL GASKET CORPORATION

#### **PSX**

Our original PSX; POSITIVE SEAL pipé-to-manhole flexible connector system. PSX is available for 8" and larger holes to seal the most commonly used pipe types and sizes.

#### **PSX ADVANTAGES**

Meets and/or exceeds Material Specifications of ASTM C-923.

\* Type 304 Stalnless Steel Compression Power Sleeve is one piece with no welds. Made with 10 and 11 gauge steel.

\* Highest Installation pressures, 2500 to 6000 PSI. The more PSI force, the better the initial and long term seal.

\* Gasket is made of high quality Polyisoprene rubber. Provides greater deflection capa bilities and greater tear resistance.

\* Type 304 Stainless Steel Take-Up Clamps.

\* For use with cored holes, or our fiberglass, or Pro-Former Hole Formers.

	T		
TEST /	ASTM METHOD	TEST REQUIREMENTS	TEST RESULTS
CHEMICAL RESISTANCE; 1N SULFURIC ACID 1N HYDROCHLORIC ACID	D 534, AT 22°C FOR 48 HRS	NO WEIGHT LOSS NO WEIGHT LOSS	NO WEIGHT LOSS NO WEIGHT LOSS
TENSILE STRENGTH	D 412	1200 PSI, MIN.	2600 PSI
ELONGATION AT BREAK	D 412	350%, MIN.	675%
HARDNESS	D 2240 (SHORE A DUROMETER)	SPECIFIED HARDNESS	45
ACCELERATED OVEN-AGING	D 0/0, raz 1 B 1 OK 1 BA1a	DECREASE OF 15%, MAX. OF ORIGINAL TENSILE STRENGTH, DECREASE OF 20%, MAX. OF ELONGATION	-13% TENSILE CHANGE, -14% ELONGATION CHANGE
COMPRESSION TEST	D 395, METHOD B, AT 70°C FOR 22 HRS	DECREASE OF 25%, MAX. OF ORIGINAL DEFLECTION	13.20%
WATER ABSORPTION	D 471 IMMERSE 0.75 BY 2-IN. SPECIMEN IN DISTILLED WATER AT 70°C FOR 48 HOURS	INCREASE OF 10%, MAX. OR ORIGINAL BY WEIGHT	3.50%
OZONE RESISTANCE	D 1171	RATING 0	PASS
LOW-TEMP, BRITTLE POINT	D 746	NO FRACTURE AT -40°C	PASS
TEAR RESISTANCE	D 624, METHOD B	200 LBF/IN. (MIN.)	318 LBF/IN.

#### PIPE INSTALLATION

Clean pipe and boot to ensure no dirt or foreign materials are present

 Clean pipe and boot to ensure no dirt or foreign materials are present.
 Clamping surface on pipe must be clean and smooth.
 Center pipe in opening and insert until pipe breaks the inside plane of manhole.
 Attach take-up clamp(s) and stagger screw(s) of clamp(s) around the groove of the gasket so that take-up pressure will be equalized. Make sure each clamp is completely in the correct groove.
 Using a torque ratchet or torque wrench, gradually tighten all screw(s) of clamp(s) in an alternating pattern to 60lbs/in torque.
 After reaching 60lbs/in torque on final screw, check all screws again to ensure equal compression of all clamps.
 Vacuum testing shall be conducted in accordance with ASTM C-1244-02.
 Adjust pipe to line and grade. Use proper bedding, backfill materials and techniques so that pipe deflaction and deformation is minimized. Installation of the concrete structure shall be such that differential settlement between the structure and the pipeline shall be less than 10% of pipe diameter for pipes less than 20" and shall be less than 5% of pipe diameter for pipes between 20 and 60 inches in diameter. and 60 Inches in diameter

9. Any pipe stubs installed in the manhole must be positively restrained from movement per ASTM C-923. Press-Seal is not responsible for blow outs due to unrestrained pipe stub or future lateral connections.

Before using the PSX-POSITIVE SEAL system for any custom applications, contact our Customer Service Department for more information.

U.S. Patent No.'s 4215880, 4478437 Copyright 1994 by Press-Seal Gasket Corporation (July 2002)

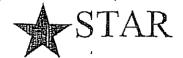
Press-Seal believes all information is accurate as of its publication date. Information, specifications, and prices are all subject to change without notice. Press-Seat is not responsible for any inadvertent errors.



#### PRESS-SEAL GASKET CORPORATION

P.O. Box 10482, Fort Wayne, Indiana 46852

Phone: (260) 436-0521 (600) 348-7325 Fax: (260) 436-1908 E-mail: sales@press-seal.com Web: www.press-seal.com







"RUBBERMAN"

PIPE-TO-MANHOLE SEAL

- 1. MEETS ASTM 923-C SPECIFICATIONS
- 2. RUBBER MEETS ASTM 443.
- 3. 20° OMNI-DIRECTIONAL DEFLECTION.
- .4. ACCOMMODATES PIPE OD'S FROM 3" AND UP.
- 5. AS SIMPLE AS LUBRICATING PIPE AND GASKET AND INSERTING PIPE.
- 6. MAY BE USED IN ROUND OR SQUARE STRUCTURES.
- 7. ACCOMMODATES SANITARY OR STORM SEWER SITUATIONS.
- 8. A COMPRESSION SEAL INTEGRALLY CAST INTO THE STRUCTURE.
- 9. HAS BEEN SUCCESSFULLY USED FOR PIPE ENTRY IN SANITARY SEWERS, STORM SEWERS, SEPTIC TANKS, AND WALLS OF TREATMENT PLANTS.
- 10. "RUBBERMAN I" LUBRICANT IS SUGGESTED AS AN ENVIRONMENTALLY FRIENDLY PRODUCT TO FACILITATE INSTALLATION PROCEDURES.
- 11. CAST IN STRUCTURE USING TWO-PIECE STEEL RINGS. ELIMINATING AN "OUT OF ROUNDNESS" SITUATION COMMON WITH FIBERGLASS RINGS.
- 12. CONSULTATION AVAILABLE CONCERNING ANY ANTI INFILTRATION-EXFILTRATION PROJECT.
- 13. OVER 40 YEARS EXPERIENCE DESIGNING, TESTING, MARKETING, AND SERVICING SEALS FOR MANUFACTURES OF PIPE, MANHOLES, SEPTIC TANKS, PUMP STATIONS, AND TREATMENT PLANTS.
- 14. "RUBBERMAN" IS A TRADE NAME FOR O RING, PROFILE GASKETS, PIPE TO MANHOLE SEALS; LUBRICANT, SEALANT, AND THE NEW PRECAST SEWER CHIMNEY AND PIPE ENCASEMENT SYSTEM.

#### NOTE:

ALL STAR-SEAL<sup>TM</sup> PIPE-TO-MANHOLE GASKETS ARE MANUFACTURED BY: HAIL MARY RUBBER CO. INC. WARMINSTER, PA. DISTRIBTED BY:CONTINENTAL CONCRETE PRODUCTS INC. POSSTOWN, PA.

Continental
Concrete
Products, Inc.
1 South Grosstown Rd. Pottstown, PA 19464
Phone: (610) 327-3700
Fax: (610) 327-9488

STAR-SEAL $^{\text{TM}}$  COMPRESSION GASKET SPECIFICATION SHEET

DATE: 6-00 SIGNITURE OF APPROVAL DIVG NO. SA12 BTH



# MultiSeal, Inc. Designed Seniant and Adhesive Technologies

11/06/03

Corporate Office & Mfg. 4329 [Hitch and Peters Rand Evansville, IN 47711 • Tel. (\$12) 428-3422 • Fax. (\$12) 428-3432 R & D / Compounding. 2131 Commercial Ct. Evansville, IN 47726 • Tel. (\$12) 428-3445

MP1255

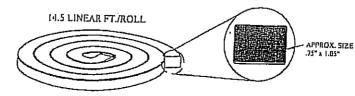
## TECHNICAL DATA

Page | of |

· · · · · · · · · · · · · · · · · · ·			
Product number:	MP1255		
Description -	Higher density polymer-based	mostic associational	
,	t annamines deliber molecure do	Higher density polymer-based mastic used to seal most clean substrates against moisture, dust, and air. Substrates include	
ASTM C990	controve, mean, plastic, and or	liër siirre aase	
76111 (.990)	Meets ASTM C990 specificati	on for Butyl Rubber Scalant	
Butyl Rubber (hydrocarbon blands)	Specification Requirement 50 % minimum	Typical Results	
	20 50 minimum	54 %	
Ash-Inert Mineral Matter	30% minimum	44	
	-	74	
Volatile Matter	2 % maximum	<2 %	
	,	/4	
Specific Gravity at 77°F	1-15 minimum, 1.50 maximum	1.36	
Ducillity at 77°F, cm	5.0 minimum	8	
Floris Being CLO C			
Flush Point, C.O.C.	350 °F minimum	>500 m	
Fire Point, C.O.C.			
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	375 °F mmunum	>500 °Jr	
Rebound Test at 77°F	3 % to 15 %		
at 32°F	3 % to 20 %	5.2 5.8	
	5 4 10 20 41	2.6	
Compression Test of 77°F, Ibi/In.	100 musimum	91	
at 32°F, lbf/in.2	200 maximum	128	
		1-11	
Low Temperature Flexibility at -10° F	1807 bend, no cracking and no	Pays	
	loss of adhesion		
T*1 . 1 *** **			
Elevated Temperature Flow, 14 days at 158°F	No sag. and no change in	Pass	
138 F	extruded shape		
Adhesion after impact	No greater loss than 50 % of		
· · · · · · · · · · · · · · · · · · ·	ingposion	Pass	
	The state of the s		
Cone Penetration at 77°F, dmm	50 to 100	80-95	
nt 32°F, dum	40 minimum	5()	
•			
Chemical Resistance	No deterioration, cracking, or	Puss	
	swelling .		
*** ******			
Shell'life	Minimum I year		



Continental Concrete Products, Inc. 1 South Grossnewn Rd. Pattsnewn, PA 1946-1 Phone: (610) 327-3780 Fax: (610) 327-9488



---// 

•	MANHOLE PRODUCTION SHEET - REV. A 8/25/05				
	BACE A				
JOB QUATEN 11-87 STRUCTURE DE	Sevenson Environmental				
PROID DATE					
DELIVERY DATEFLOW CHANNE	YES NO				
COMMERCIAL					
PENNDOT	_ <del>-</del>				
_					
PWD	12				
	\(\sime\) 11 \(\frac{12}{12}\) 1' \(\sime\)				
	34" \( \begin{align*} \text{10 Diameter} \\ \text{Diameter} \\ \end{align*}				
]	34" / 10 Diameter 2)				
Low -					
Low -	19pc + 9 = 1				
H	de 1 5 Base J				
	\_8				
	$\frac{1}{\sqrt{7}}$ 6 5 $\frac{1}{\sqrt{2}}$				
	21" /				
	34" Low-Taper Hole				
	Hole '				
1'-0" MANHOLE RISER [48" DIA]	MH FLAT LID ( W/24"o) [48" DIA.] MH FLAT LID ( W/24"o) [60" DIA.]				
2'-0" MANHOLE RISER [48" DIA]	MH FLAT LID (W30"o) [48"DIA.] MH FLAT LID (W30"o) [60" DIA.]				
3'-0" MANHOLE RISER [48" DIA]	1'-0" MANIHOLE RISER [72" DIA]				
4'-0" MANHOLE RISER [48" DIA] 2'-0" MH CONE (W/24"o)	2'-0" REDUCER CONE [60"TO48"] 2'-0" MANHOLE RISER [72" DIA]				
2'-0" MH CONE (W/30"u)	1'-0" MANHOLE RISER [60" DIA]   3'-0" MANHOLE RISER [72" ID]   2'-0" MANHOLE RISER [60" DIA]   4'-0" MANHOLE RISER [72" ID]				
3'-0" MH CONE (W/24"0)	TOUR TOUR TENT				
3'-0" MH CONE (W/30"o)	3"-0" MANHOLE RISER [60" DIA]   MH FLAT L1D (W/24"o) [72" DIA.]   4"-0" MANHOLE RISER [60" DIA]   MH FLAT L1D (W/30"o) [72" DIA.]				
NO ENTERIOR COATING ENTERIOR CO	DATING (BLACK)				
ENTERIOR EPONY (TAN) INTERIOR EP	DXY (WILLE)				
NO MILIFRAME & COVER 24" MILIF C "S	JORAN 12				
MHJT, SEALANT (ROLLS) 3 30" MH F/C ST	ORM 11 i2 I				
M.H. FRAME / COVER	10				
ADDITIONAL INFORMATION:	ADDITIONAL INFORMATION:				
X" x 36" Special F& G (TIPE 1) 12" - 24" 24" 3-					
(2) 211 0( ) 2 1 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1					
(2) 34 L PSX-2 Boots					

	MANUAL DE COLOR					
		MANHOLE PRODUCTION SHEET - REV. A 8/25/05				
	лов схоть» <u>11-87</u>	STRUCTURE DEPTH 8. 10 _CUSTOMER: Sevenson Environmental				
	BBOY3 DATE	COSTOMER: OCVENSUIT ENVIronmental				
	PROLD DATE:	LIOB NAME: Cornell-Dubilier Superfund Site				
	DEL 1 VERY DATE	FLOW CHANNELS DISTRUCTURE#				
	COMMERCIAL	M				
	PENNDOT -					
	PWD: /					
	- 1 (12)					
		11 60 Dig 1				
		10				
		34"				
		SA" LOW-Taper (9) A-6" 3-3-134"				
		Hole 7 34"				
	•	4 LOW-TODEC				
		Y 7 5 S Hole 1				
		700				
		·				
	1'-0" MANHOLE RISER (4	8" DIA] MH FLAT LID ( W.'24"o) [48" DIA.] MH FLAT LID ( W.24"o) [60" DIA.]				
	2'-0" MANHOLE RISER [4 3'-0" MANHOLE RISER [4	8" DIA]   MH FLAT LID (W70" o) (48"DIA )				
	4'-0" MANHOLE RISER [4	R" DIA1 PLO" MANHOLE RISER [72" DIA1				
1	2'-0" MH CONE (W/24"o)	11.0" MANHOLE RISER [72" DIA]				
	2'-0" MH CONE (W/30"o)	3'-0" MANHOLE RISER [72" IDI				
	3'-0" MH CONE (W/24"o)	3' O" MANIJOLE RISER [72" ID]				
	3'-0" MH CONE (W/30"o)	41-0" MAN 1/21 P.				
	NO EXTERIOR COATING					
	ENTERIOR EPONY (TAN)	INTERIOR EPONY (WHITE)				
	NOMITERAME & COVER	ENTERIOR COATING (BLACK)  INTERIOR EPONY (WHITE)  24" MULFIC "STORM"  JO" ADLECT STORM				
	MILITE SEALANT (ROLLS) 3	30" MILEC'STORM				
M.I-	M.H. FRAME/COVER					
ADDITIONAL INFORMATION:						
26" x 36" Special Fr. + Gr. (TYPE1) "3" - 34" 34" 34"						
		PECIAL 11. Or. (11 61)				
1	2) 34(1) F	254 2 2 1 4 11 1				
<u></u>	1 34L) F	SX-2 Boot Gaskets 7 6 5				

- 1					
	MANHOLE PRODUCTION SHEET - REV. A 8/25/05 PAGE 3 OF 24				
	JOB QUOTH 11-87 STRUCTURE D	Sevenson Environmental			
	PROD DATE:BASE HEIGHT	Cornell-Dubilier Superfund Site			
		YES NO			
	DELLIVERY DATE:FLOW CHANN	IFLS D STRUCTURE# C. D 4 (/ of d)			
	COMMERCIAL	•			
	PENNDOT	<u>.</u>			
	PWD / □				
	PWD 🗆	12			
		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			
		/ `			
		34" Diameter 2:30 + Low-Toper N-Taper (9) Hole			
	,	T dole			
	Loi	v-laper T			
		tole + 38" Brow +			
	·	\8 \( \sigma \)			
'	•				
		6 3			
	•				
	1'-0" MANHOLE RISER [48" DIA] 2'-0" MANHOLE RISER [48" DIA]	MH FLAT LID ( W/24"0) [48" DIA.]   MH FLAT LID ( W.724"0) [60" DIA.]   MH FLAT LID ( W/30"0) [60" DIA.]			
ŀ	3'-0" MANHOLE RISER [48" DIA]	1'-0" MANHOLE RISER [72" DIA]			
	4'-0" MANHOLE RISER [48" DIA]	2'-0" REDUCER CONE [60"TO48"] 2'-0" MANHOLE RISER [72" DIA]			
	2'-0" MH CONE (W/24"0)	1'-0" MANHOLE RISER [60" DIA] 3'-0" MANIHOLE RISER [72" ID]			
	2'-0" MH CONE (W/30"0)	2'-0" MANHOLE RISER [60" DIA] 4'-0" MANHOLE RISER [72" ID]			
	3'-0" MH CONE (W/24"o) 3'-0" MH CONE (W/30"o)	3'-0" MANHOLE RISER [60" DIA] MH FLAT LID (W/24"o) [72" DIA.] 4'-0" MANHOLE RISER [60" DIA] MI FLAT LID (W/30"o) [72" DIA.]			
		R COATING ( BLACK)			
	ENTERIOR EPONY (TAN) INTERIOR	EPONY (WHITE)			
	NO MH FRAME & COVER 24" MH F/C	"STORM"			
	MBLIT SEALANT (ROLLS) 30" MBLE C	STORM j2 1			
I,	M.H. FRAME/COVER	10 Gon 2			
	ADDITIONAL INFORMATION:	Dia meter			
7	CON 3/1 STATE OF 1 (TYPES)				
ADDITIONAL INFORMATION:  26" x 36" Special Fr. + Grafe (TYPE)  8 3'Riscr 4					
(2) 34L PSX-2 Boot Gas/sets Star-Seel 7 6 5					
	# 580 Gas Ket				
	(-P 5")				

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MANHOLE PRODUCTION SHEET - REV. A $8/25/05$ PAGE $\frac{4}{100}$ OF $\frac{1}{100}$					
SOB CHOTE: 11-87 STRICTURE DEPTH 8.10 CUSTOMER: Sevenson Environmental					
Cornell Dubiliar Superfund Cita					
PROID DATE:BASE HEIGHT: _	YES NO				
DELTI VERY DATE:FLOW CHANNELS		CB 4 (2.42)			
COMMERCIAL M					
PENNDOT	<u>.</u> *				
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·PWD 🗆		12 /			
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60° ramoter Diarroter	→ 12" ←				
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I'-0" MANHOLE RISER [48" DIA]	MH FLAT LID ( W/24"o) [48" DIA.]	MH FLAT 1_(D (W.24"o) [60" DIA.]			
2'-0" MANHOLE RISER [48" DIA] 3'-0" MANHOLE RISER [48" DIA]	MH FLAT LID (W/30"o) [48"DIA.]	MH FLAT L.ID (W/30"a) [60" DIA.] 1'-0" MANIHOLE RISER [72" DIA]			
4'-0" MANHOLE RISER [48" DIA]	2'-0" REDUCER CONE [60"TO48"]	2'-0" MANI-JOLE RISER [72" DIA]			
2'-0" MH CONE (W/24"o)	1'-0" MANHOLE RISER [60" DIA]	3'-0" MANHOLE RISER [72" ID]			
2'-0" MH CONE (W/30"a)	2'-0" MANHOLE RISER [60" DIA]	4'-0" MANFIOLE RISER [72" ID]			
3'-0" MH CONE (W/24"n)	3'-0" MANHOLE RISER [60" DIA]	MH FLAT LID (W/24"o) [72" DIA.]			
3'-0" MH CONE (W/30"o)	4'-0" MANHOLE RISER [60" DIA]	MH FLAT LID (W/30"o) [72" DIA.]			
NO ENTERIOR COATING ENTERIOR CO	DATING ( BLACK)				
ENTERIOR EPONY (TAN) INTERIOR EP	ONY (V:HITE)				
NOMH FRAME & COVER 24" MH F/C "S	TORNE				
NIHJT: SEALANT (ROLLS) 50° NIH E C ST	TORAL	11 12 1/			
M.H. FRAME / COVER		/10 2			
ADDITIONAL INFORMATION:					
ADDITIONAL INFORMATION.					
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M	ANHOLE PRODUCTION SHEET - RE	EV. A 8/25/05 PAGE 5 OF 29
	4 20 Save	enson Environmental
JOB QUOTE: 11-8/STRUCTURE DEPT	Compa	U Dubition Consentual Olta
PROD DATE:BASE HEIGHT:		ell-Dubilier Superfund Site
DELIVERY DATEFI.OW CHANNIELS	YES NO STRUCTURE#	4A
COMMERCIAL M		
PENNDOT 🗆 -		-
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Star-	Soal / 10	2
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Gasket		<i>\</i>
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		36.
	7	6 Star-Seg
	1	
		#580
		Gasket
		3
1'-0" MANHOLE RISER [48" DIA]	MH FLAT LID ( W/24"o) [48" DIA.]	MH FLAT LID (W/24"o) [60" DIA.]
2'-0" MANHOLE RISER [48" DIA]	MH FLAT LID (W/30"0) [48"DIA.]	MH FLAT L.ID (W/30"b) [60" DIA.]
3'-0" MANHOLE RISER [48" DIA]	20 00 00 00 00 00 00 00 00 00 00 00 00 0	I'-0" MANI IOLE RISER [72" DIA]
4'-0" MANHOLE RISER [48" DIA] 2'-0" MH CONE (W/24"o)	2'-0" REDUCER CONE [60"TO48"] 1'-0" MANHOLE RISER [60" DIA]	2'-0" MANI-JOLE RISER [72" DIA]  3'-0" MANI-JOLE RISER [72" ID]
2'-0" MIH CONE (W/30"0)	2'-0" MANHOLE RISER [60" DIA]	4'-0" MANHOLE RISER [72" ID]
3'-0" MI+ CONE (W/24"0)	3'-0" MANHOLE RISER [60" DIA]	MH FLAT 1.ID (W/24"o) [72" DIA.]
3'-0" MH CONE (W/30"o)	4'-0" MANHOLE RISER [60" DIA]	MH FLAT LJD (W:30"o) [72" DIA.]
NO ENTERIOR COAFING ENTERIOR CO	DATING (BLACK)	. 1
ENTERIOR EPONY (TAN) INTERIOR EP	DX7'(WHITE) 48 C	lat
NOMH FRAME & COVER 24" MH E C "S	TORAT QIA	
MHJT, SEALANT (ROLLS) 30° MH F/C ST	CORMI	/ 11 12 1/
M.H. FRAME / COVER		/10 2
ADDITIONAL INFORMATION:		13"
	+ Grate	24" 3-)
26 x 36 Special Fr.	- Orale	Y ≤ 34" → 1
(TYPE)	2/	
		7 6 5
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		INLET PRODUCT	TON SHEET	PAGE 6 OF e	24
109 00019# .11	-87 <u>structur</u> i	5.02 CUSTOMER	Sevenso		
		,		Lier Superfund Site	 e
PROID DATE:	BASE HEIGH				<u></u>
DELINERY DATE:	TOP TY	PE Sp. Fr. & Gr. STR	RUCTURE#	<u> </u>	
COMMERCIAI		· .	/ 1		
PENNDOT		Star-s	Sea l	<u>_</u> 1	
PWD		#5 Gast	.80 .1	/	
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		Star-	Stal		
	•	#410			
		Ga3	, ·	·	
		Gas	het		
RISE	ER SECTIONS/CO	NVERSION LIDS	R" Tolet		
6" RISER	30" RISER	54" RISER	Thick Inht	— 57½" ——>	
12" RISER	36" RISER	60" RISER	100	<b></b>	
18" RISER	42" RISER	33X48 CONV. LID	412		-
24" RISER	48" RISER	4' X 4' CONV. LID		_ 34" 34"	
33X48 W/ ø	OPENING			<u> </u>	
	GRA	TES			
I-I-20	1-1-20	H-25 H-25			
STD PENNDOT	BIKE PWD	STD BIKE			
VANE GR	GRATT: 2X2 F/G	SID BIKE			
2X2 F/G STD.	BIKE				
ADDITIONAL IN	VFORMATION:				İ
26" x 36	" Special I	Frame & Grate			
<u>=0 x 00</u>	(TYPE)	1)			
	(1776)	~J	•		
				_	
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INLET PRODUCT	TION SHEET PAGE 8 OF 24
JOB QUOTE 11-87 STRUCTURE DEPTH 8.47 CUSTOMER	Sevenson Environmental
	Cornell-Dubilier Superfund Site
1	
DELIVERY DATE: TOP TYPE Sp. Fr+Gr. STF	RUCTURE#
COMMERCIAL Star-S	nec.
- 4 61-	
PWD Gas he	
943 170	· ·
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C/085	
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·	·
RISER SECTIONS/CONVERSION LIDS	4" Cal = 57 4" - >
6" RISER 30" RISER 54" RISER	1 57 4" ->
12" RISER   36" RISER   60" RISER   18" RISER   42" RISER   33X48 CONV. LID	15 Co. 2
18" RISER       42" RISER       33X48 CONV. LID         24" RISER       48" RISER       4' X 4' CONV. LID	24 36
33X48 W/ Ø OPENING	34" - 17
GRATES H-20 H-20 H-25 H-25	
STD BIKE STD BIKE	
PENNIXTT PWD 6' 6' VANIE GIR GRATTE STD BIKE	
2X2 F/G 2X2 F/G STD. BIKE	
ADDITIONAL INFORMATION:	
26" x 36" Special Frame & Grate	
(TYPE2)	
0	
Steps in Riser	

	·	INLET PRODUCTIO	ON SHEET	<del></del>	PAGE 9 OF 24
11-87	STRUKTURE DERINA	6.80 CUSTOMER -	Sevens	son Envir	
		JOB NAME			
				CR	22
DELIVERY DATE:	TOPTYPE	STRU	ICTURE#	(1)	
COMMERCIAL W		< 1"			
PENNDOT  PWD		1. 5		± *	
PWD		Low-Tap Hole	<i>و</i>	/	
		1901e			
Ctops -					
019			1		
		( - 1)			
		34" Low-10 "Hole			•
		Low-10	ape-		
		Hole			
picen cec	TELONIC/CONS/EDCI	ONLLIDS	a. c. kt		
	TIONS/CONVERSI RISER 54"	RISER	8" Thet hill TAPK	57¦"	
	RISER 60"	RISER	Car W		<del></del>
	<del></del>	48 CONV. LID			<b>个</b>
		4' CONV. LID		34"	24" 36"
33X48 W/ Ø OPEN	ATIACE				<u> </u>
	GRATES				
1 1 1	14-20 14-25 BIKE STD	14-25 BIKE			
1 1 1	PWD 6' GRATE STD	6' BIKE			
2X2 F/G	2X2 F/G				·
ADDITIONAL INFORM	BIKI: L	(TYPEZ)			
26" x 36" Sp					
20 K 30 SE	Journall I all	ic & Craic			
12) 011	DCV	01/7/	<u>-</u>		
(2) 34L	PSX-2	Boot 645/4			<u> </u>
			<del></del>	-	

INLET PRODUCTION	VSHEET PAGE 6 OF 24
11-87 STRUCTURE DEPTH 5. 25 CUSTOMER	
	ornell-Dubilier Superfund Site
DELIVERY DATIETOP TYPE Sp. Fr. + Gr. STRUC	TURE#
COMMERCIAL S	
PENNDOT [] STar-Se	۹ ا
PENNDOT D Star-Ser PWD D #580 Gas Ket	
Gas Ket	
4 . F	
2 1"	
34	
Low-Tape	Comment
34" Low-Tape Hole	
RISER SECTIONS/CONVERSION LIDS 45	79
6" RISER 30" RISER 54" RISER	572"
12" RISER 36" RISER 60" RISER "	TX
18" RISER 42" RISER 33X48 CONV. LID	34"
24" RISER   48" RISER   4' X 4' CONV. LID	24" 36"
33X48 W/ ø OPENING	<u> </u>
GRATES	
H-20 H-25 H-25	
STD BIKE STD BIKE — PENNIXIT PWD 6' 6'	
VANE GR.   GRATE   STD   BIKE	
STD. BIKE	
ADDITIONAL INFORMATION: (TYPE 1)	
26" x 36" Special Frame & Grate	
34 L PSX-2 Boot Gasket	
012 10 K- 0 1067 6431111	

			INLET PRO	DUCTION SHEE	et -	PA	.ge_11_0f24
JOB C XUOTES	11	-87_structure	DUPTH 4,01 CUSTO	omer Se	evensom	Environ	mental
PROID DATE:		BASE HEIGH					
DELI VERY D.	ATE:	TOP TYF	ESP. Fr. VGr.	_STRUCTURE	#	CB 20	2
COMMER	.ClA	L S	ı				
PENNDO	Γ						<u>.</u> -
PWD		<b>-</b> /					/
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125	ar	- Seal					
		10-5	$\sim$ 1	- ("			
(-	, a.5	Ket T	2	- ( <sub>0</sub>			
	•	((0)					
		L				]	
			. 1				
			Sta	ir-Seg			
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			6	1 580 -as Ket			
	USE	R SECTIONS/CON		1 4 7 Col	₹ .		•
6" RISER 12" RISER		30" RISER	54" RISER		, <del>K</del>	57 7"	<del>-&gt;</del>
18" RISER		36" RISER 42" RISER	60" RISER 33X48 CONV. LID		/		一不
24" RISER		48" RISER	4' X 4' CONV. LID			-34"	
33X48 W/	لبا ø	OPENING	1 7 77 1 001(7.111)			34"	36
<u></u>	<del></del> -						
		GRATI					
H-20 STD		H-20 BIKE	H-25 HKE				
PENND VANE C		. PWD GRATIE	6' 6' BIKE				
2X2 F/C		2X2 F/G	ואום ן טוב.				·
SID.	יו גו	BIKE	( soor A)				
			(TYPE2)				
<u>26" x 3</u>	6"	Special F	<u>rame &amp; Grate</u>	<u> </u>			
				<del></del>			İ

1			<u></u>
	INLET PRODUCTION	N SHEET	PAGE 12 OF 24
JOB CAUTE: 11-87STRUCTURE DE	PTH 4.41 CUSTOMER _	Sevensor	Environmental _
PROID DATEBASE HEIGHT:			er Superfund Site
			CO 19
DELIVERY DATE:TOP TYPE	Sp. Fr. of Gr. STRUC	TURE#	<u>C.B.11</u>
COMMERCIAL PENDOT  PWD			
	3'		Star-Seal # 410-5 Gasket
			-
RISER SECTIONS/CONVE	ersion lids &	41.	
<del></del>	54" RISER	y cor	574"——>
	60" RISER		
	4' X 4' CONV. LID		7
33X48 W/ ø OPENING	, , , , , ,	<b>←</b> —3	4" 36"
GRATES H-20 H-2	25 H-25		
STD   BIKIE   STI   PENNIXIT   PVD   6'   VANE GR.   GRATIE   STI   2X2 F/G   2X2 F/G   STD.   BIKIE	D BIKE —	,	
ADDITIONAL INFORMATION:	(TYPE2)		
26" x 36" Special Fra	ame & Grate		
			1

	INLET PRODUCTION SHEET	PAGE 13 OF 24
IOR CANDES 11-87 STRUCTURE DUPTH	6.92 CUSTOMER Seven	<del>_</del> -
	JOB NAME Cornell-Du	——·
_		
	). Fr. *Gr. structure#	(1) 10
COMMERCIAL		
PENNDOT  PWD	·	
	. /	
5/ex5_	1	Star-Seal
		# 795 Gas Ket
	33" y 48" × 60"	7/15
		Gas Ket
	34"	
	Low-Taper	•
DIGED SECTIONS/CONTINUES	ON 1100 0° 1:3	
RISER SECTIONS/CONVERSI 6" RISER 30" RISER 54"	ON LIDS & W	··· (e0"
	RISER	
	(48 CONV. LID	1'
24" RISER   48" RISER   4' X	14 CONV. LID	45"
		I15" \
GRATES H-20 H-25	H-25	
STD BIKE STD PENNDOT PWD 6'	BIKE -	
VANIE GR.         GRATE         STD           2X2 IVG         2X2 IVG	BIKE	
STID. BIKE	(NOTA)	
ADDITIONAL INFORMATION:	TYPE 1)	
26" x 36" Special Fram		
34L PSX-2 Boot		
33 × 48" × 6" Biser		

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			•	INLETPRODU	JCTION S	HEET	PAGE HOF2
(CAS & SIRSTER)	11	-87	1011 1011 1011	7.17 CHETTON	rn	Sevenso	n Environmental
300 00000							
PROID DATE		BASE I RE	IGITI:	JOB NAME	<u> </u>	<u>ווטוו-ווטווט</u>	ilier Superfund Site
DELI VERY DA	TΕ	TOP T	YPE T	7. Fr. + Gr.	STRUCTU	JRE#	CB 15
COMMERC		A	ĺ				
PENTNDOT		🗖	-	-51		1	· · · · · · · · · · · · · · · · · · ·
PWD				0197	- Oea	[	-
		/ –		Star Gas	7,95		/
				Gas	Ket		
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510	7						
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				1			
				Star-	Seq		
				<del>기</del> 「	795		
					Ket		
				(F¢5	Ket		
R	ISE	R SECTIONS/CC	) NVERSI	ON LIDS	4,	1 003	
6" RISER		30" RISER		RISER	y-u-	1/ 2003	•
12" RISER	X	36" RISER	60"	RISER	૽ૣૺૡ	/	7
18" RISER	_	42" RISER		48 CONV. LID			
24" RISER		48" RISER	4' X	4' CONV. LID	_		34" 24" 36"
33X48 W/	Ø	OPENING					
	·	GRA	TEC			<b>—</b>	- 57½">
11-20		11-20	H-25	11-25		•	4 1
STD PENND	YT.	BIKE	STD 6'	BIKE 6'			· · · · · · · · · · · · · · · · · · ·
VANEC	R.	GRATE	SID	BIKE			
2X2 I7G - S1D.		2X2 F/G BIKE				<u></u>	
ADDITIONAL	. INI		1/	YPE 2)			
2611 × 3	G"	Special		e & Grate	<del>-</del>		
<u> </u>	<u> </u>	opedial	Hall	ic & Chale	_		
					_		
Step	ic	Rison					
					<u> </u>	· · · · · · · · · · · · · · · · · · ·	<u>-</u>

	INLET PRODUCT	JON SHEET	PAGE 15 OF 24
JOB CHATT. 11-87 STRUCTURE DE	7.97 CUSTOMER	Sevensor	Environmental
			er Superfund Site
1			
DELIVERY DATE:TOP TYPE	Sp. Fr. +Gr. STR	RUCTURE#	CB 14
COMMERCIAL S	71	- 1	
PENNDOT [	Star-S		
PWD 🗆	# 7° Gask	15 <sub>,</sub> /	
	G-95 K	iet	
[			٦
Steps _			
7			
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·			]
	Star-S		
	#58C	)eq	
	# 08C	7	
	Gask	et	
RISER SECTIONS/CONVE	EDSIONT IDS	0" - 2	
	54" RISER	8 X D	
<u> </u>	60" RISER	CAR	
<del></del>	33X48 CONV. LID		34"
	4' X 4' CONV. LID		24 36"
33X48 W/ ø OPENING			
GRATES		~	571/>
H-20 H-20 H-3	1 1 ! ! ! !	,	- 14 /1
PENNIXOT PWD 6'	6'		
VANIE GR.         GRATE         STI           2X2 I/G         2X2 F/G	D BIKI:		
STD. BIKE	(/// 1)		
ADDITIONAL INFORMATION: .	(TYPE 2)		
26" x 36" Special Fra	ame & Grate		
Steps in Riser			

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INLET PRO	DUCTION SHEET PAGE 16 OF 24
9 47	Sevensor Environmental
	<del></del>
	Cornell-Dubilier Superfund Site
DELIVERY DATETOP TYPETOP. Fr + Gr.	_STRUCTURE# CB LB
COMMERCIAL C	L < 1
PENNDOT >/	ar-Seal
PWD /	#580 Gasket
	ogs her
	<1 <1
Steps-	5tar-Segl #410-5 Gas Ket
	<u></u> 410-5
	) Gasket
< 1	
Star	Seal 0-5
#41	0-5
G-q.	sket
RISER SECTIONS/CONVERSION LIDS	4" 1 20
6" RISER 30" RISER 54" RISER	4" \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
12" RISER 36" RISER 60" RISER	
18" RISER         42" RISER         33X48 CONV. LID           24" RISER         48" RISER         4' X 4' CONV. LID	
33X48 W/ Ø OPENING	34" 34" 34"
GRATES  H-20 H-20 H-25 H-25	577 7>
STD BIKE STD BIKE PENNIXOT PWD 6' 6'	
VANEGR. GRATE STD BIKE	
2X2 F/G   2X2 F/G   STD.   BIKE	
ADDITIONAL INFORMATION: (TYPE2)	
26" x 36" Special Frame & Grate	<u> </u>
Steps in Riser	

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INLET PRODUCT	TION SHEET PAGE 17 OF 24
JOB CATOTE: 11-87 STRUCTURE DEPTH 3.50 CUSTOMER	
PROID DATEBASE HEIGHT:	•
DELIVERY DATETOP TYPE Sp. Fr. + Gr. STI	RUCTURE#CD 13/1
COMMERCIAL	
PENNDOF	<del>۔ '</del>
PWD /	/
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Sta	S- 1
Star. # 410 - Gas K	- Olay I
# 410 -	
Gas K	ct.
RISER SECTIONS/CONVERSION LIDS	The cast
6" RISER 30" RISER 54" RISER	CE VI VI
12" RISER 36" RISER 60" RISER	CR T
18" RISER       42" RISER       33X48 CONV. LID         24" RISER       48" RISER       4' X 4' CONV. LID	24" 36"
33X48 W/ Ø OPENING	34"
GRATES H-20 H-25 H-25 H-25	574"
STD BIKE STD BIKE	
PENNIXIT PWD 6' 6' VANE GR GRATTE STID BIKE	
2X2 F/G 2X2 F/G 5TID. BIKIF.	
ADDITIONAL INFORMATION: (TYPE 2)	
26" x 36" Special Frame & Grate	
20 x 00 oposiai i famo a orato	

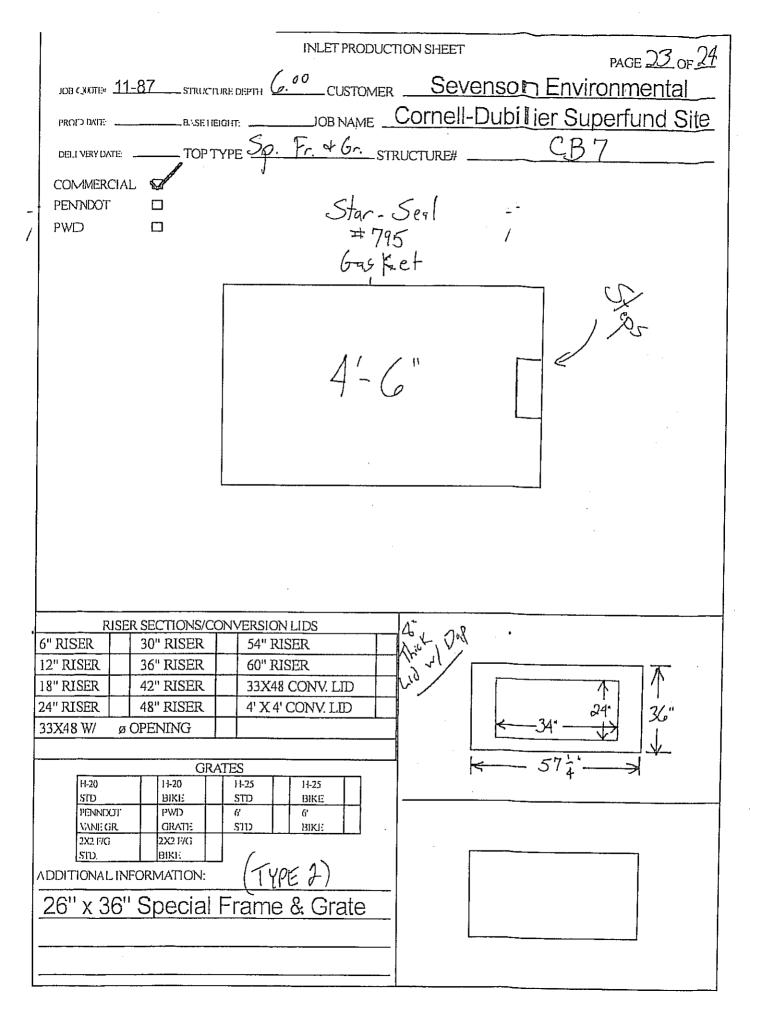
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	INLET PRODUC	CTION SHEET PAGE 18 OF 24
11 97	4 17	R Sevenson Environmental
JOB QUOTIN <u>LIPOZZ **</u> S		
PRODUNTEE	ASE HEIGHT:JOB NAME _	Cornell-Dubilier Superfund Site
DELIVERY DATE:	TOP TYPE Sp. Fr. + Gr. ST	TRUCTURE# CB 17
	1	
COMMERCIAL ST	Stor-S #41 Gast	Seal
PENNDOT	+ 11	10 <
PWID	T 7/	
	G95	Ket
	2	
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•		
	NS/CONVERSION LIDS	15 L D
6" RISER 30" RISE		45 7 0 X
12" RISER   36" RISE		
18" RISER   42" RISE 24" RISER   48" RISE		
24" RISER   48" RISE 33X48 W/ Ø OPENING	······································	34" 24" 36"
OPERATION IN BEACE		
	GRATES	571
11-20 H-20 5TD BIKE	11-25 11-25	1 - 4 >1
PENNDOT PWD	5'TD BIKE 6' 6'	
1. ANE GR. GRATE 2X2 F/G 2X2 F/G		
STD. BIKE		
ADDITIONAL INFORMATIO	IN: (TYPE2)	
26" x 36" Speci	ial Frame & Grate	
	a. I Idiilo de Oldio	-
		<u> </u>

n.	MANHOLE PRODUCTION SHEET - R	EN A DIDEIDS
11		PAGE 16 OF det
JOB CHART 11-87 STRUCTURE DEP	u 4.90 customer Seve	enson Environmental
		ell-Dubi lier Superfund Site
PROD DATEBASE HEIGHT: _		
DELIVERY DATE:FLOW CHANNELS	YES NO STRUCTURE#	CBIZ
COMMERCIAL		
		· · · · · · · · · · · · · · · · · · ·
PENNDOT		
P.WD □ ′		/
	11	12 1
		$\sum_{i}$
Star-	$\leq 1$ / 10	2
	,	
# T	79.59	2' 3
		)
Gasl	Set +	7
	8	$4\sqrt{SLCI}$
	7	otor-seal
	//	6 3 #580
		6 \$ #580 (Fas Kef
		0 45 1167
		•
1'-0" MANHOLE RISER [48" DIA]	MH FLAT LID ( W/24"0) [48" DIA.]	MH FLAT 1.1D (W/24"o) [60" DIA.]
2'-0" MANHOLE RISER [48" DIA]	MITI-LAT LID (W30"o) [48"DIA.]	MH FLAT LID (W/30"o) [60" DIA.]
3'-0" MANHOLE RISER [48" DIA] 4'-0" MANHOLE RISER [48" DIA]	2'-0" REDUCER CONE [60"TO48"]	1'-0" MANIHOLE RISER [72" DIA]
2'-0" MH CONE (W/24"o)	1'-0" MANHOLE RISER [60" DI.\]	2'-0" MANIHOLE RISER [72" DIA] 3'-0" MANIHOLE RISER [72" ID]
2'-0" MH CONE (W/30"0)	2'-0" MANHOLE RISER [60" DIA]	4'-0" MANHOLE RISER [72" ID]
3'-0" MH CONE (W/24"a)	3'-0" MANHOLE RISER [60" DIA]	MH FLAT I_ID (W/24"o) [72" DIA.]
3'-0" MH CONE (W/30"6)	4'-0" MANI IOLE RISER [60" DIA]	MH FLAT LID (W:30"o) [72" DIA.]
NO EXTERIOR COATING EXTERIOR CO	DATING (BLACK)	:
ENTERIOR EPONY (TAN) INTERIOR EPO	DYJ. (MAILLE)	· ·
NOMH FRAME & COVER 24" MH F.C."S	TORM"	
NIFIT SEALANT (ROLLS) 30° MH F/C ST	DRM	/ / 11 12 1/ >
M.H. FRAME/COVER		
ADDITIONAL INFORMATION:		
	- 1	->\(\frac{12}{9}\) \( \tag{2}\)
DG" x 36" Special F	r. e Grate	\ <del>\                                  </del>
(14)	<u> </u>	4
	,	× / <sup>7</sup> 6 5 ×
	18.11.	

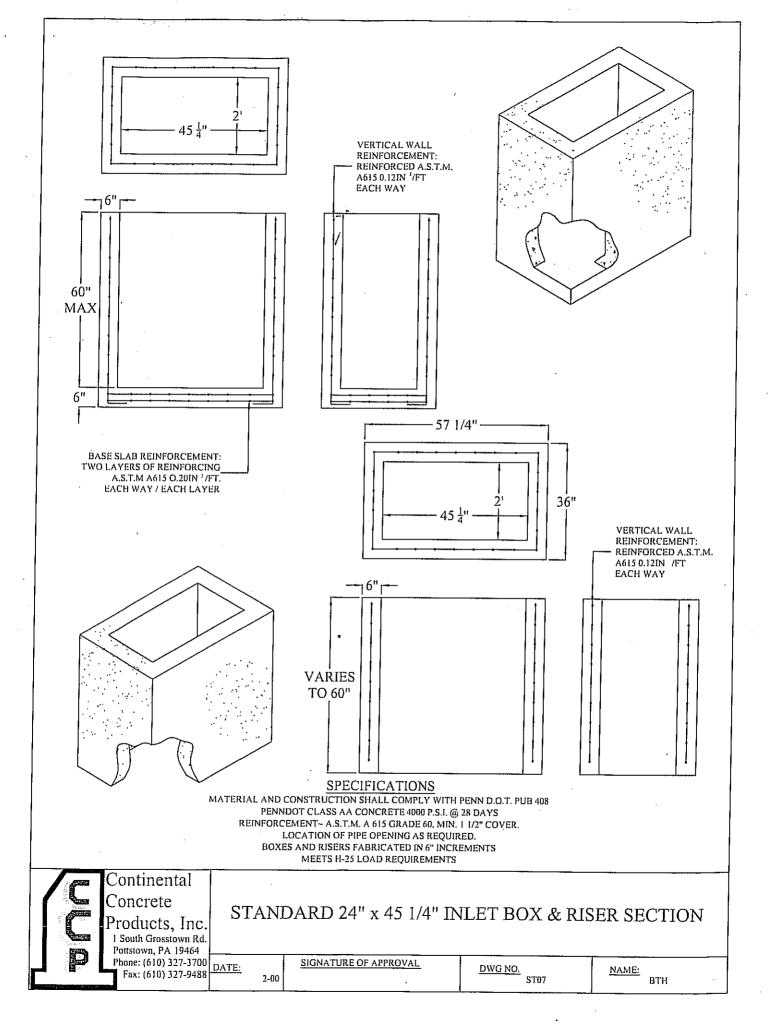
	INLET PRODUCTION SHEET	PAGE 20 OF 24
лон слютел <u>11-87</u> structu	REDIPTU 650 CUSTOMER Sevensor	
1	GIR:JOB NAME _Cornell-Dubili	
l l		
DELI VIRYDATETOP TY	PE Sp. Fr & Gr. STRUCTURE#	CB II
COMMERCIAL S		
PĒ∕NDOT □	Star-Seal	<u>-</u> -
PWD =	# 580	/
	# 580 Gas Ket	
		_
Ster J		
	$\leq 1$	
	Star-Seal # 410-S	
	Gasket	
RISER SECTIONS/CO		
6" RISER 30" RISER		574"
12" RISER 36" RISER	60" RISER CQ	
18" RISER 42" RISER	33X48 CONV. LID	<u> </u>
24" RISER   48" RISER	4' X.4' CONV. LID	- 34"   36" - 34"   1
33X48 W/ ø OPENING		
GRA	TES TES	
H-20 H-20	H-25 H-25	
LENNIXAL LAMB	STD BIKE 6' 6'	
VANE GR.   GRATE   2X2 F/G   2X2 F/G	STD BIKE	
STD. BIKI:	( ( )	
ADDITIONAL INFORMATION:	(TYPE 2)	
26" x 36" Special F	Frame & Grate	
<u> </u>		

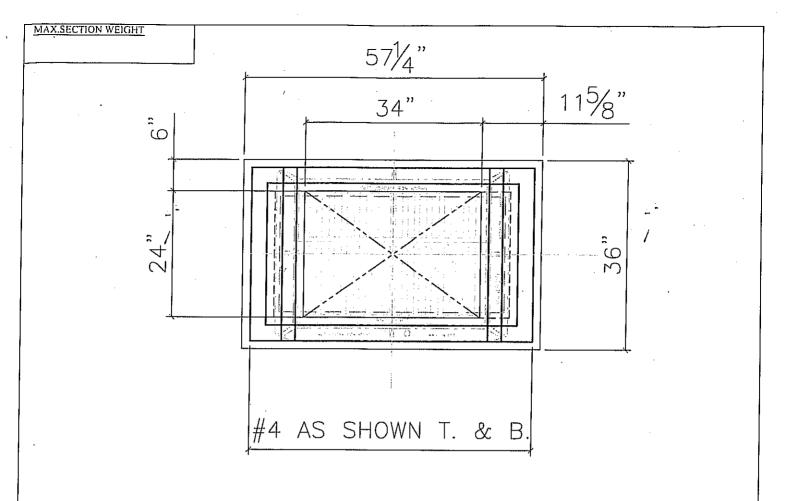
1			UCTION SHEET	PAGE 2/OF	:14
лов слите 1	1-87_structur	REDEPTH 4.80 CUSTOM	ier <u>Sever</u>	son Environmental	
PROID DATE:	BASE HEIC	om IOB NAME	Cornell-D	ubilier Superfund Si	ite
		PE Sp Fr. + Gr.		<u>CB</u> 10	
	<b>A</b>	PE 01 17. 01.	STRUCTURE#	<u> 45 10 </u>	
COMMERCIA		<1	<i>-</i> 1		
PENNDOT PWD		Otac	= Deal		
PWD	L_1	# 4	= Seal  10-5  Ket		
		6-qs	Ket		
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	ER SECTIONS/CO		45. CP.		
6" RISER	30" RISER	54" RISER	4 CR		
6" RISER 12" RISER	30" RISER 36" RISER	54" RISER 60" RISER	Mint Day		
6" RISER	30" RISER	54" RISER 60" RISER 33X48 CONV. LID	Air Day	A	
6" RISER 12" RISER 18" RISER 24" RISER	30" RISER 36" RISER 42" RISER	54" RISER 60" RISER	Si Co	34' 34' 36"	
6" RISER 12" RISER 18" RISER 24" RISER	30" RISER 36" RISER 42" RISER 48" RISER OPENING	54" RISER 60" RISER 33X48 CONV. LID 4' X 4' CONV. LID	Air Dur	34' - 17	
6" RISER 12" RISER 18" RISER 24" RISER	30" RISER 36" RISER 42" RISER 48" RISER	54" RISER 60" RISER 33X48 CONV. LID 4' X 4' CONV. LID	Aint Day		
6" RISER 12" RISER 18" RISER 24" RISER 33X48 W/ g	30" RISER 36" RISER 42" RISER 48" RISER FORENING  GRAT H-20 BIKI:	54" RISER 60" RISER 33X48 CONV. LID 4' X 4' CONV. LID  ES 11-25 STD 14-25 BIKE	8 CD	34' - 17	
6" RISER 12" RISER 18" RISER 24" RISER 33X48 W/ Ø  H-20 STID PFNNEXIT VANE GR.	30" RISER 36" RISER 42" RISER 48" RISER FOR OPENING  GRAT H-20 BIKH: PWD GRATE	54" RISER 60" RISER 33X48 CONV. LID 4' X 4' CONV. LID  ES 11-25 11-25 11-25	A COR	34' - 17	
6" RISER 12" RISER 18" RISER 24" RISER 33X48 W/ g	30" RISER 36" RISER 42" RISER 48" RISER FOR OPENING  GRAT H-20 BIKI: PWD	54" RISER 60" RISER 33X48 CONV. LID 4' X 4' CONV. LID  ES H-25 STD BIKE 6' STD BIKE	Si Col	34' - 17	
6" RISER 12" RISER 18" RISER 24" RISER 33X48 W/ Ø 11-20 STID PFENEXAT VANE GR. 2X2 17G	30" RISER 36" RISER 42" RISER 48" RISER GRAT H-20 BIKI: PWD GRATE 2X2 P/G BIKE	54" RISER 60" RISER 33X48 CONV. LID 4' X 4' CONV. LID  ES H-25 STD BIKE 6' STD BIKE	Air Din	34' - 17	
6" RISER 12" RISER 18" RISER 24" RISER 33X48 W/ Ø  H-20 STD PENNEXAT VANIE GR. 2X2 17G STD. ADDITIONAL IN	30" RISER 36" RISER 42" RISER 48" RISER 6 OPENING GRAT H-20 BIKI: PWD GRATE 2X2 F/G BIKI: NFORMATION:	54" RISER 60" RISER 33X48 CONV. LID 4" X 4" CONV. LID  TES H-25 STD BIKE 6" STD BIKE (TYPE 2)	Air Day	34' - 17	
6" RISER 12" RISER 18" RISER 24" RISER 33X48 W/ Ø  H-20 STD PENNEXAT VANIE GR. 2X2 17G STD. ADDITIONAL IN	30" RISER 36" RISER 42" RISER 48" RISER 6 OPENING GRAT H-20 BIKI: PWD GRATE 2X2 F/G BIKI: NFORMATION:	54" RISER 60" RISER 33X48 CONV. LID 4' X 4' CONV. LID  ES H-25 STD BIKE 6' STD BIKE	45 CD	34' - 17	

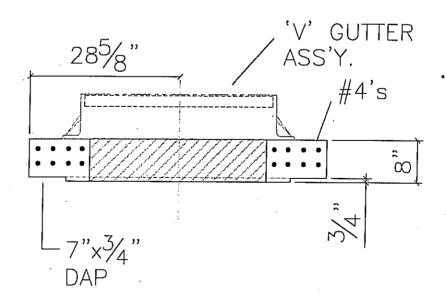
1		_				
ļ			INLET PROD	DUCTION SHEET		PAGE 22 OF 24
ICES C MAZILES	11-87 em	TELIBLE DEDTT. 5	82 CUETON	Seve	nson Enviro	nmental
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PROLO DATE		HEIGHT:			Oubilier Sup	
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COMMERC						
PENNDOT	-	•				
PWD	_ /					/
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R	ISER SECTIONS/	CONVERSION	LIDS	0, 00	<b>21</b>	
6" RISER	30" RISER	54" RIS	SER	Win 100	6	•
12" RISER	36" RISER	60" RIS	SER		<u>\</u>	<del></del>
18" RISER	42" RISER		CONV. LID		1 7 1	
24" RISER	48" RISER	4' X 4'	CONV. LID	<u>                                     </u>	34"	45"
33X48 W/	ø OPENING				<u> </u>	
	G	RATES	<del></del>		(0"	
11-20	11-20	H-25	H-25		<i>─ ω</i> " <i>─</i>	
STD PENNIX	DT PWD	STD 6'	BIKE 6'			
VANIE G	R. GRATE	SID	BIKE			:
2X2 F/G STD.	2X7 F/G BIKE					
<del></del>	INFORMATION:	- (TYI	pc1)			
<u> 20 X 3</u>	6" Specia	riame	<u> </u>	_		
					-	
405	P5x-2	Boot	Gasket			



1		
	INLET PRODUCTION SHEET	PAGE 24 OF 24
JOH CHOTE: 11-87 STRUC	TURE DEPTH 4.50 CUSTOMER Seve	enson Environmental
PROID DATE:BASE H		Dubilier Superfund Site
	A	CR 9
DELIVERY DATE:TOP	TYPE Sp. Fr. 4 (r. structure# _	
COMMERCIAL D		
PENNDOT   PWD	<del></del>	
	/	
	5	<b>)</b>
	$\epsilon 1 - \epsilon 1$	
	0197-5eg1	
	#580	
	Star-Seal #580 Gas Ket	
RISER SECTIONS/C		
6" RISER 30" RISER	ONVERSION LIDS  54" RISER  60" RISER	
12" RISER 36" RISER	60" RISER	
18" RISER 42" RISER	33X48 CONV. LID	<b> </b>
24" RISER 48" RISER	4' X 4' CONV. LID	34" 34"
33X48 W/ ø OPENING		34"
GR	ATES	< 51;" ->
11-20 11-20	H-25 H-25	7
PENNDOT PWD	STID   BIKE	
VANE GR. GRATE 2X2 F/G 2X2 F/G	STD BIKE	
STD. BIKE		
ADDITIONAL INFORMATION:	(TYPE 1)	
26" x 36" Special	Frame & Grate	







4/11/11

### **SPECIFICATIONS**

- \*MATERIAL AND CONSTRUCTION SHALL COMPLY WITH NJ D.O.T.
- \*NJ DOT CLASS B CONCRETE 4500 P.S.I. @ 28 DAYS
- \*REINFORCEMENT~ A.S.T.M. A615 GRADE 60, MIN. 1 1/2" COVER U.N.O.



Continental Concrete Products, Inc.

1 South Grosstown Rd. Pottstown, PA 19464 Phone: (610) 327-3700 DATE

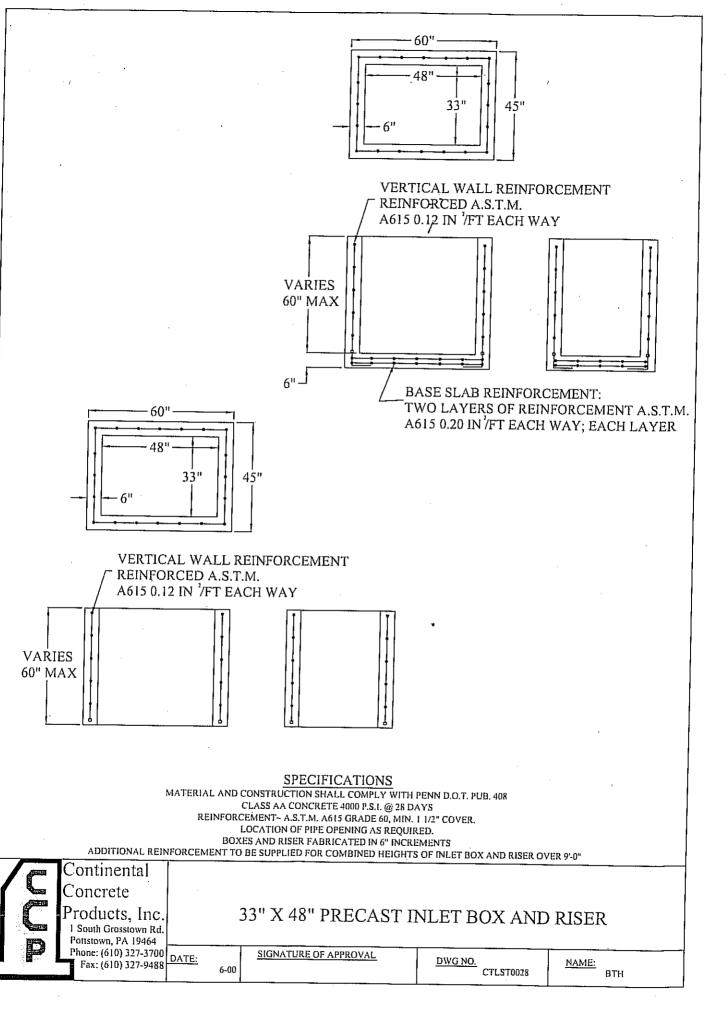
Fax: (610) 327-9488

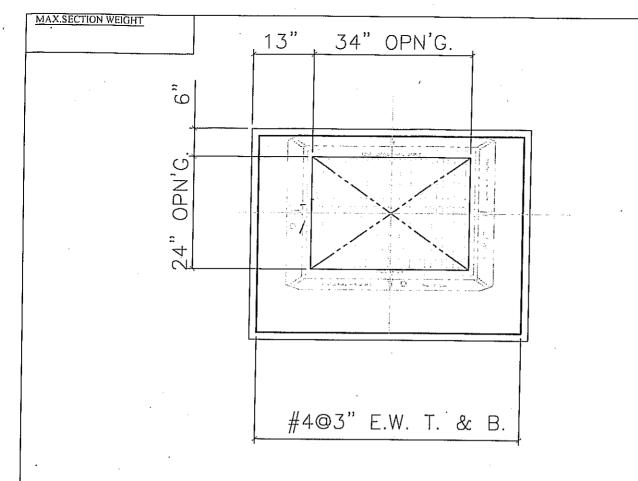
CORNELL DUBILIER SUPERFUND SITE SOUTH PLAINFIELD, N.J. SEVENSON ENVIREMENTAL SERVICES

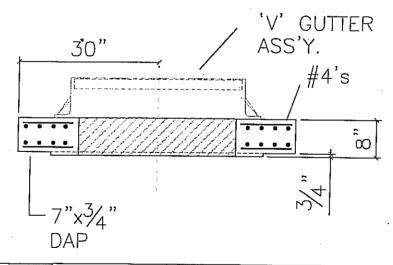
2'x4' INLET LID RRP DWG

1187-24

STRUCTURE **MULTI** 







\*MATERIAL AND CONSTRUCTION SHALL COMPLY WITH NJ D.O.T. \*NJ DOT CLASS B CONCRETE

4500 P.S.I. @ 28 DAYS \*REINFORCEMENT~ A.S.T.M. A615 GRADE 60, MIN. 1 1/2" COVER U.N.O.



Continental Concrete

Products, Inc. I South Grosstown Rd.

Pottstown, PA 19464 Phone: (610) 327-3700 DATE Fax: (610) 327-9488

CORNELL DUBILIER SUPERFUND SITE SOUTH PLAINFIELD, N.J. SEVENSON ENVIREMENTAL SERVICES

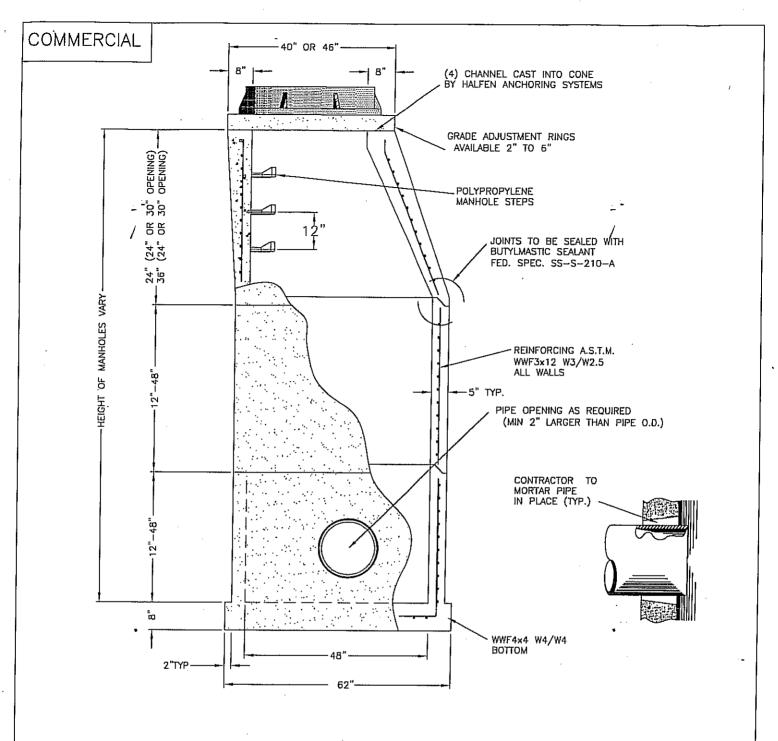
33"'x48" INLET LID

RRP DWG

4/11/11

1187-816

STRUCTURE 8,16



MINIMUM CONCRETE STRENGTH 4000 PSI @28 DAYS (PENN DOT CLASS AA)

STEEL REINFORCEMENT —ASTM A185 VERTICAL WALLS STEEL AREA 0.12 IN.2/VERTICAL FOOT.

HORIZONTAL BASE SLAB STEEL AREA 0.13 IN.2/HORIZONTAL FOOT.

1 1/2" MINIMUM CONCRETE COVER (ALL REINFORCEMENT)

MANHOLE DESIGN SPECIFICATIONS CONFORMS TO "PRECAST REINFORCED

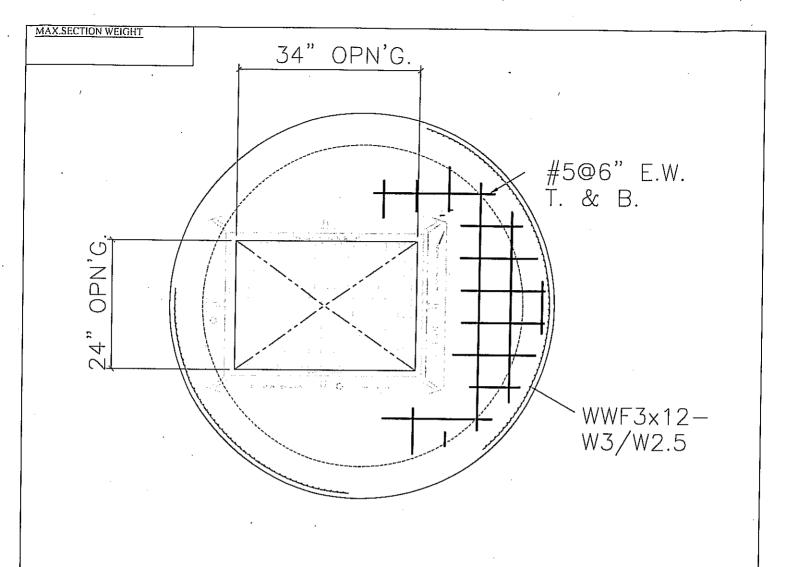
CONCRETE MANHOLE SECTIONS" ASTM C478, LATEST REVISIONS

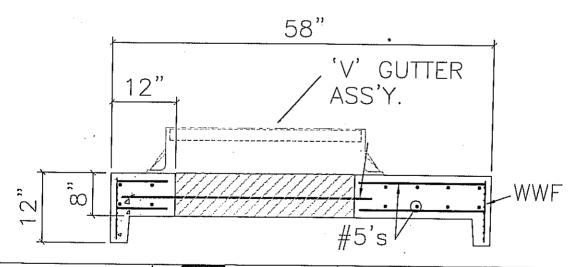
.



# 48" DIA. STORM MANHOLE

)	SIGNATURE OF APPROVAL	DATE RRP	DWG	STRUCTURE
1		9/23/04	48STMHCOMM	





- \*MATERIAL AND CONSTRUCTION SHALL COMPLY WITH NJ D.O.T. \*NJ DOT CLASS B CONCRETE 4500 P.S.I. @ 28 DAYS
- \*REINFORCEMENT~ A.S.T.M. A615 GRADE 60, MIN. 1 1/2" COVER U.N.O.



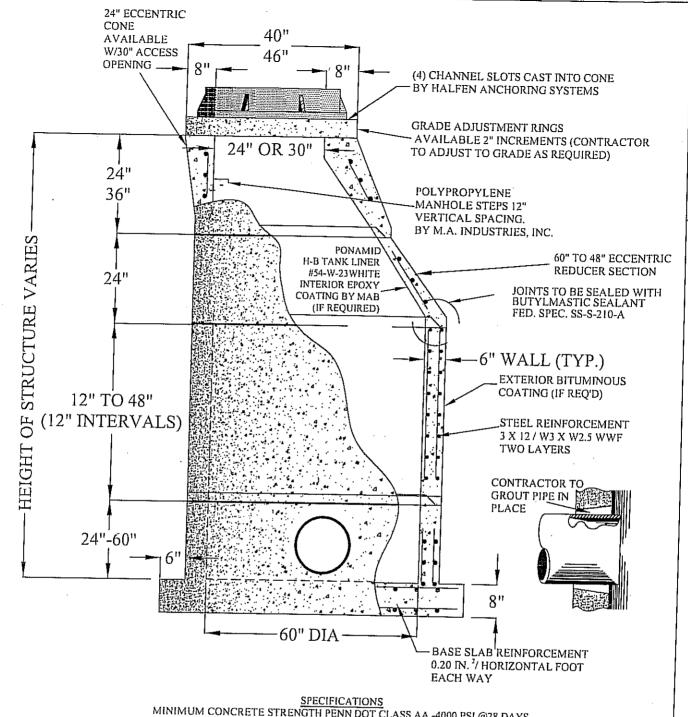
Continental Concrete

Products, Inc. 1 South Grosstown Rd. Pottstown, PA 19464

Fax: (610) 327-9488

CORNELL DUBILIER SUPERFUND SITE SOUTH PLAINFIELD, N.J. SEVENSON ENVIREMENTAL SERVICES ø48" INLET LID

RRP DWG Phone: (610) 327-3700 DATE STRUCTURE 4/11/11 1187-4A **4A** 



MINIMUM CONCRETE STRENGTH PENN DOT CLASS AA -4000 PSI @28 DAYS

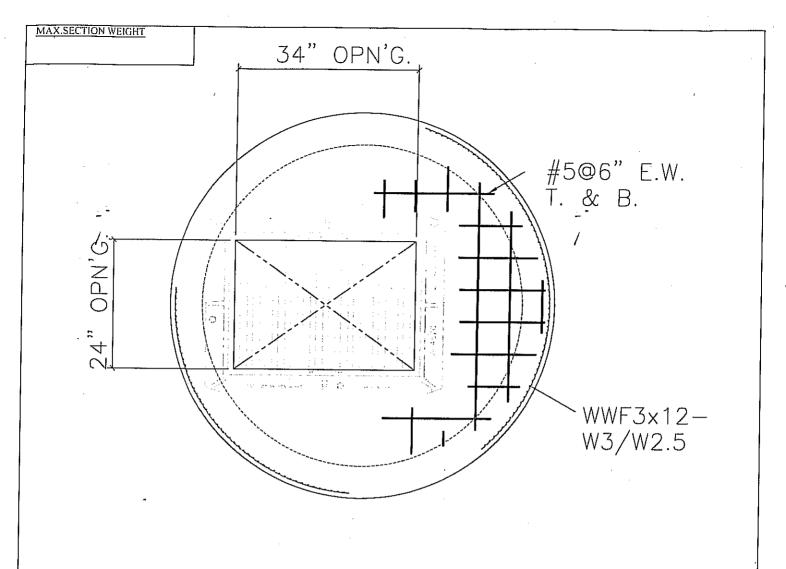
STEEL REINFORCEMENT -ASTM A185 VERTICAL WALLS STEEL AREA 0.12 IN 2/VERTICAL FT.-(48" DIA.) & 0.15 fN<sup>2</sup>/VERTICAL FT. -(60"DIA.)HORIZONTAL BASE SLAB STEEL AREA 0.20 IN <sup>2</sup>/HORIZONTAL FT. (1 1/2" MINIMUM CONCRETE COVER)

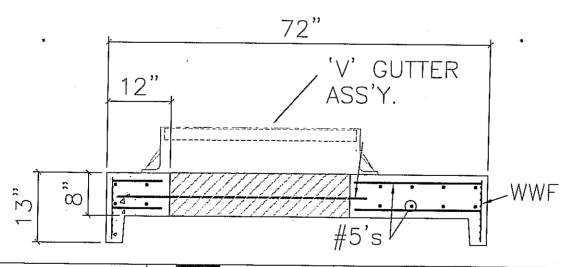
> MANHOLE DESIGN SPECIFICATIONS CONFORMS TO "PRECAST REINFORCED CONCRETE MANHOLE SECTIONS" ASTM C478, LATEST REVISIONS



60" DIA. STORM MANHOLE w/6" EXT. BASE

Rd. 4					
700 188	DATE:	2-00	SIGNATURE OF APPROVAL	DWG NO. 60STMH6X	NAME: BTH





- \*MATERIAL AND CONSTRUCTION SHALL COMPLY WITH NJ D.O.T.
- \*NJ DOT CLASS B CONCRETE 4500 P.S.I. @ 28 DAYS
- \*REINFORCEMENT~ A.S.T.M. A615 GRADE 60, MIN. 1 1/2" COVER U.N.O.



Continental Concrete Products, In

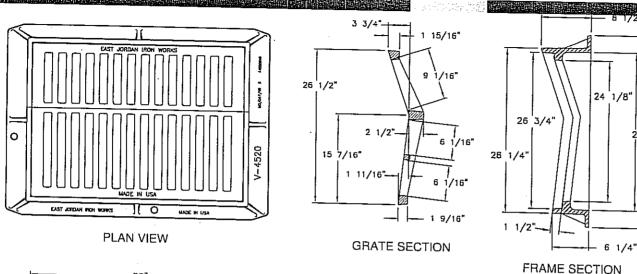
Products, Inc.
1 South Grosstown Rd.
Pottstown, PA 19464
Phone: (610) 327-3700

Fax: (610) 327-9488

CORNELL DUBILIER SUPERFUND SITE SOUTH PLAINFIELD, N.J. SEVENSON ENVIREMENTAL SERVICES Ø60" INLET LID

DATE | RRP DWG | STRUCTURE | 4/11/11 | 1187-4 | 4

# V4520 V4520-1 ASSEMBLY



### PRODUCT NUMBER 44520001

### **DESIGN FEATURES**

### MATERIALS

33"

27

**GRATE-GRAY IRON** ASTM A48 CL35B

FRAME-GRAY IRON ASTM A48 CL35B

### **DESIGN LOAD** HEAVY DUTY

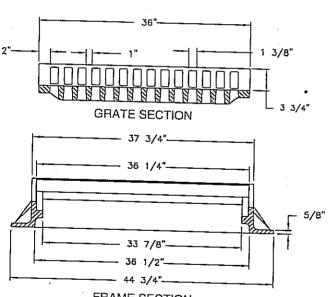
### COATING

### UNDIPPED

### **OPEN AREA**

458 SQ. INCHES

√ DESIGNATES MACHINE SURFACE



FRAME SECTION

Uncontrolled distribution.

Corporate Headquarters 301 Spring Street PO Box 439 East Jordan, MI 49727-0439 800.874.4100



Call Today for More Information

Weights (lbs./kg.) dimensions (inches/mm., and drawings provided for your guidance.

•We reserve the right to modify specifications without prior notice.

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### REFERENCE INFORMATION

44520130 44520010

### DRAWING DETAILS

ORIGINAL DRAWING: DEW 11/11/03

REVISED BY: DEW 10/29/10



# MultiSeal, Inc. Designed Scalant and Adhesive Technologies

Corporate Office & Mfg. R & D / Compounding. 4320 Hitch and Peters Road Evansville, IN 47711 •Tel. (\$12) 428-3422 • Fax. (\$12) 428-3432 2131 Commercial Ct. Evansville, IN 47720 •Tel. (\$12) 428-3443 • Fax. (\$12) 428-3445

MP1255

# TECHNICAL DATA

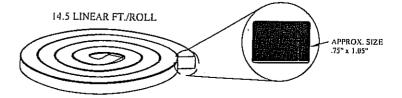
Page Lof1

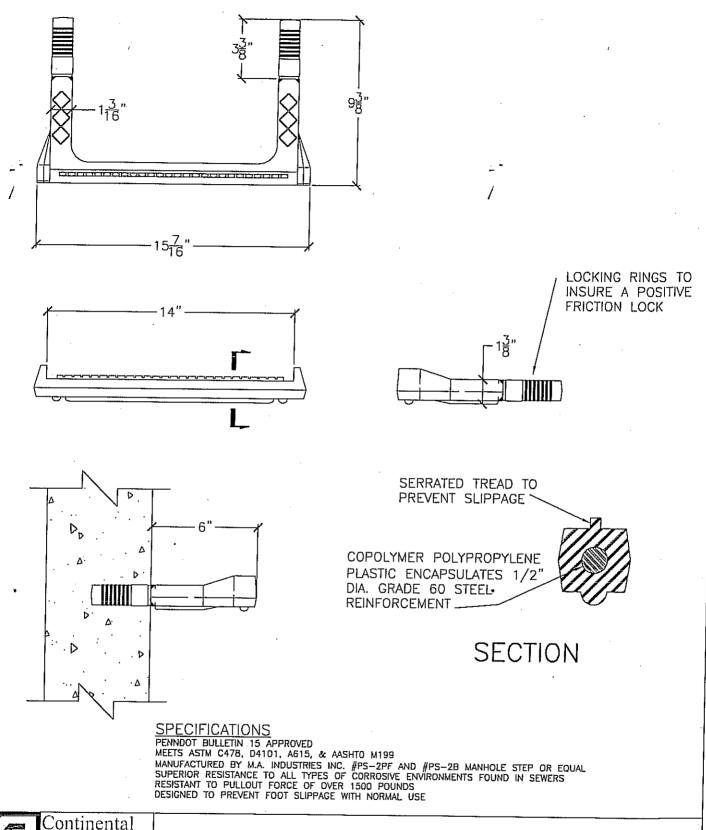
Product number:	MP1255					
Description = -	Higher density polymer-based mastic used to seal most clean					
,	substrates against moisture, dust, and air, Substrates include					
	concrete, metal, plastic, and other surfaces.					
ASTM C990	Meets ASTM C990 specification for Butyl Rubber Sealant					
-	Specification Requirement	Typical Results				
Butyl Rubber (hydrocarbon blends)	50 % minimum	54 %				
Ash-Inert Mineral Matter	30% minimum	44				
Volatile Matter	2 % maximum	< 2 %				
	·					
Specific Gravity at 77°F	1.15 minimum, 1.50 maximum	1.36				
Ductility at 77°F, cm	5.0 minimum	8				
Flash Point, C.O.C.	350 °F minimum	>500 °F				
Fire Point, C.O.C.	375 °F minimum	>500 °F				
Rebound Test at 77°F	3 % to 15 %	5.2				
at 32°F	3 % to 20 %	5.8				
Compression Test at 77°F, lbf/ln. <sup>2</sup> at 32°F, lbf/in. <sup>2</sup>	100 maximum	91				
at 32°F, lbf/in.2	200 maximum	128				
Low Temperature Flexibility at -10°F	180° bend, no cracking and no	Pass				
	loss of adhesion	1.05				
Elevated Temperature Flow, 14 days at	No sag. and no change in	Pass				
158°F	extruded shape	2 100				
	***************************************					
Adhesion after impact	No greater loss than 50 % of	Pass				
<u> </u>	ndhesion					
Cone Penetration at 77°F, dmm	50 to 100	80-95				
at 32°F, dmm	40 minimum	50				
Chemical Resistance	No deterioration, cracking, or	Pass				
	swelling .					
Shelf life	Minimum I year					
		. 1				



Continental
Concrete
Products, Inc.

1 South Grosstown Rd. Pottstown, PA 19464 Phone: (610) 327-3700 Fax: (610) 327-9488





POLYPROPYLENE MANHOLE AND INLET STEPS

RRP DWG

STEPS

STRUCTURE

DATE

3/27/06



Concrete

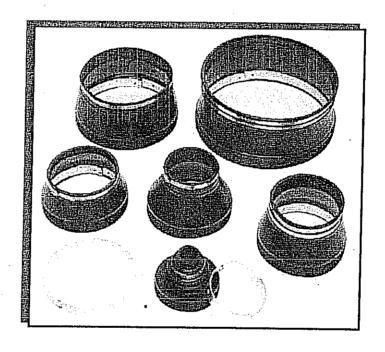
Products, Inc. I South Grosstown Rd. Pottstown, PA 19464

Phone: (610) 327-3700

Fax: (610) 327-9488

SIGNATURE OF APPROVAL

# POSITIVE SEAL GASKET SYSTEM WITH POWER SLEEVE EXPANSION



### **PSX**

Our original PSX; POSITIVE SEAL pipé-to-manhole flexible connector system. PSX is available for 8" and larger holes to seal the most commonly used pipe types and sizes.

### **PSX ADVANTAGES**

\* Meets and/or exceeds Material Specifications of ASTM C-923.

\* Type 304 Stainless Steel Compression Power Sleeve is one piece with no welds. Made with 10 and 11 gauge steel. 
\* Highest installation pressures, 2500 to 6000 PSI. The more PSI force, the better the initial and long term seal. 
\* Gasket is made of high quality Polyisoprene rubber. Provides greater deflection capabilities and greater tear resistance.

\* Type 304 Stainless Steel Take-Up Clamps.
\* For use with cored holes, or our fiberglass, or Pro-Former Hole Formers.

TEST /	ASTM METHOD	TEST REQUIREMENTS	TEST RESULTS
CHEMICAL RESISTANCE; 1N SULFURIC ACID 1N HYDROCHLORIC ACID	D 534, AT 22°C FOR 48 HRS	NO WEIGHT LOSS NO WEIGHT LOSS	NO WEIGHT LOSS NO WEIGHT LOSS
TENSILE STRENGTH	D 412	1200 PSI, MIN.	2600 PSI
ELONGATION AT BREAK	D 412	350%, MIN.	675%
HARDNESS	D 2240 (SHORE A DUROMETER)	±5 FROM:THE  MANUFACTURER'S  SPECIFIED HARDNESS	45
ACCELERATED OVEN-AGING	D 573, 70± 1°C FOR 7 DAYS	DECREASE OF 15%, MAX. OF ORIGINAL TENSILE STRENGTH, DECREASE OF 20%, MAX. OF ELONGATION	-13% TENSILE CHANGE, -14% ELONGATION CHANGE
COMPRESSION TEST	D 395, METHOD B, AT 70°C FOR 22 HRS	DECREASE OF 25%, MAX. OF ORIGINAL DEFLECTION	13.20%
WATER ABSORPTION	D 471 IMMERSE 0.75 BY 2-IN. SPECIMEN IN DISTILLED WATER AT 70°C FOR 48 HOURS	INCREASE OF 10%, MAX. OR ORIGINAL BY WEIGHT	3.50%
OZONE RESISTANCE	D 1171	RATING 0	PASS
LOW-TEMP, BRITTLE POINT	D 746	NO FRACTURE AT -40°C	PASS
TEAR RESISTANCE	D 624, METHOD B	200 LBF/IN. (MIN.)	318 LBF/IN.

#### PIPE INSTALLATION

Clean pipe and boot to ensure no dirt or foreign materials are present.

Clamping surface on pipe must be clean and smooth.

 Clamping surface on pipe must be clean and smooth.
 Center pipe in opening and insert until pipe breaks the inside plane of manhole.
 Attach take-up clamp(s) and stagger screw(s) of clamp(s) around the groove of the gasket so that take-up pressure will be equalized. Make sure each clamp is completely in the correct groove.
 Using a torque ratchet or torque wrench, gradually tighten all screw(s) of clamp(s) in an alternating pattern to 60lbs/in torque.
 After reaching 60lbs/in torque on final screw, check all screws again to ensure equal compression of all clamps.
 Vacuum testing shall be conducted in accordance with ASTM C-1244-02.
 Adjust pipe to line and grade. Use proper bedding, backfill materials and techniques so that pipe deflection and deformation is minimized. Installation of the concrete structure shall be such that differential settlement between the structure and the pipeline shall be less than 10% of pipe diameter for pipes less than 20" and shall be less than 5% of pipe diameter for pipes between 20 and 60 inches in diameter. and 60 inches in diameter.

9. Any pipe stubs installed in the manhole must be positively restrained from movement per ASTM C-923. Press-Seal is not responsible for blow outs due to unrestrained pipe stub or future lateral connections.

Before using the PSX-POSITIVE SEAL system for any custom applications, contact our Customer Service Department for more information,

U.S. Patent No.'s 4215868, 4478437 Copyright 1994 by Press-Seal Gasket Corporation (July 2002)

Press-Seal believes all information is accurate as of its publication date. Information, specifications, and prices are all subject to change without notice. Press-Seal is not responsible for any inadvertent errors.



# PRESS-SEAL GASKET CORPORATION

P.O. Box 10482, Fort Wayne, Indiana 46852

Phone: (260) 436-0521 (800) 348-7325 Fax: (260) 436-1908 E-mail: sales@press-seal.com Web: www.press-seal.com







"RUBBERMAN"

### PIPE-TO-MANHOLE SEAL

- 1. MEETS ASTM 923-C SPECIFICATIONS
- 2. RUBBER MEETS ASTM 443.
- 3. 20° OMNI-DIRECTIONAL DEFLECTION.
- 4. ACCOMMODATES PIPE OD'S FROM 3" AND UP.
- 5. AS SIMPLE AS LUBRICATING PIPE AND GASKET AND INSERTING PIPE.
- 6. MAY BE USED IN ROUND OR SQUARE STRUCTURES.
- 7. ACCOMMODATES SANITARY OR STORM SEWER SITUATIONS.
- 8. A COMPRESSION SEAL INTEGRALLY CAST INTO THE STRUCTURE.
- 9. HAS BEEN SUCCESSFULLY USED FOR PIPE ENTRY IN SANITARY SEWERS, STORM SEWERS, SEPTIC TANKS, AND WALLS OF TREATMENT PLANTS.
- 10. "RUBBERMAN I" LUBRICANT IS SUGGESTED AS AN ENVIRONMENTALLY FRIENDLY PRODUCT TO FACILITATE INSTALLATION PROCEDURES.
- 11. CAST IN STRUCTURE USING TWO-PIECE STEEL RINGS. ELIMINATING AN "OUT OF ROUNDNESS" SITUATION COMMON WITH FIBERGLASS RINGS.
- 12. CONSULTATION AVAILABLE CONCERNING ANY ANTI INFILTRATION-EXFILTRATION PROJECT.
- 13. OVER 40 YEARS EXPERIENCE DESIGNING, TESTING, MARKETING, AND SERVICING SEALS FOR MANUFACTURES OF PIPE, MANHOLES, SEPTIC TANKS, PUMP STATIONS, AND TREATMENT PLANTS.
- 14. "RUBBERMAN" IS A TRADE NAME FOR O RING, PROFILE GASKETS, PIPE TO MANHOLE SEALS; LUBRICANT, SEALANT, AND THE NEW PRECAST SEWER CHIMNEY AND PIPE ENCASEMENT SYSTEM.

### NOTE:

ALL STAR-SEAL<sup>TM</sup> PIPE-TO-MANHOLE GASKETS ARE MANUFACTURED BY: HAIL MARY RUBBER CO. INC. WARMINSTER, PA. DISTRIBTED BY:CONTINENTAL CONCRETE PRODUCTS INC. POSSTOWN, PA.



STAR-SEAL $^{\text{TM}}$  COMPRESSION GASKET SPECIFICATION SHEET

DATE: 6-00 SIGNITURE OF APPROVAL DWG NO. SA12 BTH



# PerformancePipe.com DRISCOPLEX® 4000 CDIPS 1 / 4:100 CIPS 1 HDPF PIPE DAYA SHEET

### DriscoPlex® 4000/4100 Pine meets or exceeds:

### DriscoPlex® 4000/4100 Pipe for:

ASTM F714 (4" and larger) AWWA C906 NSF 61
ASTM D3035 (up to 3") AWWA C901 NSF 61
ASTM D3350, cell classification PE345464C and PE445474C
PPI TR-4 designation PE3608 and PE4710
NSF 14 – Available upon request

Potable Water, Raw Water, Sanitary Sewer, Reclaimed Water, Storm Drain, Treated Sewage, etc.
Iron Pipe Size OD (IPS) 4" to 54",
Ductile Iron Pipe Size OD (DIPS) 4" to 42"
40' and 50' Joints / Solid Black / Color Striping Available
500' coils available in sizes through 6"

NOMINAL PIPE PROPERTIES (1)	UNIT	TEST METHOD	VALUE PE3608	VALUE PE4710
Density	gms / cm <sup>3</sup>	ASTM D1505	0.955 (black)	.0960 (black)
Melt Index (MI) Condition 190°C / 2.16kg	gms / 10 minutes	ASTM D1238	0.08	0.08
Hydrostatic Design Basis 73° F (23° C)	psi	ASTM D2837	1600	1600
Hydrostatic Design Basis 140° F (60° C)	psi	ASTM D2837	800	1000
Color: UV Stabilizer [C]	<del></del>	ASTM D3350	Min 2% Carbon Black	Min 2% Çarbon Black
NOMINAL MATERIAL PROPERTIES (1) (2)	UNIT	TEST METHOD!	VALUE PE3608	VALUE PE4710
Flexural Modulus 2% Secant – 16:1 span: depth. 0.5 in / min.	psi	ASTM D790	>110,000	>115,000
Tensile Strength at Yield	psi	ASTM D638 Type IV	3200	>3400
Elongation at Break 2 in / min., Type IV Bar	%	ASTM D638	>800	>800
Elastic Modulus	psi	ASTM D638	>150,000 •	>175,000
Hardness	Shore D	ASTM D2240	62	62
PENT	hrs	ASTM F1473	>100	>500
Vicat Softening Temperature	٥F	ASTM D1525	256	256
Brittleness Temperature	°F .	ASTM D746	< -103	< -103
Thermal Expansion	in/in/°F	ASTM D696	1.0 x 10 <sup>-4</sup>	1.0 x 10 <sup>-4</sup>

 This is not a product specification and does not guarantee or establish specific minimum or maximum values or manufacturing tolerance for material or piping products to be supplied.

Values obtained from tests of specimens taken from piping product may vary from these typical values. When Performance Matters Rely on Performance Pipe

Bulletin: PP101 / August 2009

© 2009 Chevron Phillips Chemical Company LP

Performance Pipe, a division of Chevron Phillips Chemical Company LP | 5085 W. Park Blvd | Suite 500 | Plano, TX 75093 | Phone: 800-527-0662 | Fax: 972-599-7348

This data sheet provides typical properties for Performance Pipe DriscoPlex® pipe and fittings. Before using this product, the user is advised and cautioned to make their own determination and assessment of the safety and suitability of the product for the specific use in question and is further advised against retying on the information contained herein as it may relate to any specific use or application. It is the ultimate responsibility of the user to ensure that the product is suited and the information is applicable to the user's specific application. Chevron Phillips Chemical Company LP does not make, and expressly disclaims, and expressing disclaims, including warranties of merchaniability or fitness for a particular purpose, regardless of whether oral or written, express or implied, altegedly arising from any usage of any trade or from any course of dealing in connection with the use of information contained herein or the product itself. The user expressly assumes at risk and liability, whether based in contract, for or otherwise, in connection with the use of the information contained herein or the product itself. Further, information contained herein or the product itself. Further, information contained herein or the product itself. Further, information contained herein or the product itself. The user expressly assumes at risk and liability, whether based in contract, for or otherwise, in connection with the use of the information contained herein or the product itself. Further, later to focal laws which may be encountered in the use thereof. Such questions should be investigated by the user. The data sheet may change periodically. Visit <a href="https://www.PerformancePipe.com">www.PerformancePipe.com</a> for the most current data sheet, as the or the product such as the product itself.



Bulletin: PP 152-4710

Lee Supply Co., Inc Charleroi, PA 1-800-353-3747

Revised 04-07-2009

### **IPS Size and Dimension Data**

### PE4710 (PE3408)

# DriscoPlex® Municipal & Industrial & Energy Series/IPS Pipe Data

Pressure Ratings are calculated using 0.63 design factor for HDS at 73°F as listed in PPI TR-4 for PE 4710 materials. Temperature, Chemical, and Environmental use considerations may require use of additional design factors.

Pressu	ıre		125 psi			100 psi			80 psi			63 psi		İ
Ratin	g	<del>-&gt;</del>	DR 17.0			DR 21.0			DR 26.0			DR 32,5		150 D:
IPS Pipe	Nominal	<b>@Minimum</b>	Average ID	Weight	Minimum	Average ID	Weight	Minimum	Average ID	Weight	, Minimum	Average ID	Weight	IPS Pipe
Size	OD (in)	Wall (in)	(in)	(lbs/ft)	⇒Wall (in) ⊚	(in)	(lbs/ft)	Wall (in)	(in)	(lbs/ft)	Wall (in)	(in)	(lbs/ft)	Size
原 11/4"	1,660	<b>第二次第二次第一</b>			<i>ं स्ट्रीनिक्</i> र सित्र सित्र सित्र सि			$\mathbb{K}_{N}(x) = \mathbb{K}_{n}(x) + \mathbb{K}_{n}(x)$			State of the			1 1/4"
11/2"	1.900	MUNICIPALITY CONT.			STANKE PAR									1 1/2"
2112	2.375	0.140	2.078	0.43	不是的推翻某些			feyring a few quest			grand to the			2"
57 EC 3"7 11 15 1	3.500	0.206	3.063	0.94	1985年中华			5,400,000,000			Eggli (1919)			3"
2" 4"	4.500	0.265	3.938	1.55	0.214	4.046	1.27	STATE TO STATE			Egg-Weign and			4"
6"	6.625	0.390	5.798	3,36	0.315	5.957	2.75	0.255	6.084	2.24	0.204	6.193	1.81	6"
8"	8.625	0.507	7,550	5.69	0.411	7.754	4.66	0.332	7.921	3.80	0.265	8.063	3.07	8"
<b>设备 10</b> "4。	10.750	0.632	9.410	8.83	0.512	9.665	7.24	0.413	9.874	5.91	0.331	10.048	4.77	10"
12"	12.750	0.750	11.160	12.43	0.607	11.463	10.19	0.490	11.711	8.31	0.392	11.919	6.71	12"
342314 <sup>#3</sup>	14.000	0.824	12.253	14.98	0.607	12.586	12.28	0.538	12.859	10.02	0.431	13.086',	8.09	14"
16"	16.000	0.941	14.005	19.57	0.762	14.385	16.04	0.615	14.696	13.09	0.492	14.957	10.56	16"
18!	18,000	1.059	15.755	24.77	0.857	16.183	20.30	( 0.692	16.533	16.57	0.554	16.826	13.37	18"
20"	20.000	1.176	17.507	30.58	0.952	17.982	25.07	0.769	18.370	20.45	0.615	18.696	16.50	20"
22" 3	22.000	1:294	19.257	37.00	1.048	19.778	30.33	0.846	20.206	24.75	0.677	20.565	19.97	22"
24"	24.000	1.412	21.007	44.03	1.143	21.577	36.10	0.923	22.043	29.45	0.738	22.435	23.76	24"
26"	26.000	1.529	22,759	51.67	1.238	23.375	42.36	1.000	23.880	34.57	0.800	24.304	27.89	26"
See 28" 1999	28.000	1.647	24,508	59.93	1:333	25.174	49.13	1.077	25.717	40.09	0.862	26.173	32.34	28"
30"	30.000	1.765	26.258	68.80	1.429	26.971	56.40	1.154	27.554	46.02	0.923	28.043	37.13	30"
32"	32,000	1.882	28.010	78.28	1.524	28,769	64.17	1.231	29.390	52.36	0.985	29.912	42.24	32"
34"	34.000	2.000	29.760	88.37	1.619	30,568	72.44	1.308	31.227	59.11	1.046	31.782	47.69	34"
36"	36,000	2.118	31.510	99.07	1.714	32.366	81.21	1.385	33.064	66.27	1.108	33.651	53.46	36"
42"	42.000	2.471	36.761	134.84	2.000	37.760	110.54	1.615	38.576	90.20	1.292	39.261	72.77	42"
48"	48,000	2.824	42.013	176.12	2.286	43.154	144.38	1.846	44.086	117.81	1.477	44.869	95.05	48"
54"	54.000	To the material	Ş		2.571	48.549	182.73	2.077	49.597	149.10	1.662	50.477	120.29	54"

Pipe weights are calculated in accordance with PPI TR-7. Average inside diameter is calculated using normal OD and Minimum wall plus 6% for use in estimating fluid flows. Actual ID will vary. When designing components to fit the pipe ID, refer to pipe dimension and tolerances in the applicable pipe manufacturing specification.

- ▶-Will not rust or corrode
- **▶** Will not warp or freeze open or shut
- Custom-built to customer specifications
- ▶ Low cracking pressure, low headloss
- **▶** Eliminates backflow



Materials of Construction Neoprene, Hypalon®, Buna-N, EPDM, Viton® and NSF-61-approved SBR.

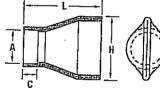
## Mounting Bands Carbon steel or stainless steel.

Red Valve's Tideflex® Check Valve has a revolutionary design for backflow prevention. It offers low cracking pressure to eliminate standing water and very low headloss that is not affected by rust, corrosion or lack of lubrication. Tideflex® Check Valves are cost-effective because they require no maintenance or repairs and have a long operational life span. Tideflex® operate using line pressure and backpressure to open and close so no outside energy source is required. Sliding, rotating, swinging and plunging parts are completely eliminated.

Tideflex<sup>5</sup> valves are excellent replacements for ineffective metal flapgate valves. Millions of dollars each year are lost in the re-treatment of unnecessary backflow because of faulty check valves that have corroded open or have been wedged open by debris. Tideflex<sup>6</sup> Check Valves close drop-tight and seal around debris with less than one psi of backpressure. Tideflex<sup>6</sup> valves will not warp or freeze and are virtually maintenance free. They will handle large obstructions without jamming, and there is no gate to hang open.

The inside diameter of the TF-2's cuff is constructed to exactly match the outside diameter of the pipe.

The valve is slid onto the pipe and held in place with steel or stainless steel band clamps, eliminating flanging costs. Tideflex TF-2 valves 18 and larger are constructed with a curved bill as standard.



16

STATE OF THE CAMP CONTROL OF	Control of the second second second	4		1			
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	<b>FRINGE OF THE PARTY OF THE PAR</b>						
3/4	. 3	1 1/2	1	28	39	. 37	8
1 1/2	<b>3</b>	1 1/2	1 1	30	42	50	9.
1 1/2	6	3	1 1	32	48	53	10
2 1/2	0	4	1	36	49	61	10
3	0	5 1/2	1	38	49	61	10
4	12	31/2	1 1/2	40	49	61	10
5	15 1/2	9	1 1/2	42	54	71,	10
. 6	- 16	10 1/2	2	44	54	71	10
8	16 1/2	13	2	48	59	Z8-	10
10	21 1/2	17	2	50	59	78	10
12	26 1/2	20 1/2	4 1/2	54° 58	69	97	10
14	26	22	4	60	69	97	10
16	26	27	5	68	74 74	97	14
18	30	29	6	72	95	97 115	14
20	33	33	8	84	92	111	16 10
22	36	33	8	90	101	119	16 16
24	39	37	Q	00	] [[]	***	TO



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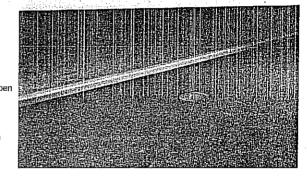
#### New CheckMate Inline Check Valve

Home - Check Valves - CheckMate® Inline Check Valve

#### **Features**

Extremely low headloss Durable 100% elastomer construction Easily installed in any type of pipe No menchanical parts 4" (100 mm) - 72" (1800 mm) size 25 year life expectancy
Operates on differential pressure Vidually maintenance-free Self-draining Less than 1" of head pressure cracks open

Eliminates standing water Silent, non-stamming Simple installation Extensive independent hydraulic testing Opens to near full pipe diameter



#### Materials Of Construction

Elastomer Information Expansion Clamps:

304 Stainless Steel (Standard) 316 Stainless Steel Special Alloys Available

To view the CheckMate® Valve in action, press the play button above...

#### Description

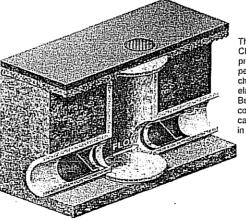
Patented by Red Valve Company, the CheckMate® Inline Check Valve is ideal for backflow prevention and odor mitigation. In outfalls, stormwater, CSO and SSO applications, the CheckMate's® custom-engineered, allrubber unibody design eliminates costly backflow from oceans, rivers and interceptors. CheckMate® Valves are readily available in 4" to 72" sizes. The CheckMate® is built to suit all your site specific and flow needs.



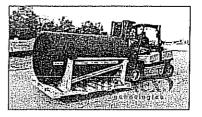
CheckMate® Inline Check Valve

#### Case Studies

Stormwater Flood Protection Odor Mitigation



CheckMate's® unique elastomer-reinforced design provides a proven record of maintenance-free performance, cost savings and results that no other inline check valve can match. The valve has a 100% fabric and elastomer construction that eliminates corrosion problems. Because the CheckMate® is made with a unibody construction, there are no mechanical components to catch debris, corrode or fail. The result is in savings - both in time and costs.



30" CB-G-TO POND



#### **PURE GUM RUBBER (PGR)**

Excellent resiliency, tensile strength, and abrasion resistance. Generally good for most weak chemicals, wet or dry organic acids, sodium hydroxide (caustic), alcohols and ketones.

**Affected By:** Ozone, strong acids, fats, oils, greases, wet chlorine gas, methane and most hydrocarbons.

Operating Temperature Range: -50° F to +180° F

#### NEOPRENE

Generally resistant to moderate chemicals, ozone, fats, sodium hydroxide (caustic), methane and most hydrocarbons.

**Affected By:** Strong oxidizing acids, acetic acid, ketones, wet chlorine gas, chlorinated and nitro-hydrocarbons, and aromatic hydrocarbons.

Operating Temperature Range: -50° F to +230° F

#### WHITE FOOD GRADE NEOPRENE

Generally resistant to moderate chemicals, ozone, fats, sodium hydroxide (caustic), methane and most hydrocarbons.

**Affected By:** Strong oxidizing acids, acetic acid, ketones, wet chlorine gas, chlorinated and nitro-hydrocarbons, and aromatic hydrocarbons.

Operating Temperature Range: -50° F to +230° F

#### **EPDM (NORDEL)**

Excellent abrasion and chemical resistance at elevated temperatures. Good with dilute acids (sulfuric & acetic), steam, ketones, sodium hydroxide (caustic), hydrogen sulfide and domestic wastewater. Good UV resistance. Also used with radioactive wastewaters.

Affected By: Petroleum oils, hydrochloric acid, concentrated methane, wet chlorine gas.

**Operating Temperature Range:** -50° F to +300° F

#### WHITE FOOD GRADE EPDM (NORDEL)

Excellent abrasion and chemical resistance at elevated temperatures. Good with dilute acids, (sulfuric & acetic), steam, ketones, sodium hydroxide (caustic), hydrogen sulfide and domestic wastewater. Good UV resistance. Also used with radioactive wastewaters.

Affected By: Petroleum oils, hydrochloric acid, concentrated methane, wet chlorine gas.

Operating Temperature Range: -50° F to +300° F



#### **BUNA-N**

Resistant to many hydrocarbons, fats, oils, grease, kerosene, and moderate chemicals. Excellent with methane.

Affected By: Ozone, strong acids, hydrogen sulfide and ketones.

Operating Temperature Range: -30° F to +230° F

#### WHITE FOOD GRADE BUNA-N

Resistant to many hydrocarbons, fats, oils, grease, kerosene, and moderate chemicals. Excellent with methane.

Affected By: Ozone, strong acids, hydrogen sulfide and ketones.

Operating Temperature Range: -30° F to +230° F

#### **HYPALON**

Resistant to heat, ozone, weathering, sodium hydroxide (caustic), and oxidizing chemicals. Good resistance to strong acids at room temperature and methane. Resistant to some hydrocarbons, alcohols

**Affected By:** Aromatic ketones, acetyl compounds, benzene compounds, petroleum oils, wet chlorine gas.

**Operating Temperature Range:** -50° F to +300° F

#### VITON

Resistant to many halogenated hydrocarbons, fats, oils, grease, sodium hydroxide (caustic), solvents and most chemicals. Excellent resistance to ozone, oxygen, methane, and weathering. **Affected By:** Ketones, esters, hydrogen sulfide, and anhydrous ammonia.

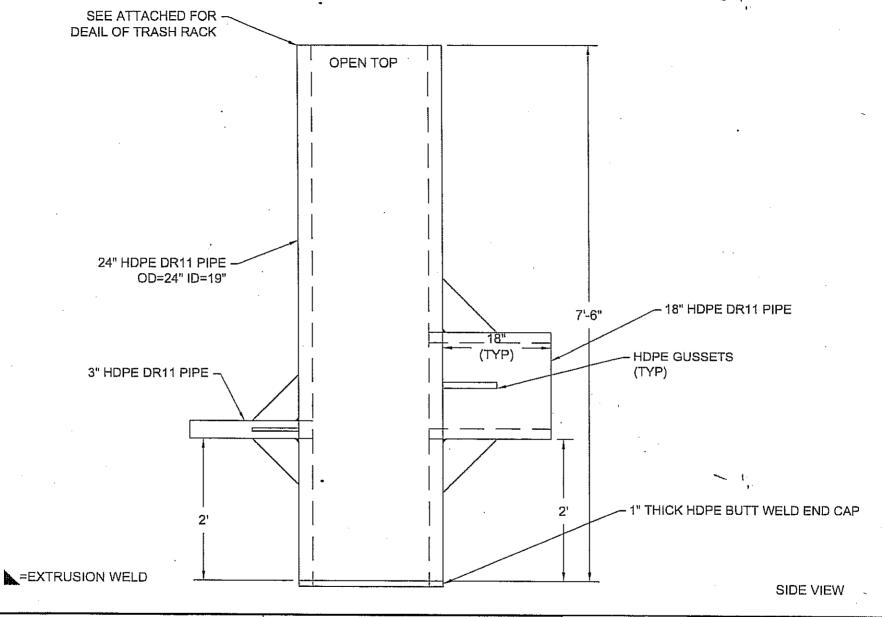
Operating Temperature Range: -10° F to +400° F

#### WHITE FOOD GRADE VITON

Resistant to many halogenated hydrocarbons, fats, oils, grease, sodium hydroxide (caustic), solvents and most chemicals. Excellent resistance to ozone, oxygen, methane, and weathering.

Affected By: Ketones, esters, hydrogen sulfide, and anhydrous ammonia.

Operating Temperature Range: -10° F to +400° F



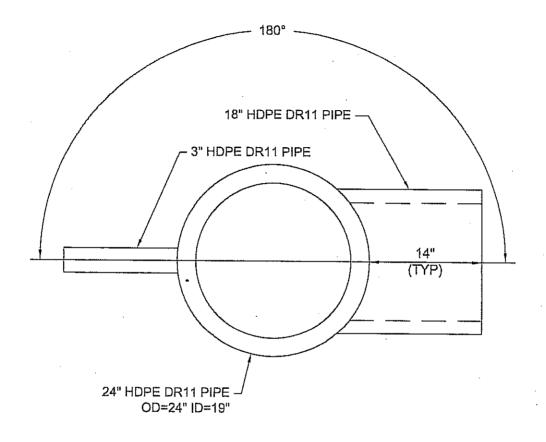
SEVENSON ENVIRON SVS SOUTH PLAIN FIELD 24" HDPE DR11 OUTLET STRUCTURE

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EXTRUSION WELDS ARE NOT PRESSURE RATED.
DRAWINGS MUST BE SIGNED FOR APPROVAL.
ONCE APPROVED ALL MATERIALS ARE NON
CANCELLABEL/NON RETURNABLE.
ALL FABRICATIONS WILL BE BASED ON
APPROVED DRAWINGS.



	CUSTOMER:	SEVENSON
	DRAWN BY:	MATTHEW
,	DATE:	MAY 20,2011
	APPROVED BY:	
	REVISIONS:	
_		



TOP VIEW

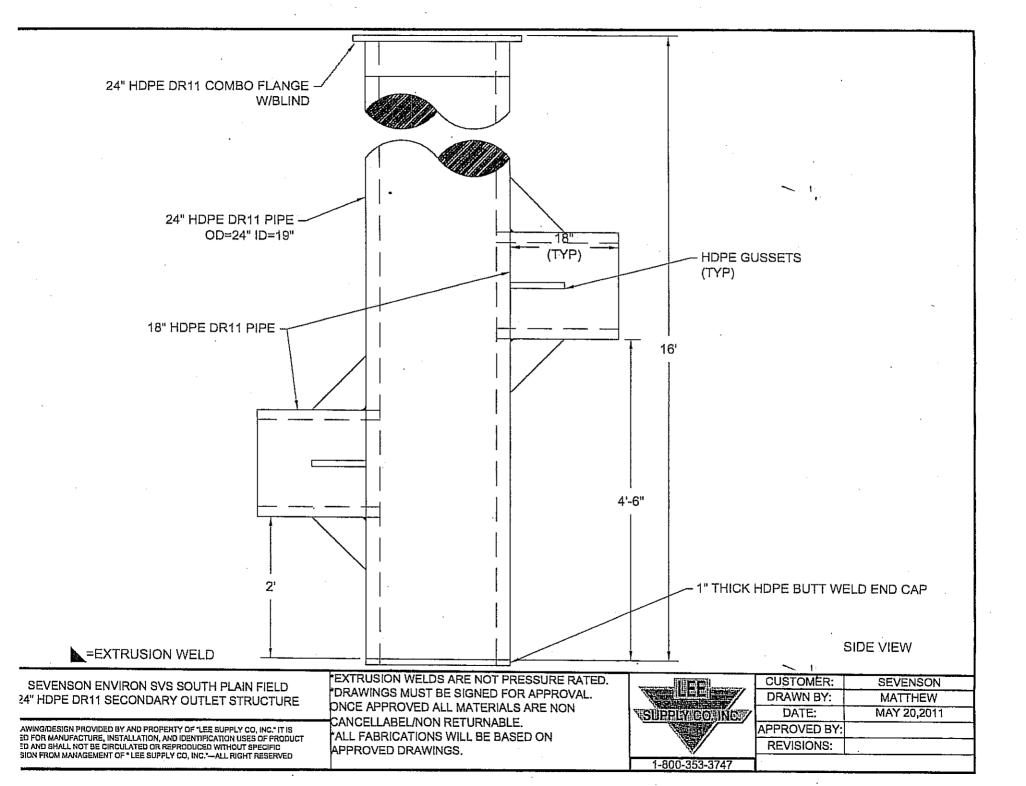
## SEVENSON ENVIRON SVS SOUTH PLAIN FIELD 24" HDPE DR11 OUTLET STRUCTURE

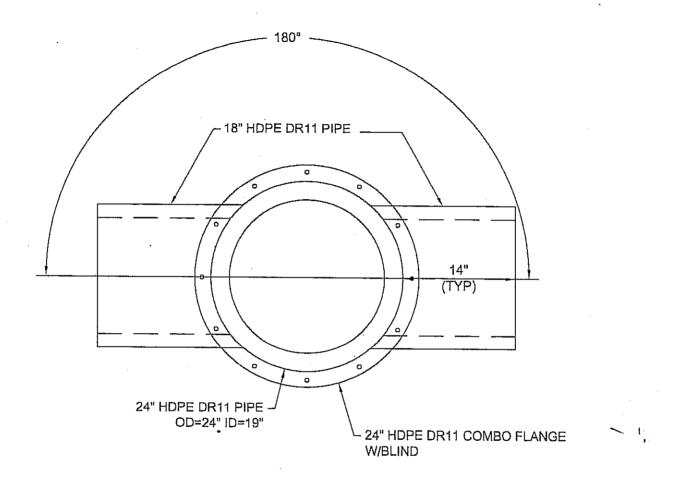
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CUSTOMER:	SEVENSON
DRAWN BY:	MATTHEW
DATE;	MAY 20,2011
APPROVED BY:	
REVISIONS:	





TOP VIEW

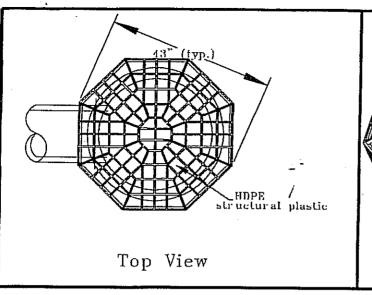
SEVENSON ENVIRON SVS SOUTH PLAIN FIELD 24" HDPE DR11 SECONDARY OUTLET STRUCTURE

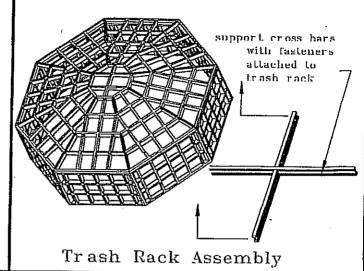
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	CUSTOMER:	SEVENSON
	DRAWN BY:	MATTHEW
•	DATE:	MAY 20,2011
	APPROVED BY:	
	REVISIONS:	





Plastic Solutions Inc. trash rack or equal 877-877-5727 metal pipe —Top of riser Elev. A-A 21", 24", 27" or 30" corrugated metal pipe - Trash rack 24 3/4 18" plastic pipe - Top of riser Elev. A-A 24" or 30" plastic pipe NOTE: CUSTOM SIZES AVAILABLE UPON REQUEST \*\*FASTENING KIT INCLUDED\*\*

PART NO

PYDP 36

36" PYRAMID
TRASH RACK
WITH METAL OR PLASTIC RISER

LEE SUPPLY CO., INC.

P.O. BOX 35 305 FIRST & LINCOLN AVE. CHARLEROI, PA 15022



January 17, 2011

Mr. John Knoeringer East Coast Liner Co., Inc. 1565 Route 37 West, Unit 11 Toms River, NJ 08755

Phone: 732-341-4000 Fax: 732-341-5412

Email: thelinerguy@aol.com

To Whom it may concern,

This correspondence serves to recognize East Coast Liner Co., Inc. as a certified installer of Solmax International flexible geomembrane products (PE & PVC).

As a certified Installer, East Coast Liner Co., Inc. has to follow the International Association Geosynthetic Installers (IAGI) field installation quality assurance manual.

East Coast Liner Co., Inc. also has access to Solmax International's technical department's representatives who can respond to questions on specifications and installation techniques.

Solmax International Inc. does not warrant nor guarantee the work of a certified installer, hence East Coast Liner Co., In. and/ or it's employees are acting as independent contractors and/ or employees of Solmax International Inc. and they may not grant any right or authority or assume or create any obligation or liability express or implied, for or on behalf of Solmax International Inc. without the written consent of an authorized representative of Solmax International Inc.

On the assumption that you will find the foregoing to your satisfaction.

Sincerely yours,

Side Bur-2011.03.24 13:42:06-04'00'

Silda Rivas ِ

South & North American Sales Representative Solmax International Inc.



Sales Office: Engineered Synthetic Products, Inc. Phone (770) 564-1857 Fax (770) 564-1818 www.espgeosynthetics.com

March 31, 2011

IWT Cargo Guard P.O. Box 454 Waretown, NJ 08758

RE: Approved Installer

To Whom It May Concern:

East Coast Liners, Inc. is an approved installer of Skaps Industries geosynthetics and is in good standing. They are an authorized installer of both our gecomposites and geotextiles. East Coast Liners is authorized to act as a representative of Skaps Industries when installing our products. Please let us know if you have any questions.

Sincerely,

Brent Beckham ESP/Skaps Industries

#### EAST COAST LINER COMPANY

John Knoeringer: Resume

#### **Employment History**

Partner in East Coast Liner Co. Sales, service, installation and repair of all types 2000 - Present

of lining systems.

Field Superintendent and Assistant Quality Control Manager - The Liner Co. 1983 - 1999

> Implemented Quality Control procedures and maintained quality control records. Personally involved and assisted in the installation of over

41,000,000 square feet of various lining systems.

Master Seamer - The Liner Co. Completed installation training provided by 1980-1982

Staff Industries (PVC & Hypalon), National Seal Company (HDPE), and

Gundle Lining Systems (HDPE).

Layout and Seaming Technician - The Liner Co. Completed on site field training 1977-1979

in panel layout and various types of seaming methods.

#### Installation History for MPC Petroguard Liners 105,812 Sq ft Installed to date

#### 2010

Waterside 22M Substation

Stamford, CT

GC: Northeast Utilities Service Co.

James D. Livingston

860-665-6790

**BOKUM 15L Substation** 

Old Saybrook, CT

GC: Northeast Utilities Service Co.

James D. Livingston

860-665-6790

Montville 4J Substation

Old Saybrook, CT

GC: Charter Oak Utility Constructors, Inc.

Dennis Keiser

860-241-8274

Secondary Containment

MPC Petroguard VI: 2,340 Sq ft

8 oz. Geotextile: 4,680 Sq ft

Secondary Containment

MPC Petroguard VI: 1,380 Sq ft

8 oz. Geotextile: 2,760 Sq ft

Secondary Containment

MPC Petroguard VI: 5,025 Sq ft

8 oz. Geotextile: 10,140 Sq ft

#### 2010 Continued

Manchester Substation

Manchester, CT

GC: Charter Oak Utility Constructors, Inc.

Dennis Keiser 860-241-8274

**Greenwich Substation** 

Greenwich, CT

GC: Northeast Utilities Service Co.

James D. Livingston 860-665-6790

Secondary Containment

MPC Petroguard VI: 8,352 Sq ft

8 oz. Geotextile: 16,704 Sq ft

Secondary Containment

MPC Petroguard VI: 1,410 Sq ft

8 oz. Geotextile: 2,820 Sq ft

2009

**Ludlow Sub Station** 

Ludlow, MA

GC: Witch Enterprises, Inc.

Lou Ramah

413-786-7314 ext:13

Secondary Containment

MPC Petroguard VI: 6,876 Sq ft

8 oz Geotextile: 13,752 Sq ft

Norwalk Harbor 6J 8x & 9x Transfromer Sump

Norwalk, CT

GC: Charter Oak Utility Cinstructors, Inc.

Dennis Keiser

8 oz Geotextile: 12,400 Sq ft

Secondary Containment

MPC Petroguard VI: 6,200 Sq ft

860-241-8274

Flax Hill 24A Substation

Norwalk, CT

GC: Northeast Utilities Service Company

James D. Livingston

860-665-6790

Secondary Containment

MPC Petroguard VI: 2,376 Sq ft

8 oz Geotextile: 4,752 Sq ft

Rood Ave. 24J Substaion

Windsor, CT

GC: Northeast Utilities Service Company

James D. Livingston

860-665-6790

Secondary Containment

MPC Petroguard VI: 2,196 Sq ft

8 oz Geotextile: 4,392 Sq ft

**Mystic 13K Substaion** 

Mystic, CT

GC: Northeast Utilities Service Company

James D. Livingston

860-665-6790

Secondary Containment

MPC Petroguard VI: 5,712 Sq ft

8 oz Geotextile: 13,500 Sq ft

#### 2009 - continued

Waterford 36f Substation

Waterford, CT

GC: Northeast Utilities Service Company

James D. Livingston

860-665-6790

Plumtree 30G Substation

Bethel, CT

GC: Charter Oak Utility Constructors, Inc.

Dennis Keiser 860-241-8274

<u>2008</u>

Hinsdale Substation

Hinsdale, MA

GC: Northeast Utilities Service Company

James D. Livingston 860-665-6790

Oxford 26N Substation

Oxford, MA

GC: Northeast Utilities Service Company

James D. Livingston

860-665-6790

Norwalk Harbor Transformer Sump

Norwalk, CT

GC: Charter Oak Utility Constructors, Inc.

Dennis Keiser 860-241-8274

Stepstone 35L Substation

Stepstone, CT

GC: Northeast Utilities Service Company

James D. Livingston

860-665-6790

Torrington Terminal 8A Transformer Sump

Torrington, CT

GC: Charter Oak Utility Constructors, Inc.

Dennis Keiser

860-241-8274

Secondary Containment

MPC Petroguard VI: 5,784 Sq ft

8 oz Geotextile: 13,500 Sq ft

Secondary Containment

MPC Petroguard VI: 6,360 Sq ft

8 oz Geotextile: 12,720 Sq ft

Secondary Containment

MPC Petroguard VI: 7,920 Sq ft

8 oz Geotextile: 15,840 Sq ft

Secondary Containment

MPC Petroguard VI: 4,560 Sq ft

8 oz Geotextile: 9,000 Sq ft

Secondary Containment

MPC Petroguard VI: 6,200 Sq ft

8 oz Geotextile: 12,400 Sq ft

Secondary Containment

MPC Petroguard VI: 4,560 Sq ft

8 oz Geotextile: 9,000 Sq ft

Secondary Containment

MPC Petroguard VI: 4,320 Sq ft

8 oz Geotextile: 9,000 Sq ft

#### 2007

Frost Bridge 8R Sub-Station

Watertown, CT

GC: Charter Oak Utility Constructors, Inc.

Dennis Keiser 860-241-8274

Secondary Containment

Secondary Containment

MPC Petrogard VI: 6,000 Sq ft

8 oz Geotexile: 12,000 Sq ft

MPC Petrogard VI: 3,000 Sq ft

8 oz Geotextile: 6,000 Sq ft

Cedar Heights 4R Substation

Stamford, CT

GC: Northeast Utilities/

James D. Livingston

860-665-5000

Partridge 15E substation

Pittsfield, Massachusetts

GC: Northeast Utilities

James D. Livingston

860-665-5000

Secondary containment

MPC Petrogard VI: 1,344 Sq ft

8 oz Geotextile: 2,688 Sq ft

Wilton 35A Substation

Wilton, CT

GC: Northeast Utilities

James D. Livingston

860-665-5000

Secondary Containment

MPC Petrogard VI: 4,500 Sq ft

8 oz Geotextile: 9,000 Sq ft

**Enfield 12C substation** 

Enfield, CT

GC: Northeast Utilities

James D. Livingston

860-665-5000

Secondary Containment

MPC Petroguard VI: 1,820 Sq ft

8 oz Geotextile: 3,640 Sq ft

**2006** 

**Naval Station Newport** 

Newport, RI

GC: TN & Associates, Inc.

Chris Miller

865-220-9000

Secondary Containment

MPC Petroguard X: 4,050 Sq ft

#### 2005

PS#37

Jersey City, NJ

GC: Greg

973-209-2545

Dickenson High School

Jersey City, NJ

GC: Occidental Construction Co., Inc.

Frank Calderone

732-537-1000

Boiler Plant Secondary Containment MPC Petroguard VI: 1,250 Sq ft

Boiler Plant Secondary Containment MPC Petroguard VI: 1,050 Sq ft

/

2001

\*GPU Whiting Substation

Manchester, NJ

GC: Henkels & McCoy

Ed McDonald 215-283-7634

\*Subcontracted from The Liner Co.

Secondary Containment MPC Petroguard VI: 1,200 Sq ft



Sales Office: Engineered Synthetic Products, Inc. Tel (770) 564-1857 Fax (770) 564-1818 www.espgeosynthetics.com

#### Geotextile Product Description Sheet SKAPS GE-110 Nonwoven Geotextile

SKAPS GE-110 is a needle-punched nonwoven geotextile made of 100% polypropylene staple fibers, which are formed into a random network for dimensional stability. SKAPS GE-110 resists ultraviolet deterioration, rotting, biological degradation, naturally encountered basics and acids. Polypropylene is stable within a pH range of 2 to 13. SKAPS GE-110 conforms to the physical property values listed below:

PROPERTY	TEST METHOD	UNIT	M.A.R.V. (Minimum Average Roll Value)		
Weight	ASTM D 5261	oz/yd² (g/m²)	10.0 (339)		
Grab Tensile	ASTM D 4632	lbs (kN)	270(1.20)		
Grab Elongation	ASTM D 4632	%	50		
Trapezoid Tear Strength	ASTM D 4533	lbs (kN)	100(0.44)		
Thickness*	ASTM D 5199	mils (mm)	110 (2.79)		
CBR Puncture Resistance	ASTM D 6241	lbs (kN)	725 (3.22)		
Permittivity*	ASTM D 4491	sec <sup>-1</sup>	0.94		
Permeability*	ASTM D 4491	cm/sec	0.3		
Water Flow*	ASTM D 4491	gpm/ft <sup>2</sup> (l/min/m <sup>2</sup> )	75 (3055)		
AOS	ASTM D 4751	US Sieve (mm)	100 (0.150)		
UV Resistance	ASTM D 4355	%/hrs	70/500		

PACKAGING							
Roll Dimensions (W x L) – ft	·15 x 570						
Square Yards Per Roll	950						
Estimated Roll Weight – Ibs	620						

<sup>\*</sup> At the time of manufacturing. Handling may change these properties.

This information is provided for reference purposes only and is not intended as a warranty or guarantee. SKAPS assumes no liability in connection with the use of this information.

**SKAPS Industries**, 335 Athena Dr., Athens, GA 30601, Phone:(706)-354-3700, Fax(706)-354-3737, www.skaps.com

Made in U.S.A.



SKAPS INDUSTRIES NONWOVEN DIVISION 316 S.Holland Drive, Pendergrass, GA 30567 Sales Office: Engineered Synthetic Products, Inc. 3985 Steve Reynolds Blvd Unit H Norcross GA USA 30093 www.espgeosynthetics.com www.skaps.com

# QUALITY CONTROL PROGRAM OUTLINE

### SKAPS Industries Nonwovens

QUALITY CONTROL PROGRAM OUTLINE

#### RAW MATERIAL QUALITY CONTROL

All raw materials used in the manufacturing of SKAPS Nonwoven products are certified by the supplier to meet the most stringent production standards in the industry. Each truckload of fiber received by SKAPS Nonwovens is certified by the resin supplier's Quality Control Manager to meet specifications as set by SKAPS Industries. All fiber released to production can be tracked by supplier and individual bale number for up to one year after the fiber is processed.

#### **DEFINITION OF 'LOT'**

A Lot is a planned production quantity satisfying all of the following:

- Manufactured under the same material specification.
- · Identified as the same style (fabric designation).
- When tested, having physical characteristics consistent with published values.

#### QUALITY CONTROL CONFORMANCE SAMPLING OF EACH LOT

As a minimum, a number of production units shall be selected at random from each lot in accordance with TABLE 1.

TABLE 1
Number of Units Selected as Lot Samples
Specification Conformance

Number of Units in Lot	Number of Units Selected
1 to 2	1
3 to 8	2
9 to 27	3
28 to 64	4
65 to 125	5
126 to 216	6
217 to 343	7 .
344 to 512	8
513 to 729	9
730 to 1000	10
1001 or more	11

Note: A production unit is considered to be a shipment roll.

Typically, the first shipment roll from each line will be sampled. It will be necessary to consider the minimum planned production quantity to determine if more frequent sampling and testing is required.

#### **Quality Control Testing of Each Sample:**

Each quality control sample shall be sent to the quality control lab before the end of the shift during which the sample was taken.-Full identification of the sampled roll will be provided with the sample.

The following tests are performed on each sample:

TEST PROPERTY	TEST METHOD
Weight	ASTM D 5261
Thickness	ASTM D 5199
Grab Tensile	ASTM D 4632
Grab Elongation	ASTM D 4632
Trapezoid Tear Strength	ASTM D 4533
Puncture Resistance	ASTM D 4833
Mullen Burst Strength	ASTM D 3786
Water Flow Rate	ASTM D 4491
Permeability	ASTM D 4491
Permittivity	ASTM D 4491
U.V. Resistance	ASTM D 4355
Apparent Opening Size (AOS)	ASTM D 4751

#### **Quality Control Test Results:**

All quality control test results will be maintained by the Quality Control Manager along with the corresponding shipment roll identification.

The Quality Control Manager will make lot testing summaries available upon request detailing the individual test results and aggregate mean, minimum and standard deviations of each test property for the shipment rolls under consideration.

#### **SKAPS NONWOVENS**

#### **QUALITY CONTROL PLAN**

The Quality Control Department tests nonwoven fabrics at the following frequencies. The tests for these properties are routine with test results reported representing each roll of fabric produced.

MINIMUM TESTING FREQUENCY IN SQUARE YARDS

PROPERTY	UNITS	TEST METHOD	MINIMUM FREQUENCY SQUARE YARDS
Mass/Unit Area	oz/yd	ASTM D 5261	10,000
Thickness	mils	ASTM D 5199	10,000
Grab Tensile Strength	lbs	ASTM D 4632	10,000
Grab Elongation	. %	ASTM D 4632	10,000
Trapezoidal Tear Strength	lbs	ASTM D 4533	15,000
Puncture Strength	lbs	ASTM D 4833	15,000
Mullen Burst	psi	ASTM D 3786	15,000
Apparent Opening Size	U.S. Sieve	ASTM D 4751	65,000
Permittivity	sec <sup>-1</sup>	ASTM D 4491	65,000
Permeability	cm/sec	ASTM D 4491	65,000
Water Flow	gpm/ft²	ASTM D 4491	65,000

Additional testing is conducted on non-routine properties in the SKAPS Quality Control Lab or at a reputable independent test lab. Examples of non-routine tests include:

PROPERTY	UNITS	TEST METHOD
Abrasion-Sliding Block	% strength retention	ASTM D 4886
Abrasion-Rotary Platform	lbs	ASTM D 3884
U.V. Resistance-Fluorescent Type	% strength retention	ASTM G 53
U.V. Resistance-Xenon Type	% strength retention	ASTM D 4355
Wide Width	lbs/in	ASTM D 4595

SKAPS conforms and adheres to the following additional ASTM Test Methods relating to fabric identification, sampling and specification conformance:

ASTM D 4873 Identification, Storage and Handling of Geotextiles

ASTM D 4354 Sampling of Geosynthetics for Testing

ASTM D 4759 Determining Specification Conformance of Geosynthetics



## CIVIL PRODUCTS

Product	GT131	GT135	GT140	GT142	GT160	GT170	GT180	GT110	GT112	GT116
Square Yard	500/600	500/600	500/600	500/600	500	500	500	500	500	250
Testing Frequency, Number of Rolls										
Mass/Unit Area	20/15	20/15	20/15	20/15	20	20	20	20	20	40
Thickness	20/15	20/15	20/15	20/15	20	20	20	20	20	40
Grab Tensile Strength	20/15	20/15	20/15	20/15	20	20	20	20	20	40
Grab Elongation	20/15	20/15	20/15	20/15	20	20	20	20	20	40
Trapezoidal Tear Strength	30/25	30/25	30/25	30/25	30	30	30	30	30	60
Puncture Strength	30/25	30/25	30/25	30/25	30	30	30	30	30	60
Mullen Burst	30/25	30/25	30/25	30/25	30	30	30	30	30	60
AOS	130	130	130	130	130	130	130	130	130	260
Permittivity	130	130	130	130	130	130	130	130	130	260
Permeability	130	130	130	130	130	130	130	130	130	260
Water Flow	130	130	130	130	130	130	130	130	130	260

## **ENVIRONMENTAL PRODUCTS**

Product	GE140	GE160	GE170	GE180	GE110	GE112	GE114	GE116		
Square Yard	2250	1500	1300	1150	950	800	650	600		
Testing Frequency, Number of Rolls										
Mass/Unit Area	4	6	7	8	10	12	15	16		
Thickness	4	6	. 7	8	10	12	15	16		
Grab Tensile Strength	4	6	7	8	10	12	15	16		
Grab Elongation	4	6	7	8	10	12	15	16		
Trapezoidal Tear Strength	6	10	11	13	15	18	23	25		
Puncture Strength	6	10	11	13	15	18	23	25		
Mullen Burst	6	10	11	13	15	18	23	25		
AOS	28	43	50	56	68	80	100	108		
Permittivity	28	43	50	56	68	80	100	108		
Permeability	28	43	50	56	68	80	100	108		
Water Flow	28	. 43	50	56	68	80	100	108		

## ROUTINE CHECKS

The following checks of SKAPS nonwoven fabrics an

- Visual inspection Line Inspector inspects fabric is correct take-up and needle streaks.
- Metal Detection Three metal detectors are position needles or other contaminates. If needles are detected and needles are located and removed.

Certifications are required on all fiber purchases to co data is required for each shipment.

In SKAPS' SPC system of quality reporting, a request for from any property failing to meet specification for three

This system has been implemented to correct all non-conductries' policy to ship only fabric meeting or exceeding issued daily summarizing manufacturing production.

## SAMPLING FREQUENCY OF WOVEN GEOTEXTILES

The sampling frequency of woven geotextiles exceeds the requirements of ASTM D-4354. ASTM D-4354 requires that the cubed root of the number of rolls in a lot be tested. Following is a table outlining the number of samples to be tested per lot size.

Number of Units in Lot / 1 to 2 3 to 8 9 to 27 28 to 64 65 to 125	Number of Units Selected  1 2 3 4 5	
65 to 125 126 to 216 217 to 343	6 7	

For the purposes of defining a lot to determine sample frequency a truckload quantity will be used. Using style W300 150" X 360', it takes 220 rolls to fill a truck. According to ASTM D-3454, the number of rolls that should be tested is seven.

- Junt	Rolls/truck	Yds/truck	Yds/beam	Samples/truck	Samples needed		
Product		34,704	6,000	10	7		
W200 150"X 432'	241			6	6		
W200 210" X 309'	176	18,128	6,000	"	-		
W300 150" X 360'	220	26,400	6,000	8	7		
		12,384	6,000	6	6		
W300 210" X 258'	144	[	'-				
		mple 1 <sup>51</sup> and	Ath master ro	all off loom for each	h loom beam		
W200 150" X 432'	38	Sample 1 <sup>st</sup> and 4 <sup>th</sup> master roll off loom for each loom beam  Sample 1 <sup>st</sup> and 4 <sup>th</sup> master roll off loom for each loom beam					
W200 210" X 309'	Sa	Sample 1" and 4" master roll off footh for each footh beam					
	ļ	51 _ 45	Alh mactor ro	all off loom for eac	h loom beam		
W300 150" X 360'		Sample 1 <sup>st</sup> and 4 <sup>th</sup> master roll off loom for each loom beam					
14000 0401 V 2501	Sam	nle 1 <sup>st</sup> , 3 <sup>rd</sup> ar	nd 5 <sup>th</sup> master	roll off loom for e	ach loom beam		
W300 210" X 258'		oumple r j v					

Consider the weave room running style W300 150" X 360' with 6,000 linear yards on yarn on a loom beam. Put-up of master rolls from the loom beam will be approximately 1,000 yards per roll. Six master rolls will be produced from the loom beam. Two samples should be taken from the loom beam. The first sample should be taken from the first master roll off the loom. The second sample should be taken from the fourth master oll doffed from the loom beam.

In the example of using a truckload lot of W300 150" X 360' with 220 rolls on a truck, there will be 26,400 linear yards of fabric in the lot. This translates to approximately 4.4 loom beams of warp yarn per truck for a total of eight samples tested. Since the number of samples tested meet the ASTM method, the requirements of ASTM D-4354 are met.

## **WOVENS SAMPLING PROCEDURE**

W200 150" X 432'	Sample 1 <sup>st</sup> and 4 <sup>th</sup> master roll off loom for each loom beam
W200 210" X 309'	Sample 1 <sup>st</sup> and 4 <sup>th</sup> master roll off loom for each loom beam
W300 150" X 360'	Sample 1 <sup>st</sup> and 4 <sup>th</sup> master roll off loom for each loom beam
W300 210" X 258'	Sample 1 <sup>st</sup> , 3 <sup>rd</sup> and 5 <sup>th</sup> master roll off loom for each loom beam

This test frequency exceeds the requirements in ASTM D-4354.

#### LAB PROCEDURES

ASTM D-5261
ASTM D-4632
ASTM D-4632
ASTM D-4533
ASTM D-3786
ASTM D-4751



## GEOTEXTILE INSTALLATION INSTRUCTIONS

SEWING / INSTALLATION GUIDE
NONWOVEN GEOTEXTILES

#### **SKAPS INDUSTRIES**

#### **Nonwoven Division**

#### Installation Procedure Geotextile Fabrics

#### I. Geotextile Unloading & Storage:

- A. The geotextile shall be labeled, stored, and handled in accordance with ASTM D 4873, "Guide for Identification, Storage and Handling of Geotextiles".
- B. Geotextile rolls are to be unloaded under supervision of the geotextile installer using straps or other devices that will prevent damage to the geotextile material.
- C. The geotextile shall be kept dry and wrapped in a waterproof wrapping so that it is protected from UV light and the elements during shipping and storage. Torn wrapping shall be repaired as quickly as possible using an approved protective covering.
- D. Rolls should be stored on supports that will not damage the material. The material must be elevated at least 2 inches above the sub grade.
- E. If any material is found to be damaged during unloading, a notation should be made as to the roll number, location of damage and type. This information should be given to the Project Manager.

#### II. Material Deployment

- A. No material is to be deployed until the Project Inspector has inspected and approved installation of the geotextile.
- B. Material will not be deployed when moisture, high winds, or other adverse weather conditions are expected. This determination will be made by the Field Installation Superintendent (FIS).
- C. Geotextile materials are to be deployed using methods that will not damage the material. The material will be visually inspected during deployment and any faulty or unsatisfactory areas will be marked for corrective action.
- D. If necessary, temporary sand bags may to be used to prevent material uplift and movement from winds during geotextile installation. The number and location of sand bags will be determined by the FIS.
- E. All folds and excessive wrinkles are to be removed prior to sewing adjacent panels together.
- F. On slopes, the geotextile shall be anchored at the top and unrolled down the slope.
- G. Material may be deployed by one three methods, it may be overlapped, sewn or heat seamed together as specified by the site engineer.

#### III. Material Seaming

- A. Field seams are to be made by using sewing machines and thread specifically adapted for this purpose.
- B. Adjacent panels are to be overlapped a minimum of six inches and sewn together.

  A sewing crew is to consist of a sewing machine operator and at least one assistant to help align the materials. The machine operator and assistant are to inspect opposite sides of the seam for dropped or incorrect stitches.
- C. Seams shall be sewn utilizing one or two rows of stitching. Each row shall consist of 4 to 7 stitches per inch.
- D. Damaged areas of geotextile are to be patched with an additional layer of geotextile material. The patch is to overlap the damaged area by a minimum of six inches on each side and is to be heat bonded to the main layer of geotextile.
- E. Thread should be of contrasting color to the fabric to facilitate seam inspection.
- F. The installer shall ensure that no soil materials are present within seams or overlaps.
- G. See below for heat seaming instructions.

#### IV. Project Documentation

- A. The FIS will maintain the following documentation on a daily basis:
  - Log of job activities, including number of personnel, weather conditions, and quantity of geotextile deployed.
  - 2. Listing of material placed, including panel size and location, and a cross reference of panel numbers.
  - 3. Listing of patches and repairs, including location and reason.
- B. Upon completion of the project, the following documentation is to be provided to the owner or inspector:
  - 1. Copies of Items 1, 2, and 3 above.
  - 2. Copies of Material Certifications from the Geotextile Manufacturer, if required by the project specifications.

#### THREAD SPECIFICATION

Threads used to sew geotextiles should be:

Polyester, Polypropylene, or Nylon Bonded and Thermally Set 1800 Denier Minimum

SKAPS Nonwovens recommends the use of BT207 - nylon sewing thread which meets or exceeds all these criteria.

Unless otherwise specified, the thread should be of contrasting color to the fabric to facilitate seam inspections.

Thread weight is typically expressed as "denier" or "tex". Denier is the weight in grams of 9000 linear meters of thread. Tex is the weight in grams of one kilometer of thread.

2000 denier = .222 g/m

230 tex = .230 g/m

For example, one pound of 2000 denier thread contains approximately 6,700 feet (2045 m) of thread.

#### THREAD CONSUMPTION

Thread consumption is the length of thread required to sew a linear seam, i.e., seven feet of thread is required to sew one foot of seam. Thread consumption rates for the two-thread and single-thread machines are as follows:

	Length Ratio
<u>Machine</u>	Thread:Stitch
Two-Thread, Double-Locked Stitch	7:1
Single Thread, Chain Stitch	4:1

#### **NEEDLES**

Needle size is critical to the efficiency of the sewing operation. Needles should be compatible with machine and sewing thread. Needles are available through sewing machine manufacturer representatives.

#### **SEWING MACHINES**

#### **MODELS**

Field seaming of geotextiles can be accomplished with the following types of sewing machines:

Single Thread, Chain Stitch
Union Special, American Newlong, or equal
(Federal Class 101)

(Refer to the Federal Standard on stitches, seams and stitching).

#### **SEAMS AND STITCHES**

#### **SEAMS**

Three seams that will provide optimum strength for geotextile sewing are the Flat (Prayer) seam, the "J" seam, and the Butterfly-folded seam.

When sewing a flat seam, the stitching should be approximately 1.5 inches from the outside edge of the fabric (not in the selvage or at the selvage edge). The "J" fold and Butterfly fold seams require a fold of 1.25 inches to 2 inches from the fabric edge with the stitching approximately 1 inch from the folded edge.

Care should be taken with either seam to assure that the two fabric edges are even during seaming.

Folders can be attached to the sewing machine to fold and guide the edges of the two fabric layers into the sewing head.

#### **STITCHES**

Seams should contain between four and seven stitches per inch to assure adequate strength.

#### **SKAPS Nonwovens**

#### HEAT SEAMING of Nonwoven Geotextiles

#### **HEAT SEAMING INSTALLATION**

On geotextiles seven (7) ounces per yard or heavier, fusion seaming with a heat gun may be used. The minimum overlap for this type of welding is four (4) inches. Prior to fusion seaming the geotextile together, the installer must demonstrate to the Field Engineer the ability to perform this type of installation. Areas burned through by fusion welding shall be properly repaired. Care should be taken during installation to prevent damage to the geotextile. Torn or punctured material shall be patched with sufficient overlap to prevent separation.

#### **SKAPS Nonwovens**

#### **SEWING PROCEDURE**

Fabric layers should be placed on the ground (preferably firm ground) so that the edges to be sewn are parallel and overlapping. This can be accomplished by a variety of placement techniques. The sewing operation typically requires three men; a machine operator and a man on each side of the machine to aid in fabric throughout. The lead man should hold the fabric edges evenly together and feed the fabric into the sewing machine head or folder. The man behind the machine should hold tension on the fabric so the machine operator has a taut and straight edge to sew across. All three men advance at the machine sewing speed.

If the machine misses a stitch or runs off the fabric, terminate the seam by cutting and tying the thread. Begin a new seam approximately one foot behind the broken seam.



#### CHEMICAL RESISTANCE OF POLYPROPYLENE GEOTEXTILES

SKAPS Industries nonwoven geotextiles are manufactured from polypropylene with ultraviolet stabilizing additives. The excellent chemical resistance of SKAPS Industries polypropylene geotextiles is one of the qualities which has established SKAPS Industries as a leading producer of geotextiles for use in the waste containment industry. This technical note addresses the chemical resistance of polypropylene with a focus on recent testing programs which have clearly demonstrated the durability of SKAPS Nonwovens fabrics in a variety of chemical environments.

Of the polymers used to manufacture geotextiles, polypropylene exhibits the greatest resistance to chemical attack. In fact, polypropylene is the polymer of choice for such commonly used products as synthetic grass for athletic fields, outdoor carpeting, battery cases, bleach bottles, antifreeze jugs, washing machine agitators, and thousands of other commonly used items that are routinely exposed to chemical environments. Polypropylene is stable within a pH range of 2 to 13, making it one of the most stable polymers available for manufacturing geotextiles. Polypropylene geotextiles have been found to be durable in a wide range of chemical environments, (Bell, et. al., 1980; Haxo, 1978, 1983; Pucetas, et. al., 1991; Tisinger, et. al., 1989). Research has found both woven and nonwoven polypropylene geotextiles to be nonbiodegradable and resistant to commonly encountered soil-bound chemicals, landfill leachates, mildew, and insects.

Numerous laboratory test programs have subjected polypropylene to severe chemical environments such as solutions of organic solvents, oils, organic acids and inorganic acids. The laboratory tests are generally performed in accordance with ASTM D 543. "Standard Test Method for Resistance of Plastics to Chemical Reagents". These test programs have found polypropylene to exhibit superb chemical resistance.

In the ASTM D 543 procedure, specimens are immersed in a concentrated chemical solution at a specified temperature for a specified exposure period. This test method exposes the polypropylene to extremely harsh conditions which are considerably more severe than those encountered in most civil engineering applications.

The chemical compatibility of geotextiles with leachates is determined by EPA Test Method 9090 (EPA 9090), "Compatibility Test for Wastes and Membrane Liners". This is the laboratory method used in the geotextile test programs. Geotextile samples are immersed in a constant temperature leachate bath for four months. At the end of each month, samples of the fabric are removed and subjected to physical testing. Changes in properties may indicate chemically imposed degradation.

In all testing programs there was no indication of geotextile degradation due to exposure to landfill leachates. These results demonstrate the excellent chemical resistance of polypropylene geotextiles and their suitability for use in waste containment applications.

#### HAZARDOUS WASTE LEACHATE

A laboratory testing program was performed to evaluate the chemical compatibility of polypropylene geotextiles with a hazardous waste leachate. The program included EPA 9090 testing of nonwoven specimens. The testing exposed the geotextile to leachate in both the laboratory and in a leachate collection sump at a hazardous waste landfill.

Test evaluation incorporated detailed microstructural analyses which are not typically incorporated into chemical resistance testing programs. Methods included differential scanning calorimetry, thermal gravimetric analysis, and infrared spectro-photometry. These analyses were performed to isolate any changes in the microstructure of the geotextile due to immersion in the leachate.

The results of this testing program found the geotextile microstructure remained intact, stable, and unchanged. These results demonstrate the superior chemical resistance of polypropylene geotextiles in hazardous waste applications.

#### **MUNICIPAL WASTE LEACHATE**

The chemical resistance of polypropylene geotextiles to municipal solid waste leachate was evaluated in laboratory testing programs. The testing programs evaluated changes in physical properties of the specimens, including dimension, thickness, grab tensile strength and elongation, puncture resistance, burst strength, and tear strength. In all cases there were no measurable changes in physical properties of the specimens after exposure to leachate.

All SKAPS Nonwovens geotextiles are equally resistant to chemical degradation because all are manufactured using the same polymer and additives. This conclusion is supported by the test results which demonstrated no difference in chemical resistance for different types of SKAPS Nonwovens geotextiles. This technical note is considered to be applicable to all SKAPS Nonwovens geotextiles regardless of weight, thickness, or strength.

## **SKAPS INDUSTRIES**

#### **Nonwoven Division**

Material Safety Data Sheet (MSDS)

#### Section 1 - Product Identification

#### Manufacturer's Name:

SKAPS Industries, Nonwoven Division 316 S. Holland Drive Pendergrass, GA 30567

#### **Emergency Phone Number:**

(706) 693-3440

Date Prepared:

October 21, 2003

#### Section 2 - Hazardous Ingredients

No hazardous components in geotextile fabrics at or above threshold limit values.

#### Section 3 – Physical/Chemical Characteristics

**Boiling Point:** 

Not Applicable

Vapor Pressure:

**Not Applicable** 

Specific Gravity:

0.90 - 0.905

**Melting Point:** 

120 - 170 Degrees (C)

Vapor Density:

**Not Applicable** 

Evaporation Rate:

Not Applicable

Solubility in Water:

Not Applicable

Appearance and Odor:

**Essentially Odorless** 

#### Section 4 - Fire and Explosion Hazard Data

Flash Point:

>600 Degrees (F)

Extinguishing Media:

Dry Chemical, CO2, Foam, Water, Haion

Special Fire Fighting Procedure:

Avoid inhalation of vapors. Use self-contained breathing

apparatus when fire fighting in confined areas.

**Unusual Fire and Explosion Hazards:** 

Treat as a solid that can burn. Generally burns slowly with

low smoke density and flaming drips. Burns with high

smoke density under certain conditions.

#### Section 5 - Reactivity Data

Material is stable.

Hazardous polymerization will not occur.

#### Section 6 - Health Hazard Data

**Primary Routes of Entry:** 

Inhalation - Negligible

Skin Contact - Negligible

Indigestion - Not applicable

Carcinogen:

Not a carcinogen

**Emergency and First Aid Procedure:** 

Eve Contact: Flush with water.

Skin Contact: Treat as thermal burn if contact with

molten.

#### Section 7 - Precautions for Handling and Use

Practice reasonable care and caution in handling.

Waste Disposal:

Place in appropriate disposal facility in compliance

with local regulations.

Storage:

In cool, dry location away from oxidizing materials.

#### Section 8 - Control Measures

Use NIOSH respirators when hot/molten product.

**Protective Gloves:** 

Required when handling molten product.

Practice general hygiene by washing hands and clothes after handling.



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# QUALITY CONTROL PROGRAM OUTLINE



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#### **GENERAL**

#### **Scope**

The following describes parameters for the manufacture, supply, and installation of SKAPS Industries Drainage Net and Geocomposite. SKAPS Industries is dedicated to manufacturing the finest quality geosynthetics under the most rigorous testing protocol.

#### Qualifications

SKAPS Industries has successfully manufactured over 100,000,000 square feet of polyethylene drainage net each of the past ten years. SKAPS Industries operates three state-of-the-art Geonet Extrusion Lines. This ensures that our customers who have special project-specific requirements are serviced without interfering with standard daily production.

#### **Manufacturing Quality Assurance**

SKAPS Industries maintains laboratories at each of our manufacturing facilities. These Facilities maintain strict quality control over our products using the best and latest in testing equipment and techniques. The quality control testing laboratory is designed around the latest GRI and ASTM procedures and standards.



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#### **MATERIALS**

#### **Drainage Net**

The drainage net is manufactured by extruding two sets of polyethylene strands to form a three dimensional structure to provide for planar flow. The drainage net is manufactured with virgin polyethylene resin manufactured specifically for the intended application. The natural polyethylene resin without the carbon black shall meet the following requirements:

Property Test Method Requirements Density, g/cc ASTM D 1505 > 0.94 Melt Index, g/10 min. ASTM D 1238 < 1.0

The drainage net is manufactured in Commerce GA. Labels on each roll shall identify the thickness of the material, the width and length of the roll, roll number, and name of the manufacturer.

#### Geotextile

The geotextile shall be a non-woven, needle punched polypropylene fabric manufactured by SKAPS Industries. SKAPS nonwoven geotextile is a superior quality, nonwoven geotextile produced by needlepunching together 100% polypropylene staple fibers in a random network to form a high strength dimensionally stable fabric. The polypropylene fibers are specially formulated to resist ultraviolet light deterioration, and are inert to commonly encountered soil chemicals. The fabric will not mildew, is non-biodegradable, and is resistant to damage from insects and rodents. Polypropylene is stable within a ph range of 2 to 13.

#### Geocomposite

The geocomposite shall consist of the SKAPS Industries HDPE drainage net heat bonded to one layer or sandwiched between two layers of geotextile to create a single-sided or double-sided geocomposite. The geotextiles shall extend 6 inches beyond the edges of drainage net on both sides of the geocomposite roll.



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#### **GEONET / GEOCOMPOSITE TESTING PROCEDURES**

#### **OC Sampling Schedule**

All tests are performed every 35,000 square feet of production except for compressibility, which is tested once per shift (approximately every 250,000 square feet of production). Transmissivity is done on a requested basis.

#### Weight / Area (ASTM D 5261)

The width is determined by measuring the sample in three places--once across each cut end and once across the center. The three measurements are then averaged and reported in inches. The length is also determined by measuring three places--along both edges and along the center. These values are averaged and reported in inches. Samples are then taken and weighed to the nearest .001 lb/sf. The weight is divided by the average width to obtain a weight per length value. The weight/length number is divided by the average width value to obtain weight per area. The value is reported in lbs/sf.

#### Thickness (ASTM D 5199)

Five specimens are cut from across the width of the lab sample. A thickness gauge with a 3/4 inch presser foot is used to measure the thickness of each specimen. The values are recorded and reported as an average in inches.

#### Tensile Strength (ASTM D 5035)

Five specimens are cut from across the width of the lab sample. They are then placed in the jaws of the Instron Machine and a load is applied at a constant strain of 12 in/min until yield. The results of the tensile test are then averaged and recorded.



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#### % Carbon Black (ASTM D 4218)

The carbon black test determines the percent by weight of the product that is carbon black. The percent of carbon black is the ratio of the residue weight after pyrolysis in a muffle furnace compared to the weight of input specimen. Two grams of the net are cut and placed in aluminum dishes. The samples are then placed in a muffle furnace for ten minutes at 600 degrees centigrade. The samples are removed and allowed to cool. The carbon black percentage is calculated and recorded.

#### Ply Adhesion (ASTM D 7005)

Five specimens are cut from across the entire width of the composite sample, each measuring one inch wide by ten inches long. The strain rate for the test is 10 in/min. The fabric is clamped in one jaw of the Instron machine while the net is clamped in the other. The fabric is pulled away from the net to test the adhesion of the fabric to the net.

#### **Transmissivity (ASTM D 4716)**

The transmissivity test for the composite is identical to the test for the geonet.

#### Melt Index (ASTM D 1238)

The melt index determines the rate of the extrusion of the molten resin through a die of specified length and diameter at a temperature of 190 degrees centigrade under a load of 2.16 kg and is measured in g/10min. A sample of approximately 2.5 grams of geonet is then put through the melt plastometer to verify flow rates.

#### **Density of Polymer (ASTM D 1505)**

Taking samples from the melt index test, small strands are cut and measured in a density column. A mixture of distilled water and isopropyl alcohol is used as the suspension fluid.



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#### **Transmissivity (ASTM D 4716)**

The transmissivity test measures the inplane flow of water across the net sample. In the standard test, the sample is placed between two steel plates with the water temperature at 20 degrees centigrade. Different gradients and loads are applied to the sample. The values are then calculated and converted to gallons per min/ft, or meters²/sec. Transmissivity is not a standard manufacturing quality control test but rather a design indicator and is tested on a per project request basis.



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## TRANSNET

**DRAINAGE NET** 

**GEOCOMPOSITE** 

HANDLING AND INSTALLATION
MANUAL



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#### Introduction

Geocomposites provide a solution to various drainage problems. As with any synthetic product, the quality assurance and quality control does not stop once the product is shipped from the factory. Whether the product has been specified for vertical wall hydrostatic relief or horizontal flow zones for landfill cells/closure and roadways, care in handling and installation is critical to the future functioning of the product.

TRANSNET is manufactured utilizing high quality HDPE resin and lamination of high strength to weight ratio nonwoven geotextiles. The lamination process is completed at the same location where the net is manufactured, minimizing additional handling and allowing for supply of custom lengths. TRANSNET can have one or both sides laminated in order to meet the design specification.

#### <u>Manufacturing</u>

TRANSNET is manufactured utilizing state-of-the-art counter rotating dies and the highest quality resin. TRANSNET is manufactured with the addition of carbon black to stabilize against degradation from UV exposure.

#### Packaging

Upon completion of the lamination process, the geocomposite will be wrapped in an opaque wrap to prevent exposure to UV and for protection from the weather, dust, etc. In the event only TRANSNET is required, shipping in a wrapper is not necessary.



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Each roll will be labeled or tagged so that the following information is available at all times from the manufacturer:

- Manufacturer's Name
- Product Identification
- Lot Number
- Roll Dimensions

#### **Shipping and Storage**

Geocomposite rolls will be shipped in original packaging. In the event the packaging is damaged during shipment, repairs should be made to ensure protection against UV and weather. Care should be used during the off loading to ensure that the machinery used does not penetrate packaging.

Storage of the rolls prior to installation should be in an area where they are not in standing water. For storage longer than 30 days, rolls should be elevated off the ground with tires, pallets or 2x4's to prevent water from saturating the bottom row. The stack should then be covered with a material that will give additional protection from the elements. Should the product be exposed to excessive dust, the product should be washed prior to installation.

#### Site Preparation

The design engineer will determine how and where the geocomposite is to be utilized. With any application, care should be used in placing net or composite so that it is not damaged by stones or other protrusions that may compromise the functionality of the product.



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#### **Installation**

TRANSNET should be installed by hand. Once the roll is delivered to the installation location via rubber-tired loader or other appropriate machinery, the rolls should be inspected for any damage from shipping or handling. Once the rolls are positioned, they should be unrolled by hand. For slope applications, the rolls should be rolled from top to bottom and hand tightened to remove any wrinkles. The TRANSNET portion of adjacent rolls shall be overlapped two to four inches or according to the Engineer's recommendation. When placing TRANSNET end to end, overlap in shingle placement fashion a minimum of one foot. For end-to-end placement, the top layer of geotextile shall be peeled back and excess TRANSNET will be trimmed so that the top layer of geotextile covers the attachment of the two layers of geocomposite. The TRANSNET will be attached to adjacent rolls utilizing plastic wire ties. These ties will be placed at a maximum spacing of 5 feet along the sides of the rolls and a maximum of 2 feet for end to end attachment, or according to the Engineer's specification.

Metal ties or hog rings are not to be used.

#### **Anchoring**

For slope applications, TRANSNET should be placed in a trench so that pull out or slippage is prevented. The trench should be in accordance with the Design Engineer's requirements. Sand bags should be on hand at all times and placed on edges not seamed to prevent uplift from the wind. Welding of the TRANSNET to HDPE liner or any other geomembrane is not recommended.



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#### **INSTALLATION GUIDELINES**

Nonwoven Geotextile, Nets and Composites

#### **Heat Seaming**

#### Nonwoven Separate or Laminated

Nonwoven geotextiles can be joined together by using fusion seaming methods. The minimum overlap for this type of welding is four inches. Prior to fusion seaming the geotextile together, the installer must demonstrate to the Field Engineer the ability to perform this type of installation method. Areas burned through that are damaged by fusion welding shall be properly repaired. Care should be taken during installation to prevent damage to the geotextile. Torn or punctured material shall be patched with sufficient overlap to prevent separation.

#### Sewing Procedure

#### Nonwoven Separate or Laminated

Fabric layers should be placed on the ground (preferably firm ground) so that the edges to be sewn are parallel and overlapping. The sewing operation typically requires three men--a machine operator and a man on each side of the machine. The lead man should hold the fabric edges evenly together and feed the fabric into the sewing machine head or folder. The man behind the machine should hold tension on the fabric so the machine operator has a taut and straight edge to sew across. If the machine misses a stictch or runs off the fabric, terminate the seam by cutting and tying the thread. Begin a new seam approximately one foot behind the broken seam.



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#### **Overlapping**

Nonwoven Separate or Laminated

Roll goods form of geotextile should be overlapped a minimum of 12". Care should be taken that roll goods remain parallel to each other. Extreme care should be taken to assure that soil does not intrude into the composite structure thus clogging the drainage net.

### **SKAPS Industries**

571 Industrial Parkway Commerce GA 30529 Phone (770)564-1857

#### DRAINAGE PRODUCT DESCRIPTION SHEET TRANSNET 220-2-6

Transnet 220-2-6 is a superior quality drainage media made by extruding two sets of HDPE strands together to form a diamond shaped net. The net is then heat laminated on two sides to a 6 ounce non-woven fabric. This three dimensional structure provides excellent planar liquid flow. SKAPS drainage geocomposites are manufactured from first quality virgin resin geonets and a full range of nonwoven geotextiles. The Transnet 220-2-6 certifies to the physical property values listed below:

NET PROPERTY	TEST METHOD	UNITS	MINIMUM AVERAGE ROLL VALUE		
Mass Per Unit Area	ASTM D-5261	lbs/ft²	0.162 / .		
Thickness	ASTM D-5199	inches	0.220 +/02		
Density of Polymer	ASTM D-1505	g/cm²	0.94		
Carbon Black	ASTM D-4218	%	2-3		
Melt Index	ASTM D-1238	g/10min.	1 max		
Tensile Strength	ASTM D-5035	lbs/in.	45		
Ply Adhesion	ASTM D-7005	lb/in	1		
Transmissivity (geocomposite)	ASTM D-4716	gpm/sf	1 x 10 <sup>-4</sup> *		

Transmissivity measured using water at 20 Degrees C with a gradient of 0.1 between steel plates, under a confining pressure of 10,000 psf, after 15 minutes. Values may vary based on dimension of the transmissivity specimen and specific laboratory.

#### STYLE SKAPS GE160

SKAPS GE160 is a superior quality, nonwoven geotextile produced by needlepunching together 100% polypropylene staple fibers in a random network to form a high strength dimensionally stable fabric. The polypropylene fibers are specially formulated to resist ultraviolet light deterioration, and are inert to commonly encountered soil chemicals. The fabric will not mildew, is nonbiodegradable, and is resistant to damage from insects and rodents. Polypropylene is stable within a ph range of 2 to 13. SKAPS GE160 conforms to the physical property values below:

FABRIC PROPERTY	TEST METHOD	UNITS	MINIMUM AVERAGE ROLL VALUE
Weight	ASTM D-5261	oz.	6.0
Grab Tensile	ASTM D-4632	lbs	160
Grab Elongation	ASTM D-4632	%	50
Trap Tear •	ASTM D-4533	lbs	65 •
Puncture	ASTM D-4833	lbs	95
Water Flow Rate	ATMD D-4491	gpm/ft²	125
Permittivity*	ASTM D-4491		1.63
Permeability*	ASTM D-4491	cm/sec	0.48
AOS	ASTM D-4751	US Sieve	70 max
UV Resistance	ASTM D-4355	% hrs	70 @ 500
Roll Size			14.5' x 250'

At time of manufacturing. Handling may change these properties.

To the best of our knowledge the information contained herein is accurate. However, ESP, Inc. cannot anticipate all conditions under which ESP's product information and our products, or the products of other manufacturers in combination with our products, may be used. We accept no responsibility for results obtained by the application of this information or the safety or suitability of our products either alone or in combination with other products. Final determination of the suitability of any information or material for the use contemplated, of its manner of use, and whether the suggested use infringes any patents is the sole responsibility of the user.



## TEGINIONEDATASHIFI

## Stoppensche Keltenschemens

Sofmax International Inc., 2801 Bont, Marie-Victorin, Varennes, Qc, Cauada, J5X 1P7 Tel.: (450) 929-1234 Fax: (450) 929-2550 www.sofmax.com

PROPERTY	TEST METHOD	FREQUENCY (1)	UNIT Imperial	Solmax 440T-1000
SPECIFICATIONS				
Thickness (min. avg.)	ASTM D-5994	Every roll	mils	38.0
Lowest individual for 8 out of 1	0 values		mils	36.0
Lowest individual for 10 out of	10 values		mils	34.0
Asperity Height (min. avg.) (3)	ASTM D-7466	Every roll	mils	15
Resin Density	ASTM D-1505	1/Batch	g/cc	> 0.932
Melt Index - 190/2.16 (max.)	ASTM D-1238	1/Batch	g/10 min	1.0
Sheet Density	ASTM D-1505	Every 2 rolls	g/cc	> 0.94
Carbon Black Content	ASTM D-4218	Every 2 rolls	%	>2.0 / <3.0
Carbon Black Dispersion	ASTM D-5596	Every 6 rolls	Category	Cat. 1 & Cat. 2
Oxidative Induction Time (min. ave)	ASTM D-3895	1/Batch	min	100
Tensile Properties (min. avg) (2)	ASTM D-6693	Every 2 rolls	a see a saar aan aan aa aa ah aa ah aa ah ah ah ah ah ah ah	to the time of the control of the second of the control of the con
Strength at Yield			ppi	88
Elongation at Yield			%	12
Strength at Break			ppi	88
Elongation at Break			%	150
ear Resistance (min. avg.)	ASTM D-1004	Every 6 rolls	lbf	30
uncture Resistance (min. avg.)	ASTM D-4833	Every 6 rolls	lbf	90
Dimensional Stability	ASTM D-1204	Every 6 rolls	%	± 2
tress Crack Resistance (SP-NCTL)	ASTM D-5397	1/Batch	hr	400
ven Aging - % retained after 90 days	ASTM D-5721	Per formulation		
HP OIT (min. avg.)	ASTM D-5885		%	80
V Resistance - % retained after 1600 h	r GRI-GM-11	Per formulation		
HP-OIT (min. avg.)	ASTM D-5885		%	50
Supremersed on the state of the	dikdinensions may yan t			
Roll Dimension - Width			ft	22.3
Roll Dimension - Length	-	<i>_</i>	ft	780
Area (Surface/Roll)	-	, 	sf	17 394

#### **NOTES**

- 1. Testing frequency based on standard roll dimensions and one batch is approximately 180,000 lbs (or one railcar).
- 3. Of 10 readings; 8 out of 10 must be >7 mils (0.18 mm), and lowest individual reading must be >5 mils (0.13 mm). ASTM D7466 is identical to GRI-GM12.
- 2. Machine Direction (MD) and Cross Machine Direction (XMD or TD) average values should be on the basis of 5 specimens each direction.
- \* All values are nominal test results, except when specified as minimum or maximum.
- \* The information contained herein is provided for reference purposes only and is not intended as a warranty of guarantee. Final determination of suitability for use contemplated is the sole responsability of the user. SOLMAX assumes no liability in connection with the use of this information.

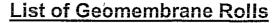
MF-CQ-34 (Rev. 02 / 10-04-07) Revision Date: 2010-04-07



2801, Boul. Marie-Victorin

Varennes, Quebec, Canada, J3X 1P7

Tel.: 1-450-929-1234 Tel.: 1-800-571-3904 Fax: 1-450-929-2547



MF-CQ-01

Rev. 05/ 21 mars 2006

Project Name: SOUTH PLAINFIELD, NJ

Project Number: 7073

Reference Number:

104818

Invoice Number:

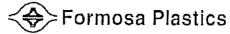
210257

Roll Numbe	r Product Code	Resin Lot Number	Manufactured Date		<i>P-NCTI</i> TM D5397	
			<u>.</u>	Specification	Result	Roll Tested
2-59453	Solmax 440-1000	11H1298	13-déc-11	>400	500	2-59428

Quantity (rolls):

1

	<u>Resin</u>	Certification		
Resin Lot	Melt index ASTM D1238 g/10min	Density ASTM D1505 g/cc	OIT ASTM D3895 min	HP-OIT ASTM D5885 min
11H1298	0.070	0.937	120	N/A



#### FORMOSA PLASTICS CORPORATION, TEXAS

201 FORMOSA DRIVE

FO BOX 700 POINT COMFORT

TX 77978

PHONE: ( 888 ) FPCUSA3

#### Certificate of Analysis

CUSTOMER: SOLMAX INTERNATIONAL INC.

2801 MARIE-VICTORIN

S/O NO : EL9A708

CUSTOMER PO : 108042-0

DATE SHIPPED: 10/24/11

VARENNES

QC J3X 1

LOT NO : 11H1298

FRODUCT : DF3812A

WEIGHT :

196,750.00

RAILCAR

FPAX980250

CUSTID: FT03B2B

SPIDE3

TEST ITEM

REFERENCE METHOD

TEST VALUE

Melt Index,g/10min HLMI, g/10 min. Density, g/cm3

ASTM D1238 ASTM D1238 ASTM D1505 .070 10.2 .9373

Notes:

\* OIT AT 200 DEGREES C IS GREATER THAN 120 MINUTES

Chantal Gagnon பிருந்திய 2011.11.0 4 14:05:32 -04'00'

linda Kas

QC SUPERVISOR: LINDA KAO



Product: Solmax 440-1000

2801, Boul. Marie-Victorin

Varennes, Quebec, Canada, J3X 1P7 Tel.: 1-450-929-1234

Tel.: 1-800-571-3904 Fax: 1-450-929-2547 Manufacturing Quality Control Test Results - Rolls

MF-CQ-11

Rev. 04/ 10-12-04

Project Name:

SOUTH PLAINFIELD, NJ Project Number: 7073

104818

Ref. Number:

Invoice Number: 210257

Properties Unit Test Method Frequency Specification	Thickness ave / min. mils D5199 Each roll 40 / 36	Geomembrane Density g/cc D1505/D792 1/2 ro > 0.94	Carbon Black Content % D4218 1/2 ro > 2.0 / < 3.0	Carbon Black Dispersion Cat. 1 and Cat. 2 D5596 1/6 ro Cat. 1 _Cat. 2	Strength ppi		Strength ppi ision (D 638, 2 ro	eak Elong. % Type IV) 700	Tear Resistance lbs D1004 1/6 ro 28	Puncture Resistance lbs D4833 1/6 ro 80	Dimension. Stability % D1204 1/6 ro ± 2	Asperity Height in / out mils N/A
2-59453 MD XD	41 / 41	0.946	2.6	10 /10 Views	102.8 111.9	19.8 15.0	216 219	807 903	37.2 33.4	118.8	-0.45 0.09	/



Project Number: 7086

Salmax International Inc. 2801, Boul. Marie-Victorin Varennes, Quebec, Canada, J3X 1P7

Tel.: 1-800-571-3904 Fax: 1-450-929-2547

List of Geomembrane Rolls

MF-CQ-01 Rev 06/ 2011-12-23

ACCEPTED

Reference Number:

104875

Invoice Number:

210337

Roll Number	Product Code	Resin Lot Number	-  Manufactured Date	Resin Melt Index 190/2.16 g/10 min D1238	Resin Density g/cc D1505	OIT  Spec Result  min  D3895	HPOIT  Spec Result min D5885	ESCR SP-NCTL Spec Roll Tested hours D5397
2-60554	Solmax 440-1000	8210746	27-janv-12	0.15	0.935	100 182		>400 2-60506 PASS

Quantity (rolls):

Project Name: SOUTH PLAINFIELD, NJ



Solmax International Inc. 2801, Boul. Marie-Victorin Varennes, Quebec, Canada, J3X 1P7

Manufacturing Quality Control
MF-CQ-11 Rev. 05/2011-12-23

## Test Results - Rolls

Tel.: 1-800-571-3904 Fax: 1-450-929-2547

Project Name: SOUTH PLAINFIELD, NJ

Project Number: 7086

Reference Number:

Invoice Number:

104875 210337

1

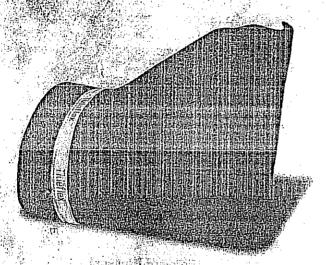
Product: Solmax 440-1000

Properties	Thickness ave / min.	Geo- membrane Density	Carbon Black Content	Carbon Black Dispersion	Yie Strength	eld	isile Br Strength	eak Elong,	Tear Resist.	Puncture Resist.	Dimension. Stability	Asperity Height in / out
Unit Test Method Frequency	mils D5199 Each roll	g/cc D1505/D792 1/2 ro	% D4218 1/2 re	Cat. I and 2 D5596 I/6 ro	ppi	% D66 1/2		%	lbs D1004 1/6 ro	lbs D4833 1/6 ro	% D1204 I/6 ro	mils N/A
Specification	40 / 36	> 0.94	> 2.0 / < 3.0	Cat. 1 _ Cat. 2	84	13	162	700	28	80	± 2	
2-60554 MD XD	41 / 39	0.949	2.9	10 /10 Views	98.0 103.3	19.7 16.4	211 215	877 968	33.3 30.6	108.2	-0.53 0.05	/

## Tiden exp Division of Fixed Valve Inc.

# VALUE CHECK VALUES

## INSTALLATION, OPERATION, AND MAINTENANCE MARUAL

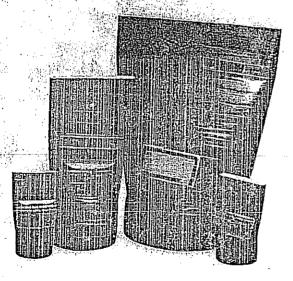


36 TF-1

The revolutionary design of the all rubber Tideflex® Check Valve provides reliable backflow protection. This unique "duck bill" design eliminates costly back-flow from oceans, rivers or storm water and is the ideal valve for effluent diffuser systems.

Tideflex® Valves seal on entrapped solids and debris without jamming. Unlike traditional flap gates there are no hinged gates to hang open and no warping or freezing. It's virtually maintenance-free.

The Tideflex® Check Valve is available in a wide variety of elastomers and is designed to meet your exact flow specifications.



TF-2

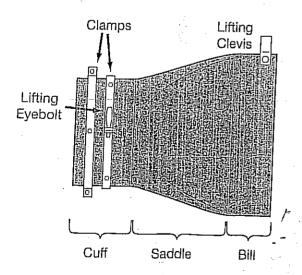
#### **IMPORTANT**

Please take a moment to review this manual. Before performing any maintenance on the valve be sure the pipeline has been de-pressurized. The improper installation or use of this product may result in personal injury, product failure, or reduced product life. Tidefiex® Technologies can accept NO liability resulting from the improper use or installation of this product. If you have any questions or problems, please call the customer service department at (412) 279-0044. We appreciate your comments. Thank you for choosing Tideflex® Technologies.

#### GENERAL DESCRIPTION

The Tideflex® Technologies' Tideflex® Check Valve is an all-elastomer, one-piece check valve. Terms used in this I.O.M. to refer to various parts of the valve are described below.

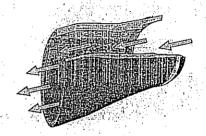
- 1. **Guff** The Cuff is designed with a full round bore and slips over the end of the pipe.
- 2. **Saddle** The Saddle is the middle part of the valve, tapening from the round cuff to the flat bill. The Saddle directs the flow to the bill, and is flexible to sustain increased flow conditions.
- 3. **Bill** The Bill is the discharge end of the valve. The Bill flexes to allow flow to discharge, yet is stiff enough to prevent the valve from opening without line pressure. Back pressure pressure created on the exterior of the valve by reverse flow or submersion will seal the lips of the bill tightly together, preventing backflow into the valve.
- 4. Clamps The clamps are tightened around the Cuff after the Cuff has been slipped over the end of the discharge pipe. These clamps are normally furnished by Red Valve Company, Inc. Hose clamps are supplied for valves up to 12". Valves 14" and up are supplied with fabricated clamps. 14"-20" are supplied with one set, 20"-54" are supplied with two sets and sizes 60" and up are supplied with three sets.
- 5. Lifting Clevis A lifting clevis is attached to the Bill of the Check Valve for valves 36" and up. This clevis is used during installation to assist in lifting the valve, and may be used to attach a line to the bill to help support the valve after installation.



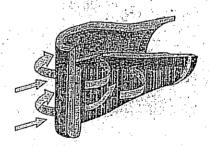
#### **OPERATION**

Tideflex® Check Valves are custom made products intended for a specific application and haves been designed to respond to criteria unique to that purpose, such as line pressure, minimum and maximum back pressure and chemical compatibility. Should the conditions for which the valve has been designed be altered or change in any way, it could affect the normal operation of the valve.

Tideflex® Check Valves work on backpressure exerted on the bill area to seal the valve. The bill may appear to be slightly open when installed. This slight opening does not affect the operation of the valve, as the valve depends on backpressure to seal.



Forward Pressure Opens Valve



Reverse Pressure Seals Valve

NEVER...
Cut or modify check valve.

DO.,

Use a soapy water solution to slide Tideflex® on pipe.

DO...

Keep valve on pallet until ready to install.

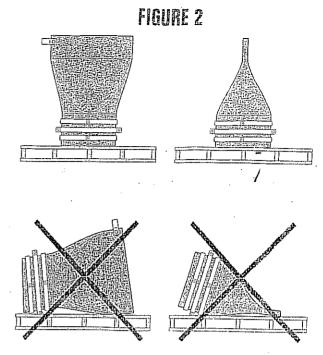
DO...

Tighten clamp bolts evenly.

#### STORAGE

Tideflex® Check Valves should be stored in a cool, dry location on original shipping pallet with the bill facing upward (not on side) (Figure # 2). Do not drop, bend or twist Check Valve or damage may occur.

- 1. Store valve in a cool, clean, dry location.
- Avoid exposure to light, electric motors, dirt or chemicals. Resilient Check Valves are subject to deterioration when exposed to ozones and noncompatible chemicals. Ozone especially causes age hardening of the elastomer.
- **3.** Store Installation Operation Manual with pro-duct so it will be readily available for installation.
- **4.** Do not remove wooden brace or metal "shipping ring" (36"+) until valve is installed.



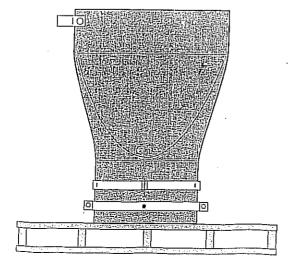
**NEVER STORE HORIZONTALLY** 

## INSTALLATION INSTRUCTIONS — LARGE DIAMETER TIDEFLEX® CHECK VALVES 24" AND OVER

#### 1. INSPECTION OF CHECK VALVE:

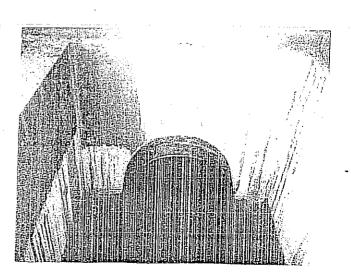
Check the inside diameter of the Cuff of the Tideflex® Check Valve to compare it to the O.D. of the outfall pipe. Inspect the outfall pipe for sharp or damaged areas. The Pipeline should be in a smooth condition to prevent cutting the Rubber Check Valve. Lifting clevis and Lifting Eye Bolts are provided only for sizes 36" vided only for sizes 36" and over.

Imperfections on the inside of the cuff area can be filled with a silicone sealant prior to installing the valve on the pipe. This will ensure a seal in the cuff area after clamps are tightened.



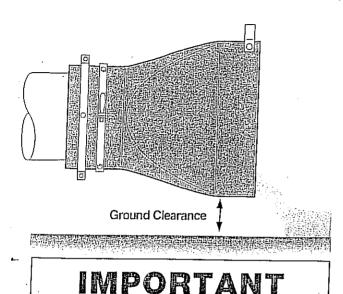
#### 2. INSPECTION OF THE PIPE

Check the outside diameter of the pipe to determine if it matches the I.D. of the Cuff of the Tideflex® Check Valve. The Cuff of the Check Valve is usually made slightly larger to permit ease of installation.



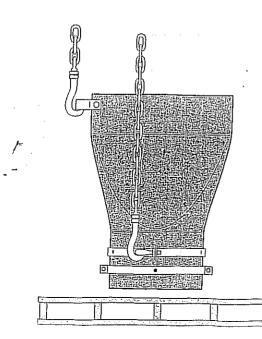
#### 3. CLEARANCE

Make certain that sufficient ground clearance exists below the valve, at least 10% of the valve diameter. (I.E. 6" for a 60" valve)



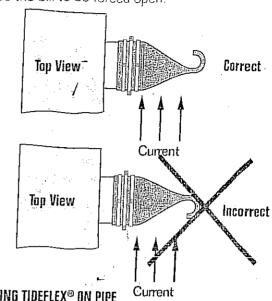
## 5. REMOVING THE VALVE FROM PALLET OR CRATING

A lifting clevis is provided at the top end of the Tideflex® Check Valve. Lifting eye bolts are provided on the clamps. Remove the cuff retainer "Shipping Ring" or wooden brace located inside the Cuff of the valve. The valve should be lifted from the pallet using both the clevis and the lifting eye bolts.



#### 4A. TIDEFLEX® WITH CURVED BILL INSTALLATION IN CURRENT

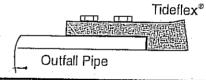
For Tideflex® fabricated with a curved bill, the valve should be installed so the bill points in the direction of the current, not facing the current which may cause the bill to be forced open.



4B. FITTING TIDEFLEX® ON PIPE

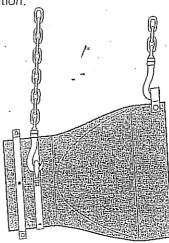
A. To facilitate the insertion of the pipe into the Tideflex® Check Valve, it might be necessary to grind a bevel on the inside cuff diameter.

B. Sometimes it is necessary to grind the inside of the cuff or add gasket material to the O.D. of the pipe to properly fit the Tideflex® Check Valve



#### 6. LIFTING THE VALVE

Do not discard the metal clamps holding the valve onto the pallet; THESE CLAMPS ARE NEEDED to install the Tideflex® Check Valve. In lifting the Tideflex® Check Valve from the pallet, keep the bill end of the Tideflex® higher than the cuff for ease of installation.



#### 7. POSITIONING THE VALVE

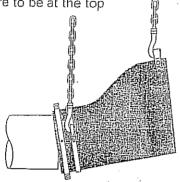
Apply a soap/water solution to the outside of the pipe in which the check valve is being installed on, to ease installation.

#### **TF-2**

With the bill end of the Tideflex® lifted higher than the cuff end start to fit cuff on the outfall line. The Tideflex® Check Valve should fit snugly against the outfall pipe, leaving no gap.

#### TF-1

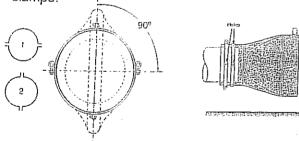
Flat portion of the valve to be at the bottom of the pipe. Flare to be at the top



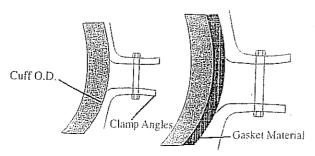
After the unit is securely pegged into position, proceed to install and tighten the first clamp. A mild lubricant may be applied to the I.D. of the **clamp** to prevent a brake shoe effect when tightening down clamps.

#### 9. POSITIONING FOR 2 CLAMPS

Install the second clamp on the cuff of the Tideflex. Rotating the clamp 90° in relation to the first clamp will ensure even pressure around the valve and pipe, thus increasing the effectiveness of the clamps.

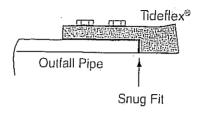


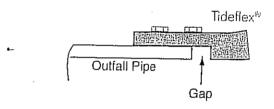
If a greater distance between the "angles" of the clamps is required to provide more range for tightening the bolts (especially if angles are bottoming out), gasket material can be wrapped around the OD of the cuff as shown.



#### 8. SEAT TIDEFLEX® ON PIPE

The Tideflex® Check Valve should fit snugly against the outfall pipe, leaving no gap. If possible, inspect installation from the inlet end of the Tideflex® Check Valve to insure that the Check Valve Cuff fits snugly on the pipe. Do not allow a gap between the cuff and the end face of the outfall pipe. A gap will create an imbalance which will not provide proper support for the Tideflex® Check Valve. For more information, see troubleshooting.

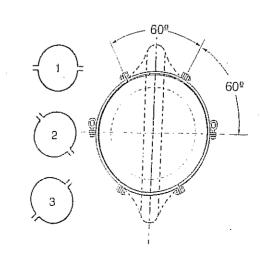




#### 10. POSITIONING FOR 3 CLAMPS

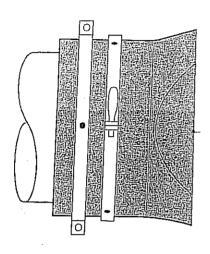
After the unit is securely pegged into position, proceed to install and tighten the first clamp. A mild lubricant may be applied to the I.D. of the **clamp** to prevent a brake shoe effect when tightening down clamps.

Install the second and third clamps on the cuff of the Tideflex®. Rotating the first and second clamps 60° and 120°, respectively, in relation to the first clamp will ensure even pressure around the valve and pipe, thus increasing the effectiveness of the clamps.



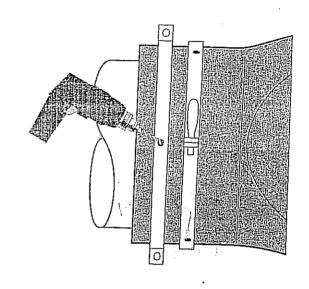
#### 11. POSITIONING BLANK HOLES IN CLAMPS

Tighten all clarnps and bolts once all components have been positioned properly. Pre-drilled holes are drilled in each clamp. These are provided so as to secure the Tideflex® Check Valve with "holding pins" to the outfall pipe. This will secure the Tideflex® Check Valve to the pipe and assure a long, trouble-free service life. After tightening the clamps, the pre-drilled holes should be staggered. Holes are not drilled in the/rubber cuff of the Tideflex® at the factory since they would **not** line up to the tightened clamps.



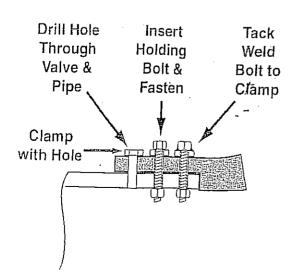
### 12. TACK WELDING HOLDING BOLTS TO CLAMPS

Once clamps are secure use a standard steel drill bit and drill holes through the rubber cuff. Insert holding bolts through the cuff and secure opposite side with nut, if possible. Holding bolts should be stainless steel. Steel bolts can corrode and break off, causing the Check Valve to slip off the pipe. Holding bolts are not provided because of various widths of the outfall pipe.



#### 13. BOLTS TACK WELDED TO CLAMPS

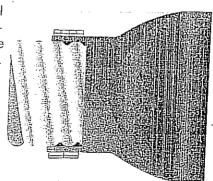
After tightening, heads of holding bolts can be tack welded to the clamps using small tacks. Certain installations will not permit installing of nuts to bolts. In these situations, the tightness of the clamps and tack weld of the bolts will assure good support.



## 14. CORRUGATED PIPE AND SMOOTH WALL (PVC, HDPE) PIPE INSTALLATION

For installation on corrugated pipe it is recommended that the corrugations be filled with hydraulic cement (or similar material) that will provide a smooth O.D.

For smooth wall pipe it is recommended that the valve be pinned.



#### TROUBLESHOOTING

#### Valve will not fit to pipe

- Make certain that the inside cuff retainer ring has been removed prior to fitting the valve to the pipe.
- Verify that the valve has enough area to fit over the pipe.
- If the pipe can be removed, or if an adapter ring which bolts to the wall or inside a vault is used, a crane or high-lift may be used to lower the valve onto the ring with the valve turned on end and the bill facing up.

## Valve will not close fully, or check flow in opposing direction

- Possible obstruction in line. Inspect the valve for entrapped foreign objects which may have lodged between the lips of the valve.
- Valve may not be installed high enough to clear the ground under the bill. Ensure that there is enough space between the bottom of the valve and the ground in order to prevent contact of the two or debris buildup.
- Back-pressure may not be sufficient to completely seal the valve.
- The Valve may not have been installed in a vertical position.

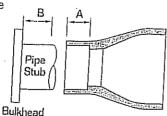
#### Valve will not stay on pipe

- Check all clamp bolts to assure that all bolts are tightened sufficiently.
- Valve may not be fully seated onto outfall line.
- Clamps are not rotated 90° from each other in order to provide adequate holding power.
- Valve cuff has a much larger I.D. in relation to pipe O.D.
- Make sure holding pins are used on 42" and larger Check Valves in order to prevent the valve from slipping off the line.

TF-2 Check Valves are designed to slide over a pipe stub. Too short of a pipe stub may cause the Check Valve to slip off a pipe stub may cause the Check

Valve to slip off or cause the Check Valve to gap open.

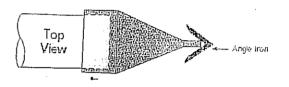
For valves up to 4", the pipe stub length "B" should be a minimum of 1/2" longer than cuff depth "A".

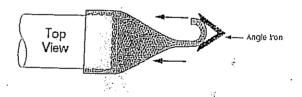


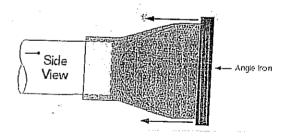
6"-14"	1" longer
16"-24"	2" longer
30"-60"	2 1/2" longer
72" and up	3" longer

#### \* Hints to install large diameter check valves

During the installation of the check valve, if force is needed to seat the valve to the cuff stop on large diameter check valves, the force required should be induced equally around the cuff of the check valve, never at only the top, bottom or in the center. The force required to push the check valve onto the pipe can be placed on the bill but it should be distributed evenly over the entire length of the bill. Failure to distribute the pressure equally may cause improper performance of the check valve. Use a wide angle iron or large wooden planks across the bill to distribute the force equally.







#### MAINTENANCE

Line pressure should flush the valve clean of debris in most cases. Periodic inspections for trapped debris should be conducted.

In vacation seashore areas quart size plastic bottles have a tendency to float on top and not flush through except during a major storm.

A feathered 1" x 4", 1-1/2" x 12", or syitable plank inserted into the bill of the valve and turned 90° is a simple method of clearing the Check Valve of small debris which may be trapped between the lips.

CAUTION: Sharp objects should not be used on the Tideflex® as there is a chance of cutting the rubber and damaging the protective fabric covering.

Any gouges in the cover wrap that occur should be sealed to safeguard against ozone or chemical attack. This is best done with rubber cement or a good brand of silicone or polyurethane rubber sealer made by the major manufacturers.

### Tideflex® Technologies Warranty

WARRANTIES - REMEDIES - DISCLAIMERS - LIMITATION OF LIABILITY
Unless otherwise agreed to in writing signed by Tidellex® Technologies, all Products supplied by Tidellex® Technologies will be described in the specifications set forth on the face hereof.

THE WARRANTIES SET FORTH IN THIS PROVISION ARE EXCLUSIVE AND IN LIEU OF ALL OTHER WARRANTIES WHETHER STATUTORY, EXPRESS OR IMPLIED (INCLUDING ALL WARRANTIES OF MERCHANTABILITY AND FITNESS FOR A PARTICULAR PURPOSE AND ALL WARRANTIES ARISING FROM COURSE OF DEALING OR USAGE OR TRADE).

Tidellex® Technologies Products are guaranteed for a period of one year from date of shipment, against defective workmanship and material only, when properly installed, operated and serviced in accordance with Tidellex® Technologies' recommendations. Replacement for items of Red Valve's manufacture will be made free of charge if proved to be defective within such year; but not claim for transportation, labor or consequential damages shall be allowed. We shall have the option of requiring the return of the defective product to our factory, with transportation charges prepaid, to establish the claim and our lability shall be limited to the repair or replacement of the defective product, F.O.B. our factory. Tideflex® Technologies will not assume costs incurred to remove or install defective products nor shall we inpur backcharges or liquidated damages as a result of warranty work. Tideflex® Technologies does not guarantee resistance to corrosion erosion, abrasion or other sources of faiture, nor does Tideflex® Technologies guarantee a minimum length of service, or that the product shall be lift for any particular service. Faiture of purchaser to give prompt written notice of any alleged defect under this guarantee forthwith upon its discovery, or use, and possession thereof after an attempt has been made and completed to remedy defects therein, or faiture to return product or part for replacement as herein provided, or faiture to install and operate said products and parts according to instructions turnished by Tideflex® Technologies, or faiture to pay entire contract price when due, shall be a waive, by purchaser of all rights under these representations. All orders accepted shall be deemed accepted subject to this warranty which shall be exclusive of any other or previous warranty, and shall be the only effective guarantee or warranty binding on Tideflex® Technologies, anything on the contrary contained in purchaser's order, or represented by any agent or employee of Tideflex® Technologies in writing or otherwis



700 North Bell Avenue Carnegie, PA 15106 A phone: 412 279-0044 fax: 412 279-7878 WEB: www.tideflex.com

## **APPENDIX G**

## APPENDIX G Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

IN	SPECTION BEING CONDUCTED:	
QI	UARTERLY ANNUALLY AFTER 1" or GREATER RAINF.	ALL_ø/2 michi
Αŀ	TTER 1.25" IN 2 HOUR STORM EVENT UNDER SPECIAL CIRC	UMSTANCES
In:	spection Date: ////////////////////////////////////	.Çoµ∿
«Į	BASIN" Inspection:	
		Yes No N/A
	Catch Basins (23 Structures):	
1.	Are catch basins properly draining?	$X \square$
2.	Are the catch basins clear of trash, sediment, and debris?	
3.	Has vegetation been removed from all catch basin areas?	
4.	Are there any signs of damage or deterioration of catch basins?  If yes, which catch basin(s)?  (Refer to Record Drawings for catch basin numbers)	
	Stormwater Detention Basin and Surface Sand Filter:	
5.	Does the basin have pooled or standing water?  If yes, describe where:	
6.	What is the water height?	
	Approximately how many hours ago was the last rainfall?	
	How many inches of rain?	
7.	Does the bottom appear relatively flat? No sand has washed away?	X
8.	Are concentrated flows of runoff being unexpectedly directed into the basin?  If yes, describe where:	
9.	Is there any damage to the sand bed or berms?	
	Has vegetation been removed from the basin areas?	

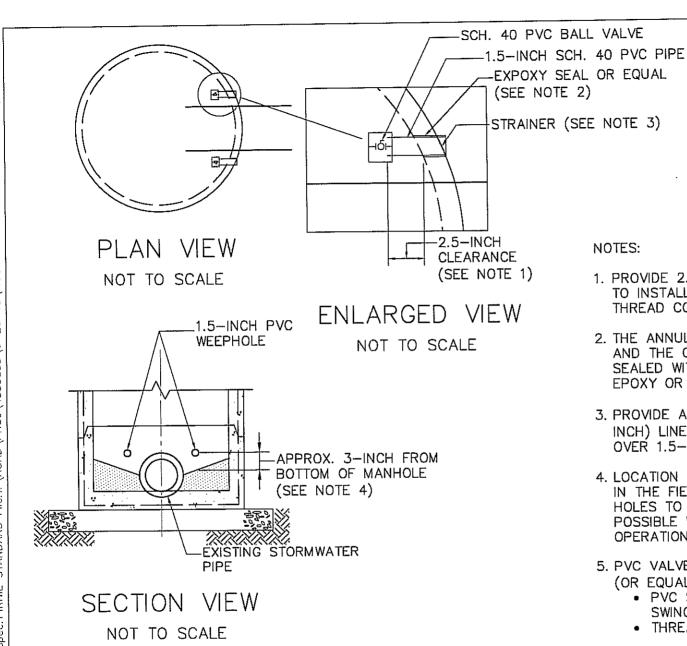
# APPENDIX G Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

INS	PECTION BEING CONDUCTED:					
QUA	ARTERLY ANNUALLY A	FTER 1" o	GREATER RAINFA	.LL		
AFT	AFTER 1.25" IN 2 HOUR STORM EVENT UNDER SPECIAL CIRC			JMSTAN	CES_	
Insp	pection Date: 10/30/13 V	Veather:	Cloury	<u>s</u>	5 -	5
Insp	pection Date: $\frac{10/30/13}{\text{None:}}$ William $\frac{2an}{1}$	1BRANI	7			
"B.	ASIN" Inspection:					
				Yes	No	N/A
	Catch Basins (23 Structures):					
1.	Are catch basins properly draining?			$\times$		
2.	Are the catch basins clear of trash, sedime	nt, and del	oris?	X		
3.	Has vegetation been removed from all cate	ch basin ar	eas?	X		
4.	Are there any signs of damage or deterioral If yes, which catch basin(s)? (Refer to Record Drawings for catch basin	<u>.</u>		Committee of the Commit	X	
	Stormwater Detention Basin and Surface	ce Sand Fi	ilter:			
5.	Does the basin have pooled or standing was If yes, describe where:				X	
6.	What is the water height?					
	Approximately how many hours ago was t	the last rai	nfall?			
	How many inches of rain?					
7.	Does the bottom appear relatively flat? No	sand has	washed away?	X	d makes of district	
8.	Are concentrated flows of runoff being unbasin?  If yes, describe where:	expectedly	y directed into the		X	y and out to my
9.	Is there any damage to the sand bed or ber	rms?			X	
	Has vegetation been removed from the bar			$\times$	· · · · · · · · · · · · · · · · · · ·	

#### APPENDIX G

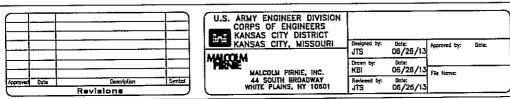
### Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

	Inlet and Outlet Structures:
	11. Are the five inlet and four outlet structures draining properly?
	12. Is there any standing water?  If yes, describe where:
	13. Are the inlets clear of trash, sediment, and debris?
	14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?
	15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:
	16. Has vegetation been removed from the inlets and outlets?
	Additional descriptions of where repairs or maintenance is needed:
_	CB-12: INSTALLED SEEP Holes To Drain THE AREA
	OF THE CONTAINMENT LINER FOR THE STORM Drain
	System (ATTACHED ARE A Drawing AND photograph)
40	Repair Fence IN THE North EAST Corner of SIR.
4	Repair Fence IN THE North EAST Corner of SITE. THEIR WILL BE 7 GATES ON THE ENTIRE SITE THAT
	Will be changed To a UNIVERSAL Lock System
	with I key entry.
_	Remove all Debin by Fences.
-	THERE'S Ahole by THE FENCE FOR easy Access
	of mail ON HAMILTON Bluck.
_	CUT ANY Tree limbs THAT MY DAMAGE FENC
حنور.	SECURE The well CAP INSIDE THE CATCH BASIA
	Inspector's Signature



#### NOTES:

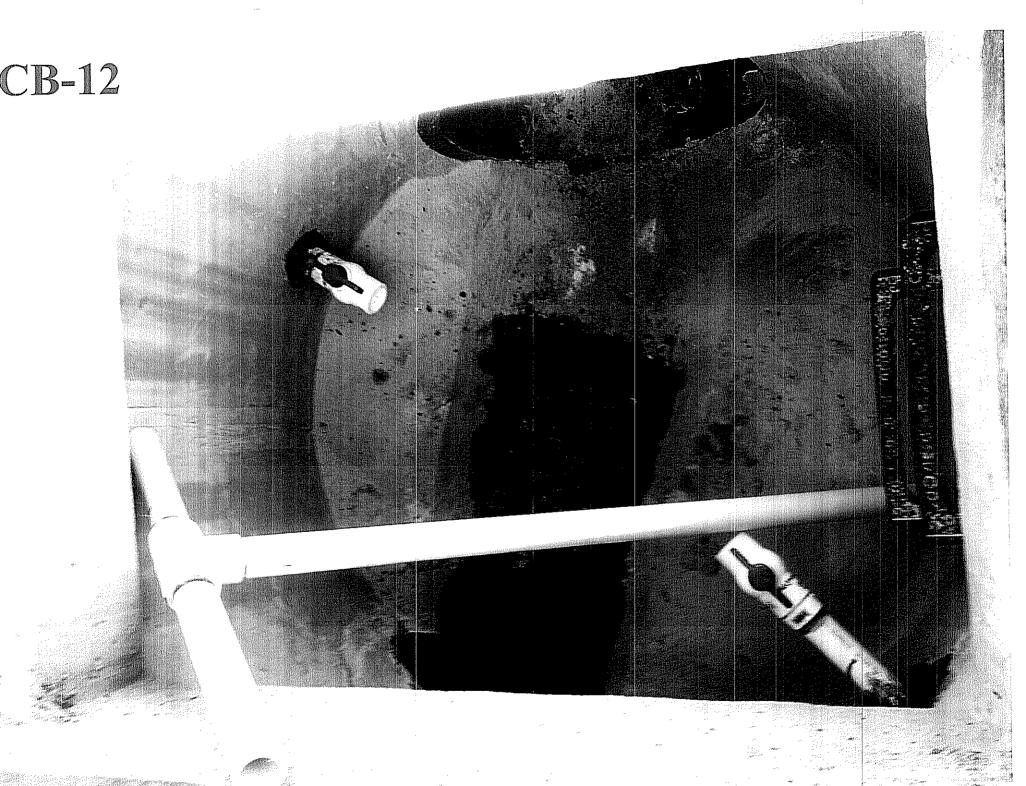
- 1. PROVIDE 2.5-INCH CLEARANCE OF PVC PIPE TO INSTALL 1.5-INCH PVC SLIP BALL VALVE. THREAD CONNECTION PREFERRED.
- 2. THE ANNULAR SPACE BETWEEN THE PVC PIPE AND THE CONCRETE MANHOLE SHALL BE SEALED WITH POLYURETHANE INJECTION GROUT. EPOXY OR EQUAL TO PROVIDE A TIGHT SEAL.
- 3. PROVIDE A STAINLESS STEEL 30 MESH (1/64th INCH) LINE STRAINER OR EQUAL THAT FITS OVER 1.5-INCH NPT.
- 4. LOCATION OF WEEP HOLES TO BE COORDINATED IN THE FIELD WITH THE ENGINEER. WEEP HOLES TO BE SITUATED AT LOWEST ELEVATION POSSIBLE WHILE ALLOWING SUFFICIENT OPERATION OF THE BALL VALVE.
- 5. PVC VALVE TO CONSIST OF THE FOLLOWING (OR EQUAL):
  - . PVC SCHEDULE 40 BALL VALVE WITH SWING HANDLE
  - THREADED CONNECTION



CORNELL-DUBILIER ELECTRONICS SUPERFUND SITE OU-2 SOILS REMEDIATION SOUTH PLAINFIELD, NEW JERSEY

BALL VALVE INSTALLATION PLAN

FIGURE 1



## APPENDIX G

## Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

"Pavement"	Inpsection	(Part of Annual	Inspection):
------------	------------	-----------------	--------------

	Yes	No	N/A
Is there any standing water?  If yes, describe where:		$\times$	
2. Are there any signs of cracking?  If so, note location and maintenance effort below.	X	1, 2° 20 2 2 2° 10° 10° 10° 10° 10° 10° 10° 10° 10° 10	
<ol> <li>Are there any signs of disintegration?</li> <li>If so, note location and maintenance effort below.</li> </ol>		X	
4. Are there any signs of distortion?  If so, note location and maintenance effort below.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	X	
5. Has all vegetation been removed?  If applicable, note location of vegetation below.	X		
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?	1	X	
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatment been applied?	S	X	
When was the date of the last CPM treatment? (Refer to Section 2.2.3 of the Operation & Maintenance Manual)			
Additional descriptions of where repairs or maintenance is needed:			
ON THE North EAST Corner of The SI	1e /	7 40	RE
ARE cracks ON THE Surpace of The A	p ho	r/T	Due
To Heavy MoisTure IN THE Area before	ore_	THE	· INVITUAL
To Heavy MoisTure IN THE Area begans placement of Asphalt. The CAD WAS .  Such, No repairs required.	NTa	cti	4ND 4S
such, No repairs required.			
FIRE HYDRENTS ON. SITE ARE NOT FUNCTION			
moter to Hydrant require repair 15 Neces	led a	Ta	later Date.
THE meter is shut of to hydrants which m Inspector's Signature preper operation at a laton	UST E	he co	hecked For
Mills	<i>5-</i> 0	7 1 4	

# APPENDIX G Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

### "Tree Grove" Inspection:

_	<b>L</b>	
1.	Is there any tree damage from storms?	Yes No N/A
2.	If yes, describe:  Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance:  (Refer to Section 2.3.2 of the Operation & Maintenance Manual)	
	Was the last Annual Arborist Inspection performed?  Date of previous inspection:  (Refer to Section 2.3.3 of the Operation & Maintenance Manual)  Iditional descriptions of where repairs or maintenance is needed:	
		= grove
<u></u>	erea Have an overgrowth.	
_		
_		
	spector's Signature	
	spector's Signature	

#### APPENDIX G

## Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

INSPECTION BEING CONDUCTED: QUARTERLY ANNUALLY AFTER 1" or GREATER RAINFALL AFTER 1.25" IN 2 HOUR STORM EVENT \_\_\_\_ UNDER SPECIAL CIRCUMSTANCES \_\_\_ Inspection Date: 10/1/13 Weather: SUNNY
Inspector's Name: William Zamajeann "BASIN" Inspection: Yes No N/A Catch Basins (23 Structures): 1. Are catch basins properly draining? 2. Are the catch basins clear of trash, sediment, and debris? 3. Has vegetation been removed from all catch basin areas? 4. Are there any signs of damage or deterioration of catch basins? If yes, which catch basin(s)? (Refer to Record Drawings for catch basin numbers) Stormwater Detention Basin and Surface Sand Filter: 5. Does the basin have pooled or standing water? If yes, describe where: 6. What is the water height? Approximately how many hours ago was the last rainfall? How many inches of rain? 7. Does the bottom appear relatively flat? No sand has washed away? 8. Are concentrated flows of runoff being unexpectedly directed into the basin? If yes, describe where: 9. Is there any damage to the sand bed or berms? 10. Has vegetation been removed from the basin areas?

	Yes	No	N/A
Inlet and Outlet Structures:		/	
11. Are the five inlet and four outlet structures draining properly?			
12. Is there any standing water?  If yes, describe where:	By a mann rung d		
13. Are the inlets clear of trash, sediment, and debris?			
14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?			
15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:			
16. Has vegetation been removed from the inlets and outlets?		James 11.2-4	
Additional descriptions of where repairs or maintenance is needed:			
		<del></del>	
<b>Th</b>			
The state of the s			
		n	<del></del>
		<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>	
MA			
Inspector's Signature			<del></del>

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

I.	Is there any tree damage from storms?	Yes No N/A
	If yes, describe:	
2.	Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
·4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance:	
6.	Was the last Annual Arborist Inspection performed?  Date of previous inspection: (Refer to Section 2.3.3 of the Operation & Maintenance Manual)	LONGE LEGISLES
	ditional descriptions of where repairs or maintenance is needed:	
<u>U</u>	SEPA/USACE requested that The rea HAVE an Overgrowth.	grove
Inst	pector's Signature	

### Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

	Yes No N/A
1. Is there any standing water?	$\sqrt{}$
If yes, describe where:	
2. Are there any signs of cracking?	$\sqrt{}$
If so, note location and maintenance effort below.	
3. Are there any signs of disintegration?	V
If so, note location and maintenance effort below.	
4. Are there any signs of distortion?	$\checkmark$
If so, note location and maintenance effort below.	
<ol> <li>Has all vegetation been removed?</li> <li>If applicable, note location of vegetation below.</li> </ol>	
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?	
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatments been applied?	
When was the date of the last CPM treatment?	
(Refer to Section 2.2.3 of the Operation & Maintenance Manual)	
Additional descriptions of where repairs or maintenance is needed:	
Small Ponding area around site: 3 in the	N.E Area,
1 at SW of water Tower, 1 at N.E. of Gate,	ncesure.
Disentergration at N.E' Corner by South E.	trance gal
Disintegration at S.C. area by manhole	13.4
	HA P
	1111
	······································
I	
Inspector's Signature	

HN	SPECTION BEING CONDUCTED:	
QI	UARTERLY ANNUALLY AFTER 1" or GREATER RAI	INFALL
Al	TTER 1.25" IN 2 HOUR STORM EVENT UNDER SPECIAL CI	RCUMSTANCES
In	spection Date: 8/29/13 Weather: PARTly spector's Name: William Zameraw, Kim Lic	cloury
In	spector's Name: William ZAMBRAN, Kim Lic	Epield
"i	BASIN" Inspection:	
		Yes No N/A
	Catch Basins (23 Structures):	
1.	Are catch basins properly draining?	30
2.	Are the catch basins clear of trash, sediment, and debris?	
3.	Has vegetation been removed from all catch basin areas?	Z
4.	Are there any signs of damage or deterioration of catch basins?  If yes, which catch basin(s)?	
	(Refer to Record Drawings for catch basin numbers)	
	Stormwater Detention Basin and Surface Sand Filter:	
5.	Does the basin have pooled or standing water?  If yes, describe where:	
6.	What is the water height?	
	Approximately how many hours ago was the last rainfall?	
	How many inches of rain?	
7.	Does the bottom appear relatively flat? No sand has washed away?	
8.	Are concentrated flows of runoff being unexpectedly directed into the basin?	
	If yes, describe where:	
9.	Is there any damage to the sand bed or berms?	
10.	Has vegetation been removed from the basin areas?	J i

	Y es	INO	N/A
Inlet and Outlet Structures:			
11. Are the five inlet and four outlet structures draining properly?	$\overline{A}$		
12. Is there any standing water?  If yes, describe where:			]
13. Are the inlets clear of trash, sediment, and debris?			
14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?	J		
15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:			
16. Has vegetation been removed from the inlets and outlets?			
Additional descriptions of where repairs or maintenance is needed:			
		-	
		-	
			<del></del>
Inspector's Signature			

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

		Yes No N/A
1.	Is there any tree damage from storms?  If yes, describe:	
2.	Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance: (Refer to Section 2.3.2 of the Operation & Maintenance Manual)	T A
	Was the last Annual Arborist Inspection performed?  Date of previous inspection:  (Refer to Section 2.3.3 of the Operation & Maintenance Manual)  ditional descriptions of where repairs or maintenance is needed:	
_(	ISEPA/USACE Requested that The	grove
	rea have an Overgrowth.	
	growth is Flourishing From the bottom.	SUT NEW
	pactor's Signature	
0		

#### Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

	Yes No N/A
1. Is there any standing water?  If yes, describe where:	
2. Are there any signs of cracking?  If so, note location and maintenance effort below.	
3. Are there any signs of disintegration?  If so, note location and maintenance effort below.	
4. Are there any signs of distortion?  If so, note location and maintenance effort below.	
5. Has all vegetation been removed?  If applicable, note location of vegetation below.	
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?	
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatment been applied?  When was the date of the last CPM treatment?  (Refer to Section 2.2.3 of the Operation & Maintenance Manual)	ts 🗌 💟 📗
Additional descriptions of where repairs or maintenance is needed:	
SES ISAWAITING DIRECTION From USACE	- USEPA ON SEALING COE
SMAIL PONDING area around SITE: 3 at N.E. ARE  1 by N.E. of Gate measure.	
DISINTERGRATION: IN The J.E. AREA of site	by MANHOLE 13 A
DISTORTION: N.E. CORNER by South entrance	
Sy the west sine of site, by water ch	
Inspector's Signature	

	Yes	No	N/A
Inlet and Outlet Structures:			•
11. Are the five inlet and four outlet structures draining properly?	$\supset$		
12. Is there any standing water?		$\overline{\mathbf{x}}$	
If yes, describe where:			
13. Are the inlets clear of trash, sediment, and debris?	X		
14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?	X		
15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:	:]	X	
16. Has vegetation been removed from the inlets and outlets?		X	
Additional descriptions of where repairs or maintenance is needed:			
•			
	<u> </u>		
			·
	<del></del>	·	
			·····
Increator's Cignature			
Inspector's Signature			

	Yes	No	N/A
1. Is there any standing water?  If yes, describe where:	]	X	. ]
2. Are there any signs of cracking?  If so, note location and maintenance effort below.		X	
3. Are there any signs of disintegration?  If so, note location and maintenance effort below.	:	$\overline{\times}$	
4. Are there any signs of distortion?  If so, note location and maintenance effort below.		$\square$	
5. Has all vegetation been removed?  If applicable, note location of vegetation below.		X	
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?			
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatments been applied?  When was the date of the last CPM treatment?  (Refer to Section 2.2.3 of the Operation & Maintenance Manual)		X	<u></u>
Additional descriptions of where repairs or maintenance is needed:			
Inspector's Signature			
maheerer a nighterite			

		Yes No N/A
1.	Is there any tree damage from storms?  If yes, describe:	
2.	Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance:  (Refer to Section 2.3.2 of the Operation & Maintenance Manual)	
6.	Was the last Annual Arborist Inspection performed?  Date of previous inspection:  (Refer to Section 2.3.3 of the Operation & Maintenance Manual)	
Ac	Iditional descriptions of where repairs or maintenance is needed:	
Ins	pector's Signature	

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

### **Basin Drainage Rate Inspection:**

Inspector's Signature

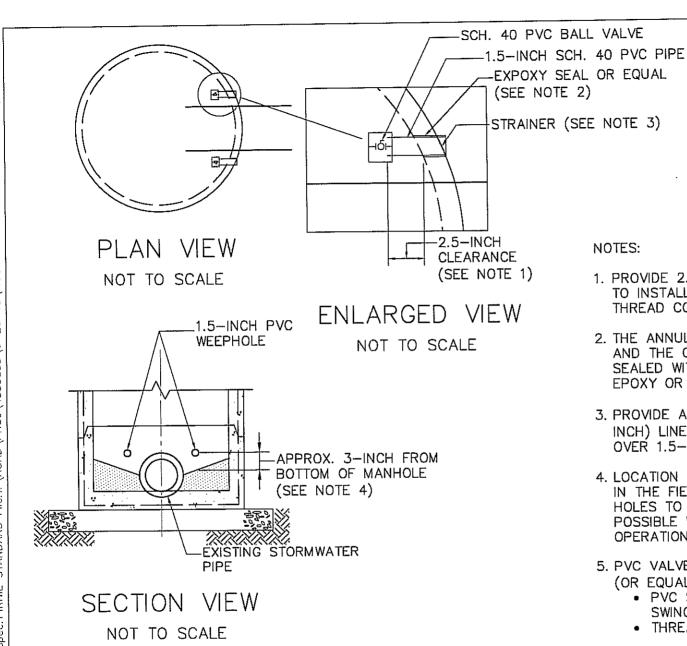
	/ 1 .	1		Δ.	1 -	11	- 1
1	(completed	i huice i	יוחסנו ח	atter	מוסוים סור	ramtall	onontl
Н	Competition	C C PP ECC L	i y c cii	ujici	CECUIZIE	, millioni	UVUILI

		ent Information		
eq1	airements: 1.25	" of rain in 2 hours		
tari	-			
top	•			
ıch	es of Rainfall: _			
กรถ	ection Data:			
		hours after design rain eve	nt	
		inspections every 2 hours	until the height of wate	r drops below the top
ggr	egate in the mid	ldle basin.		
Γ	Inspection	Target Time From	Actual Time	Water height (ft)
	Run#	Event (hrs)		<u> </u>
	1	16		
Ī	2	18		
ŀ	3	20		
-	4	22	The state of the s	
f	5	24		
ŀ	6	26		
-	7	28		
	8	30		
		32		· · · · · · · · · · · · · · · · · · ·
-	9			E .

IN	SPECTION BEING CONDUCTED:	
QI	UARTERLY ANNUALLY AFTER 1" or GREATER RAINF.	ALL_ø/2 michi
Αŀ	TTER 1.25" IN 2 HOUR STORM EVENT UNDER SPECIAL CIRC	UMSTANCES
In:	spection Date: ////////////////////////////////////	.Çoµ∿
«Į	BASIN" Inspection:	
		Yes No N/A
	Catch Basins (23 Structures):	
1.	Are catch basins properly draining?	$X \square$
2.	Are the catch basins clear of trash, sediment, and debris?	
3.	Has vegetation been removed from all catch basin areas?	
4.	Are there any signs of damage or deterioration of catch basins?  If yes, which catch basin(s)?  (Refer to Record Drawings for catch basin numbers)	
	Stormwater Detention Basin and Surface Sand Filter:	
5.	Does the basin have pooled or standing water?  If yes, describe where:	
6.	What is the water height?	
	Approximately how many hours ago was the last rainfall?	
	How many inches of rain?	
7.	Does the bottom appear relatively flat? No sand has washed away?	X
8.	Are concentrated flows of runoff being unexpectedly directed into the basin?  If yes, describe where:	
9.	Is there any damage to the sand bed or berms?	
	Has vegetation been removed from the basin areas?	

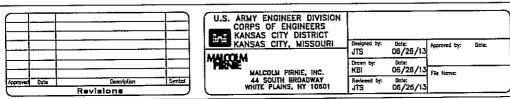
INS	PECTION BEING CONDUCTED:					
QUA	ARTERLY ANNUALLY A	FTER 1" o	GREATER RAINFA	.LL		
AFT	ER 1.25" IN 2 HOUR STORM EVENT	UNE	ER SPECIAL CIRCU	JMSTAN	CES_	
Insp	pection Date: 10/30/13 V	Veather:	Cloury	<u>s</u>	5 -	5
Insp	pection Date: $\frac{10/30/13}{\text{None:}}$ William $\frac{2an}{1}$	1BRANI	7			
"B.	ASIN" Inspection:					
				Yes	No	N/A
	Catch Basins (23 Structures):					
1.	Are catch basins properly draining?			$\times$		
2.	Are the catch basins clear of trash, sedime	nt, and del	oris?	X		
3.	Has vegetation been removed from all cate	ch basin ar	eas?	X		
4.	Are there any signs of damage or deterioral If yes, which catch basin(s)? (Refer to Record Drawings for catch basin	<u>.</u>		Committee of the Commit	X	
	Stormwater Detention Basin and Surface	ce Sand Fi	ilter:			
5.	Does the basin have pooled or standing wa If yes, describe where:				X	
6.	What is the water height?					
	Approximately how many hours ago was t	the last rai	nfall?			
	How many inches of rain?					
7.	Does the bottom appear relatively flat? No	sand has	washed away?	X	d makes of district	
8.	Are concentrated flows of runoff being unbasin?  If yes, describe where:	expectedly	y directed into the		X	y and out to my
9.	Is there any damage to the sand bed or ber	rms?			X	
	Has vegetation been removed from the bar			$\times$	· · · · · · · · · · · · · · · · · · ·	

	Inlet and Outlet Structures:
	11. Are the five inlet and four outlet structures draining properly?
	12. Is there any standing water?  If yes, describe where:
	13. Are the inlets clear of trash, sediment, and debris?
	14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?
	15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:
	16. Has vegetation been removed from the inlets and outlets?
	Additional descriptions of where repairs or maintenance is needed:
_	CB-12: INSTALLED SEEP Holes To Drain THE AREA
	OF THE CONTAINMENT LINER FOR THE STORM Drain
	System (ATTACHED ARE A Drawing AND photograph)
40	Repair Fence IN THE North EAST Corner of SIR.
4	Repair Fence IN THE North EAST Corner of SITE. THEIR WILL BE 7 GATES ON THE ENTIRE SITE THAT
	Will be changed To a UNIVERSAL Lock System
	with I key entry.
_	Remove all Debin by Fences.
-	THERE'S Ahole by THE FENCE FOR easy Access
	of mail on HAMILTON Bluck.
_	CUT ANY Tree limbs THAT MY DAMAGE FENC
حنور.	SECURE The well CAP INSIDE THE CATCH BASIA
	Inspector's Signature



#### NOTES:

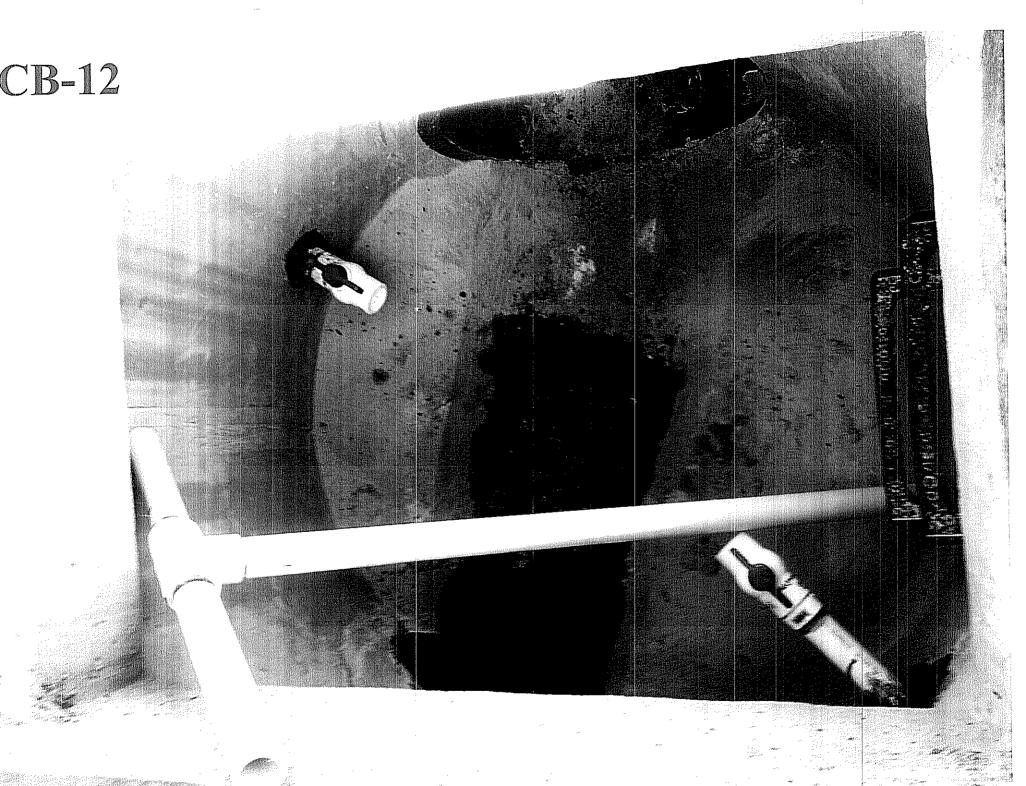
- 1. PROVIDE 2.5-INCH CLEARANCE OF PVC PIPE TO INSTALL 1.5-INCH PVC SLIP BALL VALVE. THREAD CONNECTION PREFERRED.
- 2. THE ANNULAR SPACE BETWEEN THE PVC PIPE AND THE CONCRETE MANHOLE SHALL BE SEALED WITH POLYURETHANE INJECTION GROUT. EPOXY OR EQUAL TO PROVIDE A TIGHT SEAL.
- 3. PROVIDE A STAINLESS STEEL 30 MESH (1/64th INCH) LINE STRAINER OR EQUAL THAT FITS OVER 1.5-INCH NPT.
- 4. LOCATION OF WEEP HOLES TO BE COORDINATED IN THE FIELD WITH THE ENGINEER. WEEP HOLES TO BE SITUATED AT LOWEST ELEVATION POSSIBLE WHILE ALLOWING SUFFICIENT OPERATION OF THE BALL VALVE.
- 5. PVC VALVE TO CONSIST OF THE FOLLOWING (OR EQUAL):
  - . PVC SCHEDULE 40 BALL VALVE WITH SWING HANDLE
  - THREADED CONNECTION



CORNELL-DUBILIER ELECTRONICS SUPERFUND SITE OU-2 SOILS REMEDIATION SOUTH PLAINFIELD, NEW JERSEY

BALL VALVE INSTALLATION PLAN

FIGURE 1



"Pavement"	Inpsection	(Part of Annual	Inspection):
------------	------------	-----------------	--------------

	Yes	No	N/A
1. Is there any standing water?  If yes, describe where:		$\times$	
<ol><li>Are there any signs of cracking?</li><li>If so, note location and maintenance effort below.</li></ol>	X	1 Profes 1 4 Profes	
<ol> <li>Are there any signs of disintegration?</li> <li>If so, note location and maintenance effort below.</li> </ol>		X	
4. Are there any signs of distortion?  If so, note location and maintenance effort below.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	X	
5. Has all vegetation been removed?  If applicable, note location of vegetation below.	X		
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?	1 1 1 1 1 1	X	
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatments been applied?	3	X	
When was the date of the last CPM treatment? (Refer to Section 2.2.3 of the Operation & Maintenance Manual)			
Additional descriptions of where repairs or maintenance is needed:			
ON THE North EAST Corner of The Si	1e /	7 40	RE
ARE cracks on THE Surface of The Ag	pho	17	Due
To Heavy MoisTure IN THE Area before	ve !	THE	INVITUAL
To Heavy MoisTure IN THE Area before placement of Asphalt. The CAPWASI Such, No repairs required.	NTa	cti	4ND 4S
such, No repairs required.			
FIRE HYDRENTS ON. SITE ARE NOT FUNCTION			
moter to Hydrant require repring 1 F Necd	rd a	Ta	later Date.
THE meter is shut of to hydrants which me Inspector's Signature preper operation ut a lator	1376 110a	e co	necked For
Mells	シーレ	7 1 4	

_		
1.	Is there any tree damage from storms?	Yes No N/A
2.	If yes, describe:  Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance:  (Refer to Section 2.3.2 of the Operation & Maintenance Manual)	
	Was the last Annual Arborist Inspection performed?  Date of previous inspection:  (Refer to Section 2.3.3 of the Operation & Maintenance Manual)  Iditional descriptions of where repairs or maintenance is needed:	
		e grove
<u></u>	ISEPA/USACE requested THAT The erea Have an overgrowth.	
_		
	spector's Signature	
بزد	speciol a digitatine	

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

INSPECTION BEING CONDUCTED: QUARTERLY ANNUALLY AFTER 1" or GREATER RAINFALL AFTER 1.25" IN 2 HOUR STORM EVENT \_\_\_\_ UNDER SPECIAL CIRCUMSTANCES \_\_\_ Inspection Date: 10/1/13 Weather: SUNNY
Inspector's Name: William Zamajeann "BASIN" Inspection: Yes No N/A Catch Basins (23 Structures): 1. Are catch basins properly draining? 2. Are the catch basins clear of trash, sediment, and debris? 3. Has vegetation been removed from all catch basin areas? 4. Are there any signs of damage or deterioration of catch basins? If yes, which catch basin(s)? (Refer to Record Drawings for catch basin numbers) Stormwater Detention Basin and Surface Sand Filter: 5. Does the basin have pooled or standing water? If yes, describe where: 6. What is the water height? Approximately how many hours ago was the last rainfall? How many inches of rain? 7. Does the bottom appear relatively flat? No sand has washed away? 8. Are concentrated flows of runoff being unexpectedly directed into the basin? If yes, describe where: 9. Is there any damage to the sand bed or berms? 10. Has vegetation been removed from the basin areas?

	Yes	No	N/A
Inlet and Outlet Structures:		/	
11. Are the five inlet and four outlet structures draining properly?			
12. Is there any standing water?  If yes, describe where:	By a mann rung d		
13. Are the inlets clear of trash, sediment, and debris?			
14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?			
15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:			
16. Has vegetation been removed from the inlets and outlets?		James 11.2-4	
Additional descriptions of where repairs or maintenance is needed:			
		<del></del>	
<b>Th</b>			
The state of the s			
		n	<del></del>
		<u>,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>	
MA			
Inspector's Signature			<del></del>

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

I.	Is there any tree damage from storms?	Yes No N/A
	If yes, describe:	
2.	Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
·4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance:	
6.	Was the last Annual Arborist Inspection performed?  Date of previous inspection: (Refer to Section 2.3.3 of the Operation & Maintenance Manual)	LONGE LEGISLES
	ditional descriptions of where repairs or maintenance is needed:	
<u>U</u>	SEPA/USACE requested that The rea HAVE an Overgrowth.	grove
Inst	pector's Signature	

### Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

	Yes No N/A
1. Is there any standing water?	$\sqrt{}$
If yes, describe where:	
2. Are there any signs of cracking?	$\sqrt{}$
If so, note location and maintenance effort below.	
3. Are there any signs of disintegration?	V
If so, note location and maintenance effort below.	
4. Are there any signs of distortion?	$\checkmark$
If so, note location and maintenance effort below.	
<ol> <li>Has all vegetation been removed?</li> <li>If applicable, note location of vegetation below.</li> </ol>	
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?	
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatments been applied?	
When was the date of the last CPM treatment?	
(Refer to Section 2.2.3 of the Operation & Maintenance Manual)	
Additional descriptions of where repairs or maintenance is needed:	
Small Ponding area around site: 3 in the	N.E Area,
1 at SW of water Tower, 1 at N.E. of Gate,	ncesure.
Disentergration at N.E' Corner by South E.	trance gal
Disintegration at S.C. area by manhole	13.4
	HA P
	1111
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I	
Inspector's Signature	

IN	SPECTION BEING CONDUCTED:	
QI	UARTERLY ANNUALLY AFTER 1" or GREATER RAI	NFALL
Al	TTER 1.25" IN 2 HOUR STORM EVENT UNDER SPECIAL CIT	RCUMSTANCES
In	spection Date: 8/29/13 Weather: Partly spector's Name: William Zambran, Kim Lice	cloury
In	spector's Name: William ZAMBRAN, Kim Lici	icpield
	BASIN" Inspection:	
		Yes No N/A
	Catch Basins (23 Structures):	1,11
1.	Are catch basins properly draining?	70-
2.	Are the catch basins clear of trash, sediment, and debris?	
3.	Has vegetation been removed from all catch basin areas?	
4.	Are there any signs of damage or deterioration of catch basins?  If yes, which catch basin(s)?  (Refer to Record Drawings for catch basin numbers)	
	Stormwater Detention Basin and Surface Sand Filter:	
5.	Does the basin have pooled or standing water?  If yes, describe where:	
6.	What is the water height?	
	Approximately how many hours ago was the last rainfall?	
	How many inches of rain?	
7.	Does the bottom appear relatively flat? No sand has washed away?	
8.	Are concentrated flows of runoff being unexpectedly directed into the basin?	
	If yes, describe where:	,
9.	Is there any damage to the sand bed or berms?	
10.	Has vegetation been removed from the basin areas?	V i

	Y es	INO	N/A
Inlet and Outlet Structures:			
11. Are the five inlet and four outlet structures draining properly?	$\overline{A}$		
12. Is there any standing water?  If yes, describe where:		<b>\</b>	
13. Are the inlets clear of trash, sediment, and debris?	$ \mathbf{A} $	:	
14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?	J		
15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:			]
16. Has vegetation been removed from the inlets and outlets?			
Additional descriptions of where repairs or maintenance is needed:			
//2.000.000.000			
			***
		<u>.</u>	
Inspector's Signature			

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

		Yes No N/A
1.	Is there any tree damage from storms?  If yes, describe:	
2.	Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance: (Refer to Section 2.3.2 of the Operation & Maintenance Manual)	T A
	Was the last Annual Arborist Inspection performed?  Date of previous inspection:  (Refer to Section 2.3.3 of the Operation & Maintenance Manual)  ditional descriptions of where repairs or maintenance is needed:	
_(	ISEPA/USACE Requested that The	grove
	rea have an Overgrowth.	
	growth is Flourishing From the bottom.	SUT NEW
	pactor's Signature	
0		

#### Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

	Yes No N/A
1. Is there any standing water?  If yes, describe where:	
2. Are there any signs of cracking?  If so, note location and maintenance effort below.	
3. Are there any signs of disintegration?  If so, note location and maintenance effort below.	
4. Are there any signs of distortion?  If so, note location and maintenance effort below.	
5. Has all vegetation been removed?  If applicable, note location of vegetation below.	
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?	
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatment been applied?  When was the date of the last CPM treatment?  (Refer to Section 2.2.3 of the Operation & Maintenance Manual)	ts V
Additional descriptions of where repairs or maintenance is needed:	
SES ISAWAITING DIRECTION From USACE	- /USEPA ON SEALING COM
SMAIL PONDING area around SITE: 3 at N.E. ARE  1 by N.E. of Gate measure.	
DISINTERGRATION: IN The J.E. AREA OF SITE	
DISTORTION: N.E. CORNER by South entrance	
Sy the west sine of site, by water ch	
Inspector's Signature	

	Yes	No	N/A
Inlet and Outlet Structures:			•
11. Are the five inlet and four outlet structures draining properly?	$\supset$		<del></del>
12. Is there any standing water?		X	
If yes, describe where:			
13. Are the inlets clear of trash, sediment, and debris?	X		
14. Are the outlets (standpipe, 3" orifice, secondary outlet, and emergency spillway) clear of trash, sediment, and debris?	X		
15. Are there any signs of damage or deterioration of inlet/outlet structures?  If yes, describe where:	: ]	X	]
16. Has vegetation been removed from the inlets and outlets?	4	X	
Additional descriptions of where repairs or maintenance is needed:			
•			
			<del></del>
		.,	·····
Inspector's Signature			<del></del>

	Yes	No	N/A
1. Is there any standing water?  If yes, describe where:	]	X	
2. Are there any signs of cracking?  If so, note location and maintenance effort below.		X	
3. Are there any signs of disintegration?  If so, note location and maintenance effort below.	:	$\overline{\times}$	
4. Are there any signs of distortion?  If so, note location and maintenance effort below.		$\square$	
5. Has all vegetation been removed?  If applicable, note location of vegetation below.		X	
6. Has the usage of the Site increased to a point that warrants a Pavement Condition Index (PCI) survey?			
7. Have any Critical Preventative Maintenance (CPM) Pavement Treatments been applied?  When was the date of the last CPM treatment?  (Refer to Section 2.2.3 of the Operation & Maintenance Manual)		X	J
Additional descriptions of where repairs or maintenance is needed:			
T			
Inspector's Signature			

		Yes No N/A
1.	Is there any tree damage from storms?  If yes, describe:	
2.	Is there an accumulation of tree debris?  If yes, describe:	
3.	Do any trees appear infested?  If yes, describe:	
4.	Do any trees appear malnourished?  If yes, describe:	
5.	Was the last Quarterly Seasonal Maintenance performed?  Date of previous maintenance:  (Refer to Section 2.3.2 of the Operation & Maintenance Manual)	
6.	Was the last Annual Arborist Inspection performed?  Date of previous inspection:  (Refer to Section 2.3.3 of the Operation & Maintenance Manual)	
Ac	Iditional descriptions of where repairs or maintenance is needed:	
Ins	pector's Signature	

# Operation & Maintenance Inspection Form Cornell-Dubilier Electronics Superfund Site Operable Unit (OU-2)

### **Basin Drainage Rate Inspection:**

Inspector's Signature

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Н	Competition	C C PP ECC L	i y c cii	ujioi	CECUIZIE	, millioni	UVUILI

		ent Information		
eq1	airements: 1.25	" of rain in 2 hours		
tari	-			
top	•			
ıch	es of Rainfall: _			
กรถ	ection Data:			
		hours after design rain eve	nt	
		inspections every 2 hours	until the height of wate	r drops below the top
ggr	egate in the mid	ldle basin.		
Γ	Inspection	Target Time From	Actual Time	Water height (ft)
	Run#	Event (hrs)		<u> </u>
	1	16		
Ī	2	18		
ŀ	3	20		
-	4	22	10 00 00 00 00 00 00 00 00 00 00 00 00 0	
f	5	24		
ŀ	6	26		
-	7	28		
	8	30		
		32		
-	9	<u></u>		E .